

12

SECTION 12 - CONTENTS

Further investigations

12 Further investigations

- 12.1 We have endeavoured to provide a comprehensive investigation for the proposed development within budgetary constraints there are areas, which we recommend further investigations be carried out. These are as follows: -
 - Subject to development proposals, further investigations in the area of the possible former air raid shelters to determine the depth and extent may be considered necessary
 - Further sampling and subsequent laboratory testing in the locality of the hydrocarbon impacted soils (trial pit TP14 and TP09) with a view to establishing the likely extent of such contamination and if the contamination has affected groundwater in the area, and if so derive remedial solutions (if any)
 - Further sampling and subsequent laboratory testing in the locality of borehole DTS05, with a view to establishing the likely extent of such contamination.
- 12.2 We would be pleased to carry out any of the supplementary investigations described above and provide proposals with costings on further instructions.

13 Remediation strategy and specification

13.1 At this stage, no remediation is required for the bulk of the campus, however there is the potential for some remedial works to be carried out in the areas local to trial pits TP09, and TP14, together with borehole DTS05. Recommendations for such supplementary investigations are provided in section 12. On completion of these investigations, a remedial strategy (if any) can be established.



















Appendix A

Definition of geotechnical terms used in this report. (sheet 1 of 2)

Foundations

Strip foundations. A foundation providing a continuous longitudinal ground bearing.

Trench fill concrete foundation. A trench filled with mass concrete providing continuous longitudinal ground bearing.

Pad foundation. An isolated foundation to spread a concentrated load.

Raft foundation. A foundation continuous in two directions, usually covering an area equal to or greater than the base area of the structure.

Substructure. That part of any structure (including building, road, runway or earthwork) which is below natural or artificial ground level. In a bridge this includes piers and abutments (and wing walls), whether below ground level or not, which support the superstructure.

Piled foundations and end bearing piles. A pile driven or formed in the ground for transmitting the weight of a structure to the soil by the resistance developed at the pile point or base and the friction along its surface. If the pile supports the load mainly by the resistance developed at its point or base, it is referred to as an end-bearing pile; if mainly by friction along its surface, as a friction pile.

Bored cast in place pile. A pile formed with or without a casing by excavating or boring a hole in the ground and subsequently filling it with plain or reinforced concrete.

Driven pile. A pile driven into the ground by the blows of a hammer or a vibrator.

Precast pile. A reinforced or prestressed concrete pile cast before driving.

Driven cast in place pile. A pile installed by driving a permanent or temporary casing, and filling the hole so formed with plan or reinforced concrete.

Displacement piles. Piled formed by displacement of the soil or ground through which they are driven.

Skin friction. The frictional resistance of the surrounding soil on the surface of cofferdam or caisson walls, and pile shafts.

Downdrag or negative skin friction. A downwards frictional force applied to the shaft of a pile caused by the consolidation of compressible strata, e.g. under recently placed fill. Downdrag has the effect of adding load to the pile and reducing the factor of safety.

Appendix A

Definition of geotechnical terms used in this report. (sheet 2 of 2)

Bearing values

Ultimate bearing capacity. The value of the gross loading intensity for a particular foundation at which the resistance of the soil to displacement of the foundation is fully mobilised.

Presumed bearing value. The net loading intensity considered appropriate to the particular type of ground for preliminary design purposes. The particular value is based on calculation from shear strength tests or other field tests incorporating a factor of safety against shear failure.

Allowable bearing pressure. The maximum allowable net loading intensity at the base of the foundation, taking into account the ultimate bearing capacity, the amount and kind of settlement expected and our estimate of ability of the structure to accommodate this settlement.

*Factor of safety.*__The ratio of the ultimate bearing capacity to the intensity of the applied bearing pressure or the ratio of the ultimate load to the applied load.

Road Pavements.

The following definitions are based on Transport and Road Research Laboratory (TRRL) Report LR1132.

Equilibrium CBR values. A prediction of the CBR value, which will be attained under the completed pavement.

Thin pavement. A thin pavement (which includes both bound and unbound pavement construction materials 1 in 300mm thick and a thick pavement is 1200mm thick (typical of motorway construction).

Appendix B (sheet 1 of 4)

Definition of geo-environmental terms used in this report.

