



Civil Engineers & Transport Planners

Kingston
Bridge House

Drainage
Strategy

March 2022

201345/DS/JR/KBL/02



Civil Engineers & Transport Planners

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1 INTRODUCTION

1.1 Scope

1.1.1 Lanmor Consulting has been commissioned by Westcombe Group to prepare a Drainage Strategy for the proposed development at Kingston Bridge House, Church Road, Hampton Wick, KT1 4AG. This report has been prepared in support of redevelopment of the site and has been commissioned to advise on the feasibility of providing a solution for the foul and surface water drainage for the proposed development.

1.1.2 This report will consider the drainage regime for the site and sets out the drainage strategy for the development including discharge rates and any requirements for attenuation.

1.1.3 The information within this report will be refined, modified, and updated as the detailed design is progressed. The scope of the works for this drainage strategy report is outlined below:

- Review available data in relation to on-site drainage and other drainage networks near the site
- Review of the ground conditions for the suitability of Sustainable Drainage Systems (SuDS)
- Consider the use of SuDS as an option for disposal of surface water runoff from the proposed development
- An assessment of the run-off likely to be generated.
- Undertake drainage assessments to establish attenuation requirements to deal with any increase in surface water runoff from the development.

2 SITE LOCATION AND DESCRIPTION

2.1 Location

2.1.1 The site is located within the Borough of Richmond. The site is located at the junction of Church Grove and Hampton Court Road, opposite the Kings Field. The River Thames is located just east of the site, approximately 140m away. Figure 2.1 below shows the location of the site.

2.1.2 Kingston Bridge House is currently made up of student living facilities which spans over 7 floors. Drawings FLU.1191.3.03 – 09 in Appendix A show the plans for the existing development.

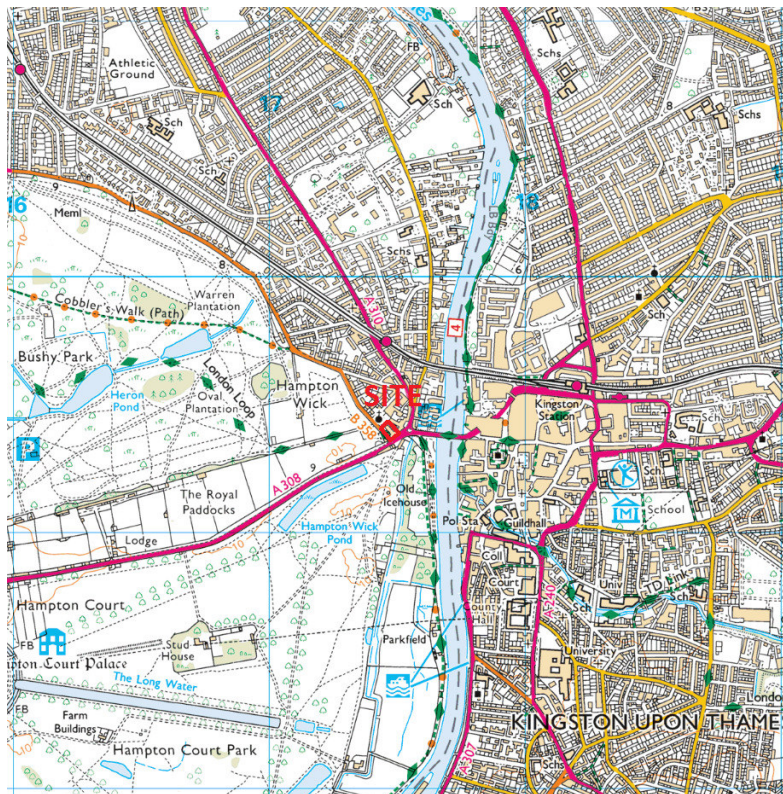


Figure 2.1 – Site Location

2.2 Existing Geology

2.2.1 The British Geological Survey indicates that the site has an underlying bedrock of London Clay Formation, which consists primarily of clay, silt and sand. Sedimentary bedrock formed between 56 and 47.8 million years ago during the Palaeogene period.

2.2.2 Superficial deposits have also been recorded at the site. The superficial geology consists Kempton Park Gravel Member, which is made up of sand and gravel. These deposits were formed between 1.6 and 1.8 million years ago during the Quaternary period.

2.3 Proposed Development

2.3.1 The proposed development will consist of the conversion of the existing student living accommodation to residential apartments a total of 70 units will be provided.

2.3.2 Drawings FLU.1191.3.11 – 17 included in Appendix B shows the proposed make-up of the development at Kingston Bridge House.

3 EXISTING DRAINAGE

3.1 Existing Foul Drainage

3.1.1 As part of the investigation, Thames Water asset mapping was requested. The records show that there is an existing foul sewer located beneath Church Grove flowing from northwest to southeast. The pipe is 175mm in diameter and flows to another network located beneath Hampton Court Road. The nearest manholes located to the site are 4301 and 4302. Unfortunately, Thames Water have not provided any level information for either of these manholes.