Conceptual model

Textual and/or schematic hypothesis of the nature and sources of contamination, potential migration pathways (including description of the ground and groundwater) and potential receptors, developed on the basis of the information obtained from the investigatory process.

Contamination

Presence of a substance which is in, on or under land, and which has the potential to cause harm or to cause pollution of controlled water.

Controlled water

Inland freshwater (any lake, pond or watercourse above the freshwater limit), water contained in underground strata and any coastal water between the limit of highest tide or the freshwater line to the three mile limit of territorial waters.

Harm

Adverse effect on the health of living organisms, or other interference with ecological systems of which they form part, and, in the case of humans, including property.

Pathway

Mechanism or route by which a contaminant comes into contact with, or otherwise affects, a receptor.

Receptor

Persons, living organisms, ecological systems, controlled waters, atmosphere, structures and utilities that could be adversely affected by the contaminant(s).

Risk

Probability of the occurrence of, and magnitude of the consequences of, an unwanted adverse effect on a receptor.

Risk Assessment

Process of establishing, to the extent possible, the existence, nature and significance of risk.

Appendix B (sheet 2 of 4)

Definition of environmental risk / hazard terms used in this report. (sheet 1 of 2)

Based on CIRIA report C552 'Contaminated land risk assessment – A guide to good practice'.

Potential hazard severity definition

Category	Definition					
Severe	Acute risks to human health, catastrophic damage to buildings/property, major					
	pollution of controlled waters					
Medium	Chronic risk to human health, pollution of sensitive controlled waters, significant					
	effects on sensitive ecosystems or species, significant damage to buildings or					
	structures.					
Mild	Pollution of non sensitive waters, minor damage to buildings or structures.					
Minor	Requirement for protective equipment during site works to mitigate health effects,					
	damage to non sensitive ecosystems or species.					

Probability of risk definition

Category	Definition
High likelihood	Pollutant linkage may be present, and risk is almost certain to occur in long term, or there is evidence of harm to the receptor.
Likely	Pollutant linkage may be present, and it is probable that the risk will occur over the long term
Low likelihood	Pollutant linkage may be present, and there is a possibility of the risk occurring, although there is no certainty that it will do so.
Unlikely	Pollutant linkage may be present, but the circumstances under which harm would occur are improbable.

Level of risk for potential hazard definition

Probability of	Potential severity							
risk	Severe	Medium	Mild	Minor				
High Likelihood	Very high	High	Moderate	Low/Moderate				
Likely	High	Moderate	Low/Moderate	Low				
Low Likelihood	Moderate	Low/Moderate	Low	Very low				
Unlikely	Low/Moderate	Low	Very low	Very low				

Refer sheet 2 for definitions of 'very high' to 'low'



Appendix B

(sheet 3 of 4)

Definition of environmental risk / hazard terms used in this report. (sheet 2 of 2)

Based on CIRIA report C552 'Contaminated land risk assessment – A guide to good practice'.

Risk classifications and likely action required:

Very high risk

High probability that severe harm could arise to a designated receptor from an identified hazard OR there is evidence that severe harm to a designated receptor is currently happening. This risk, if realised is likely to result in substantial liability. Urgent investigation and remediation are likely to be required.

High risk

Harm is likely to arise to a designated receptor from an identified hazard. This risk, if realised, is likely to result in substantial liability. Urgent investigation is required and remedial works may be necessary in the short term and are likely over the long term.

Moderate risk

It is possible that harm could arise to a designated receptor from an identified hazard. However, it is either relatively unlikely that any such harm would be severe, or if any harm were to occur it is likely that the harm would be relatively mild. Investigation is normally required to clarify risks and to determine potential liability. Some remedial works may be required in the long term.

Low risk

It is possible that harm could arise to a designated receptor from an identified hazard but it is likely that this harm, if realised, would at worst normally be mild.

Very low risk

It is a low possibility that harm could arise to a designated receptor. On the event of such harm being realised it is not likely to be severe.