3.1.2 The next nearest manholes are 4306 and 4308, which are located to towards the south of the site at the junction between Hampton Court Road and Church Grove. Manhole 4305 has no information. Manhole 4308 has a cover level of 7.61m and an invert level of 5.48m.

3.2 Existing Surface Water Drainage

3.2.1 According to the sewer records, there is an existing surface water located to the southeast of the site beneath Hampton Court Road, flowing from the southwest to the northeast. The sewer is a 450mm diameter pipe with the nearest manhole to the site being 4306. Manhole 4306 has a cover level of 7.49m and an invert level of 5.18m.

3.2.2 The Thames Water records can be found in Appendix B of this report.

4 PROPOSED DRAINAGE REGIME

4.1 Proposed Foul Drainage

4.1.1 The proposed foul drainage will utilise the existing foul drainage pipe network on site. The existing building accommodates approximately 216 students and has the potential to generate up to 10 l/s foul flows.

4.1.2 The proposed development will include for 70 new residential units with up to 210 residents in the development. Based on Sewers for Adoption 0.046 l/s per dwelling the 70 residential units might generate 3.2 l/s.

4.1.3 The proposed discharge rate will be a reduction on the current facility and therefore there will be no capacity issues with the existing drainage network as it will be approximately 50% less.

4.2 Proposed Surface Water Drainage

4.2.1 The development proposals do not involve any extension of to the existing buildings. The existing site is largely hard surfaced as indicated in Figure 4.1 below.

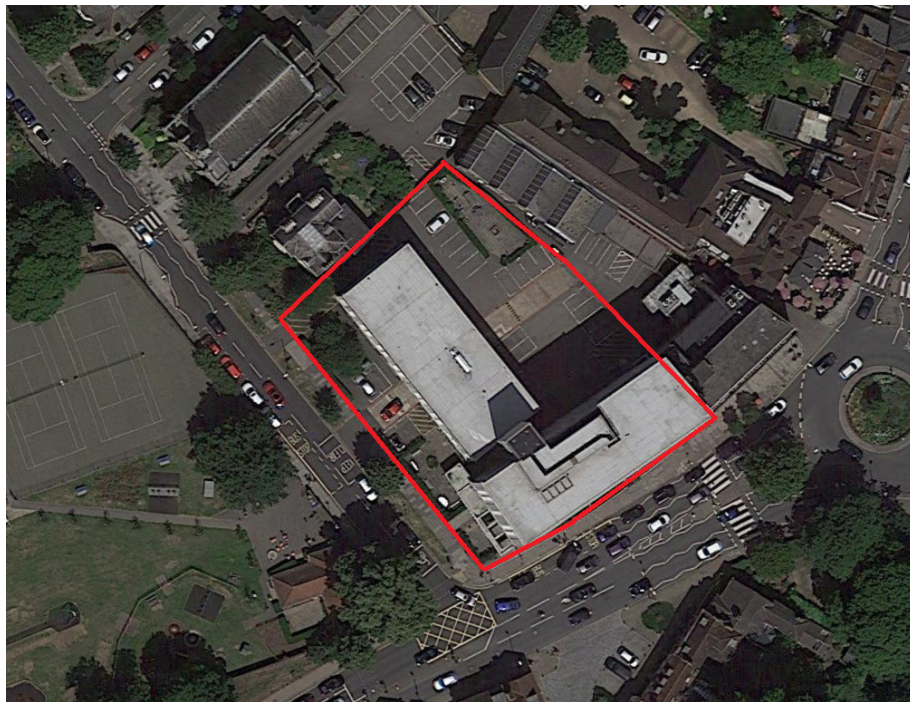


Figure 4.1 – Arrival View of Site

- 4.2.2 The proposed development will incorporate landscaped areas so the proposals will reduce the impermeable area on site. Sustainable Drainage Systems (SuDS) were considered as part of this assessment for disposing of the surface water run from the development. A copy of the proposed site plan is included in Appendix C as drawing FLU.1191.3.10. However, the building is already drained, and it is not possible to introduce any recycling of rainwater, or attenuation within the building.
- 4.2.3 Also, for rainwater harvesting to be affective the harvesting tank needs to be empty to receive the next storm. For these reasons, rainwater harvesting has been discounted.
- 4.2.4 Next on the sustainable drainage hierarchy is the use of ground infiltration techniques such as soakaways and infiltration basins. BGS records indicate the underlying bedrock to consist of London Clay. Therefore, since the ground conditions are not viable and the there is a lack of space on site, the use of infiltration techniques have been discounted.
- 4.2.5 The hierarchy suggest the next method of discharge is to a watercourse. The River Thames is the nearest but is located 140m away to the east of the site and is concluded to be too far from the site to discharge surface water run-off there. Thus it is not possible to adopt this method of discharge.
- 4.2.6 Therefore, in accordance with the SuDS hierarchy, surface water runoff will discharge to the existing surface water sewer, the proposed development will reduce the volume and rate of discharge from the development due to the introduction of soft landscaped areas. The existing drainage network for the building is already in place and connected to the existing Thames Water sewer, the drainage will therefore be utilised for the proposed development.
- 4.2.7 The drainage proforma is included in Appendix D and shows the proposed / existing drainage areas and discharge rates.