Appendix B (sheet 4 of 4)

Gaseous contamination - Extract copy of table 3 of BS8485:2007 Solutions scores

PROTECTION ELEMENT/SYSTEM	SCORE	COMMENTS			
a) Venting/dilution (see Annex A of BS8485)		•			
Passive sub-floor ventilation (venting layer	Very good	2.5	Ventilation performance in accordance		
can be a clear void or formed using gravel,	performance		with Annex A of BS8485.		
geocomposites, polystyrene void formers, etc)	Good	1	If passive ventilation is poor this is		
A)	performance		generally unaccentable and some form		
		1	of active system will be required		
Subfloor ventilation with active abstraction/pres	surization (venting		There have to be reduct management		
layers can be a clear void or formed using grave	el, geocomposites,	25			
polystyrene void formers, etc) ^{A)}		2.0	systems in place to ensure the		
			Ventilation system.		
			Active ventilation can always be		
			designed to meet good performance.		
			Mechanically assisted systems come in		
			two main forms: extraction and positive		
			pressurization		
			Assume car park is vented to deal with		
Ventilated car park (basement or undercroft)			car exhaust fumes, designed to		
		4	Building Regulations Document F and		
			IStructE guidance.		
b) Barriers		1			
Floor Slabs			It is good practice to install ventilation in		
Block and beam floor slab		0	all foundation systems to effect		
Reinforced concrete ground bearing floor slab	on roft with limited	0.5	pressure relief as a minimum.		
service penetrations that are cast into slab		1.5	Breaches in floor slabs such as joints		
Reinforced concrete cast in situ suspended	slab with minimal		have to be effectively sealed against		
service penetrations and water bars around a	I slab penetrations	1.5	gas ingress in order to maintain these		
and at joints	F		performances.		
Fully tanked basement		2			
c) Membranes					
laped and sealed membrane to reas	onable levels of	0.5	The performance of membranes is		
workmanship/in line with current good practice	with validation -,, -,		heavily dependent on the quality and		
workmanshin/in line with current good practice	under independent		design of the installation, resistance to		
inspection (COA) ^{B), C)}		1	damage after installations, and the		
Proprietary gas resistant membrane installed to	reasonable levels		integrity of joints.		
of workmanship/in line with current good practi	ce under CQA with				
integrity testing and independent validation.		2			
d) Monitoring and detection (not applicable	o non-managed pr	operty, or ir	isolation)		
Intermittent monitoring using hand held equipme	ent	0.5			
			Where fitted, permanent monitoring		
Permanent monitoring and alarm system ^{A)}	Installed in the	2	system ought to be installed in the		
	underfloor		underfloor venting/dilution system in the		
	venting/dilution		first instance but can also be provided		
-	System	4	within the occupied space as a fail safe.		
	building	1			
e) Pathway Intervention	building				
Pathway intervention		-	This can consist of site protection		
,			measures for off-site or on-site sources		
		(see Annex A of BS8485)			
NOTE In practice the choice of materials might well rely on factors such as construction method and the risk of damage after					
installation. It is important to ensure that the chosen combination gives an appropriate level of protection.					
^{A)} It is possible to test ventilation systems by installing monitoring probes for post installation validation.					
" It a 200g DPM material is to function as a gas barrier it should be installed according to BRE 212)/BRE 414), being taped					
$^{\text{C}}$ Polymeric Materials > 1 200g can be used to	improve confidence	in the barrie	r. Remember that their gas resistance is little		

⁵⁷ Polymeric Materials > 1 200g can be used to improve confidence in the barrier. Remember that their gas resistance is little more than the standard 1 200g (proportional to thickness) but their physical properties mean that they are more robust and resistant to site damage.

KEY TO LEGENDS (Extract from BS 5930;1999 table 11)

SOILS



Topsoil Made ground Boulders & Cobbles Gravel Sand Silt Clay Peat/Organic clays



Chalk Limestone Sandstone Siltstone Mudstone Shale Coal Conglomerate

Composite soil types are signified by combined symbols.

KEY TO SYMBOLS USED ON TRIAL PIT RECORDS

SAMPLING

В	bulk disturbed sample	U (38)	undisturbed (38mm) sample
D	disturbed sample	W	water sample
J	jar sample	CBR	undisturbed sample taken in CBR mould

NOTES ASSOCIATED WITH INSITU TESTING

Hand Held Shear Vane (V) The hand held shear vane provides a quick and direct measurement of undrained shear strength and is calibrated in kN/m^2 (Pa). The apparatus reads to a maximum shear strength of 150KPa. The results are reported in columns to the right of the trial pit legend.

Pocket Penetrometer Results (P) The pocket penetrometer is calibrated in kg/cm² and is deemed to measure the unconfined compressive strength of the soil under test. Under most conditions the unconfined compressive strength of the soil is twice the undrained shear strength of a soil. Thus, an Unconfined compressive strength 1kg/cm² = 0.009807 x 100 x 100 / 2 = 49.04, say 50KN/m² (equivalent undrained shear strength).

Tests are carried out in the sides of trial pits where access can be safely achieved otherwise testing is carried out on excavated intact lumps. Neither the hand held shear vane nor the pocket penetrometer is recognised in British Standard publications.



STANDARD KEY TO TRIAL PIT RECORDS

SEDIMENTARY ROCKS







Photographic record of trial pit TP04





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Photographic record of trial pit TP05



Notes.

1. Trial pit excavated using hand tools

2. Trial pit sides remained stable

3. Groundwater encountered at 0.65m.

4. Trial pit terminated due to water inflow.

5. Steel bar used to estimate base of

foundation.

6. All dimensions shown in metres

Richmond-Upon-Thames College, Egerton Road, Twickenham.

Detail of foundation exposed in Trial pit TP05

scale	date	drawn by	checked by
1:25 @ A4	28.05.08	MOW	
proje	ct ref	drg No.	revision
STE1	297R	TP05	







Cedar Barn, White Lodge, Walgrave, Northampton, NN6 9PY Tel: (01604) 781877 Fax: (01604) 781007 E-mail: mail@soiltechnics.net project ref drg No. STE1297R TP08







Cedar Barn, White Lodge, Walgrave, Northampton, NN6 9PY Tel: (01604) 781877 Fax: (01604) 781007 E-mail: mail@soiltechnics.net Section B - B' looking east





Photographic record of trial pit TP11

Notes.

 Trial pit excavated using hand tools
 Trial pit sides remained stable and no groundwater encountered.
 All dimensions shown in metres
 Steel bar used to estimate presence of foundation below 1.75m

Richmond-Upon-Thames College, Egerton Road, Twickenham.

Detail of foundation exposed in Trial pit TP11

scale	date	drawn by	checked by
1:25 @ A4	28.05.08	RC	
proje	ct ref	drg No.	revision
STE1297R		TP11	





DESCRIPTION		LEGEND DEPTH S (m) DE		INSITU SHEAR STRENGTH TEST DATA		
		()		DEPTH	TYPE	RESULT
Dark brown slightly gravelly SAND, with occasional root and rootlets. Gravel consists of flint. (TOPSOIL)	s	0.0 0.12	0.1m J			
Light brown gravelly SAND, with occasional roots. Grav consists of flint.	vel	0.42	0.3m J 0.4m J			
	/					
 TRIAL PIT TERMINATED AT 0.42m Notes: Trial pit sides were upright and stable. No groundwater encountered. Creosote odour between depths of 0.0-0.2m. 						
REFER TO KEY AT BEGINNING OF THIS A	APPENDIX FC		ANATION	OF SYME	BOLS	
GR	ROUND LEVEL		METHO H	od of excav	ATION	
	LOCATION PLAN ON DRAWING No STE1297R-02		DATE	DATE OF EXCAVATION 27.05.08		
SOILTECHNICS	oject Richmond-l Twickenhar	Jpon-Th n.	ames Colle	ege, Ege	erton Ro	oad,
UEUIELHNILAL ENGINEEKS, ENYIKUNMENIAL CONSULIANIS Cedar Barn, White Lodge, Walgrave, Northampton. NN6 9PY. Tel: (01604) 781877 Fax: (01604) 781007 E-mail: mail@soiltechnics.net	.net PROJECT REF. STE1297R			. PIT No TP1	4	

DESCRIPTION		LEGEND	DEPTH	SAMPLE		SHEAR S	TRENGTH
				DEPTH	TYPE	RESULT	
Grass onto dark brown slightly gravelly SAND, with occasional roots and rootlets. Gravel consists of flint brick. (MADE GROUND) Loose light brown silty gravelly SAND, with occasiona Gravel consists of flint grading into brown silty SAND. (KEMPTON PARK GRAVELS)	and al roots.		0.0 0.3 0.7	B 0.4-0.7r	n		
Notes: 1. Trial pit sides were upright and stable. 2. No groundwater encountered.							
REFER TO KEY AT BEGINNING OF THIS			R EXPL	ANATION	OF SYME	BOLS	
TRIAL PIT RE	CORD	PAGE 1	OF 1				
	GROUND LI	EVEL		MET	HOD OF EXCAN	ATION IS	
(\mathbf{S})	LOCATION PLAN ON DRAWING N₀ DATE OF EXCAVATION STE1297R-02 20.06.08						
SOILTECHNICAL ENGINEERS. ENVIRONMENTAL CONSULTANTS	PROJECT Rich Twie	nmond-L ckenhan	Jpon-Th n.	iames Col	lege, Ege	erton Ro	oad,
Cedar Barn, White Lodge, Walgrave, Northampton. NN6 9PY. Tel: (01604) 781877 Fax: (01604) 781007 E-mail: mail@soiltechnics.net	.net PROJECT REF. STE1297R			TR	AL PIT No TP1	5	

DESCRIPTION		LEGEND	DEPTH (m)	SAMPLE DEPTH TYP		SHEAR S	TRENGTH TA
			()		DEPTH	TYPE	RESULT
Grass onto dark brown slightly gravelly SAND, with occasional roots and rootlets. Gravel consists of flint brick. (MADE GROUND) Light brown silty gravelly SAND, with occasional roots	and s.		0.0 0.3	B 0.4-0.7			
(MADE GROUND)			0.7				
TRIAL PIT TERMINATED AT 0.7m Notes:							
 Trial pit sides were upright and stable. No groundwater encountered. 							
		NDIX FO		ANATION	OF SYME	BOLS	
	GROUND LI			METH	od of excav	ATION	
	LOCATION PLAN ON DRAWING No STE1297R-02			DATE	DATE OF EXCAVATION 20.06.08		
SOILTECHNICS	PROJECT Rich Twie	nmond-l ckenhan	Jpon-Th n.	ames Coll	ege, Ege	rton Ro	oad,
Cedar Barn, White Lodge, Walgrave, Northampton. NN6 9PY. Tel: (01604) 781877 Fax: (01604) 781007 E-mail: mail@soiltechnics.net	net PROJECT REF. STE1297R			TRIA	TRIAL PIT No TP16		

Brief soil description and strata for borehole DTS08:

DESCRI	PTION	LEGEND	DEPTH (m)
Grass o slightly (consists (MADE Loose b very gra (KEMPT	Into dark brown slightly clayey slightly silty gravelly SAND, with some rootlets. Gravel s of ash and flint. GROUND) Decoming medium dense light brown orange avelly SAND. Gravel consists of flint. TON PARK GRAVEL)	• 0 0 0 0 0 0 0 0 0 • 0 • 0 • 0	0.0
BOREH Notes:	IOLE TERMINATED AT 2.0m		
1.	Temporary slotted standpipe installed to 2.0m to maintain borehole stability.		
2.	Disturbed samples taken from 0.05-0.2m and 1.2-1.5m depths.		

Test observations:

Time (seconds)	Water level (from GL)	Head of water above ground water level (H)
0	0.48	0.61
60	0.67	0.42
120	0.79	0.3
180	0.87	0.22
240	0.92	0.17
300	0.95	0.14
360	0.96	0.13
480	0.98	0.11
600	1.02	0.07
900	1.02	0.07
1200	1.04	0.05
1800	1.07	0.02



Adopting the basic time lag method

k = permeability =
$$\frac{A}{FT}$$

$$F = \frac{2\pi L}{In\left[\frac{L}{D} + \sqrt{1 + \left(\frac{L}{D}\right)^2}\right]} = 1.5$$

Then

$$k = \frac{0.008}{1.5 \times 180} = 3.0 \times 10^{-5} \text{ ms}^{-1}$$

DETERMINATION OF PERMEABILITY OF Soils between 0.48 and 1.07m in borehole DTS08

Following BS 5930: 1999, (Section 25.4) and CIRIA special publication 25 'site investigation manual' (Variable Head Test)

Test 1

Permanent slotted casing installed between				0.5	m	-	2.0	m
Borehole depth - 2.00 m								
$ \begin{array}{rcl} L & = & le\\ D & = & d\\ A & = & c\\ G & = & G \end{array} $	length of borehole test area diameter of borehole cross sectional area Groundwater level			0.59 m 0.101 m 0.008 m ² 1.09 m				