- 4.2.8 The existing parking area is currently impermeable, part of this will be landscape so will be permeable and the remainder will be replaced with a new hard surfaced car park. This gives the opportunity to incorporate permeable paving into the development which will further reduce the runoff for the site. The rear car park will therefore have permeable surface over a 300mm gravel layer with a restricted discharge to the on-site drainage system.
- 4.2.9 Drawing 201345/DS/01 included in appendix D shows the proposed SuDS features on site and the Microdrainage calculation are also included in Appendix D demonstrating the paving has been designed to accommodate a 1 in 100 year +40% storm event.

5 SURFACE WATER DRAINAGE MAINTENANCE

5.1 General

5.1.1 Regularly inspection of the surface water drainage network for blockages and clearing unwanted debris / silt from the system should improve the performance of the surface water network and decrease the need for future repairs. In the event of blockages, high pressure water jets can be used to clear the gullies and pipes to ensure they are functioning correctly, this should be undertaken by certified trained professionals.

5.1.2 The level and frequency of maintenance required on site is dependent on the type of facility. The type of maintenance will fall into one of three categories “regular maintenance”, “occasional maintenance” and “remedial maintenance”.

5.1.3 Regular maintenance of the drainage features will include, inspections, removal of litter / debris and sweeping of the surfaces. Occasional maintenance will include removal of sediment etc. and remedial maintenance may include structural repairs and infiltration reconditioning if required.

5.1.4 Following completion of the development a Management Company will be set up to maintain all the communal areas, including the drainage. It will be their responsibility to maintain the drainage networks.

5.2 Inspection, Manhole, Catchpit Chambers and Pipes

5.2.1 The appropriate health and safety equipment must be used when accessing manholes. Confined space certificates must be held by any personnel entering a manhole and the appropriate permits should be obtained.

5.2.2 Pipes are intended to be the main conveyance across the development. They are intended to be dry except for during rainfall events. These have been designed to be self-cleaning where possible for smaller diameter pipes, and for larger diameters the risk is reduced due to the overall pipe size.

5.2.3 For the Inspection, Manhole, Catchpit Chambers and Pipes, the following maintenance will be required.

Manhole / Pipe Maintenance Schedule		
	Required Action	Typical Frequency
Regular maintenance	Inspect for evidence of poor operation via water level in chambers. If required, take remedial action.	3-monthly, 48 hours after large storms.
	Check and remove large vegetation growth near pipe runs.	Monthly or as required
	Remove sediment from structures.	Annually or as required
Remedial Actions	Rod through poorly performing runs as initial remediation.	As required
	If continued poor performance jet and CCTV survey poorly performing runs.	As required
Monitoring	Inspect/check all inlets, outlets, to ensure that they are in good condition and operating as designed.	Annually
	Survey inside of pipe manholes for sediment build-up and remove if necessary	Every 5 years or as required

Table 5.1 – Manhole, Catchpit and Pipes Maintenance

5.3 Drainage Channels and Gullies

5.3.1 For the Inspection, drainage channel and gullies, the following maintenance will be required.

Channel and Gully Maintenance Schedule		
	Required Action	Typical Frequency
Regular maintenance	Litter and debris removal	Monthly or as required
	Check and remove large vegetation growth near channel runs	Monthly or as required
	Inspect for evidence of poor operation and/or weed growth. If required, take remedial action. Inspect silt accumulation rates and establish appropriate brushing frequencies. Silt can also be caused by adjacent landscaping areas which should be reprofiled to provide a flat area or berm adjacent to the paving	3-monthly, 48 hours after large storms
Remedial Actions	Inspect access/outlet boxes and rod through poorly performing channels and outlets as initial remediation.	As required
Monitoring	Inspect/check all inlets, outlets, to ensure that they are in good condition and operating as designed.	Annually
	Survey inside of gullies for sediment build-up and remove if necessary	Every year or as required

Table 5.2 – Channel and Gully Maintenance

Permeable Paving

5.3.2 For permeable paving areas, the following maintenance is recommended.

Permeable Paving Maintenance Schedule		
	Required Action	Typical Frequency
Regular maintenance	Remove debris and leaves etc.	Once a year, after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturer's recommendations – pay particular attention to areas where water runs onto pervious surfaces from adjacent impermeable areas as this area is most likely to collect the most sediment.
Occasional maintenance	Stabilise and mow contributing and adjacent areas	As required
	Removal of weeds	As required- once per year on less frequently used pavements
Remedial Actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50 mm of the level of the paving	As required
	Remedial work to any depressions, rutting etc	As required
	Rehabilitation of surface and upper substructure	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)
Monitoring	Inspect for evidence of poor operation and/or weed growth - if required, take remedial action.	Three-monthly, 48 hours after large storms in the first six months
	Inspect silt accumulation rates and establish appropriate frequencies for rehabilitation	Annually
	Monitor inspection chambers	Annually

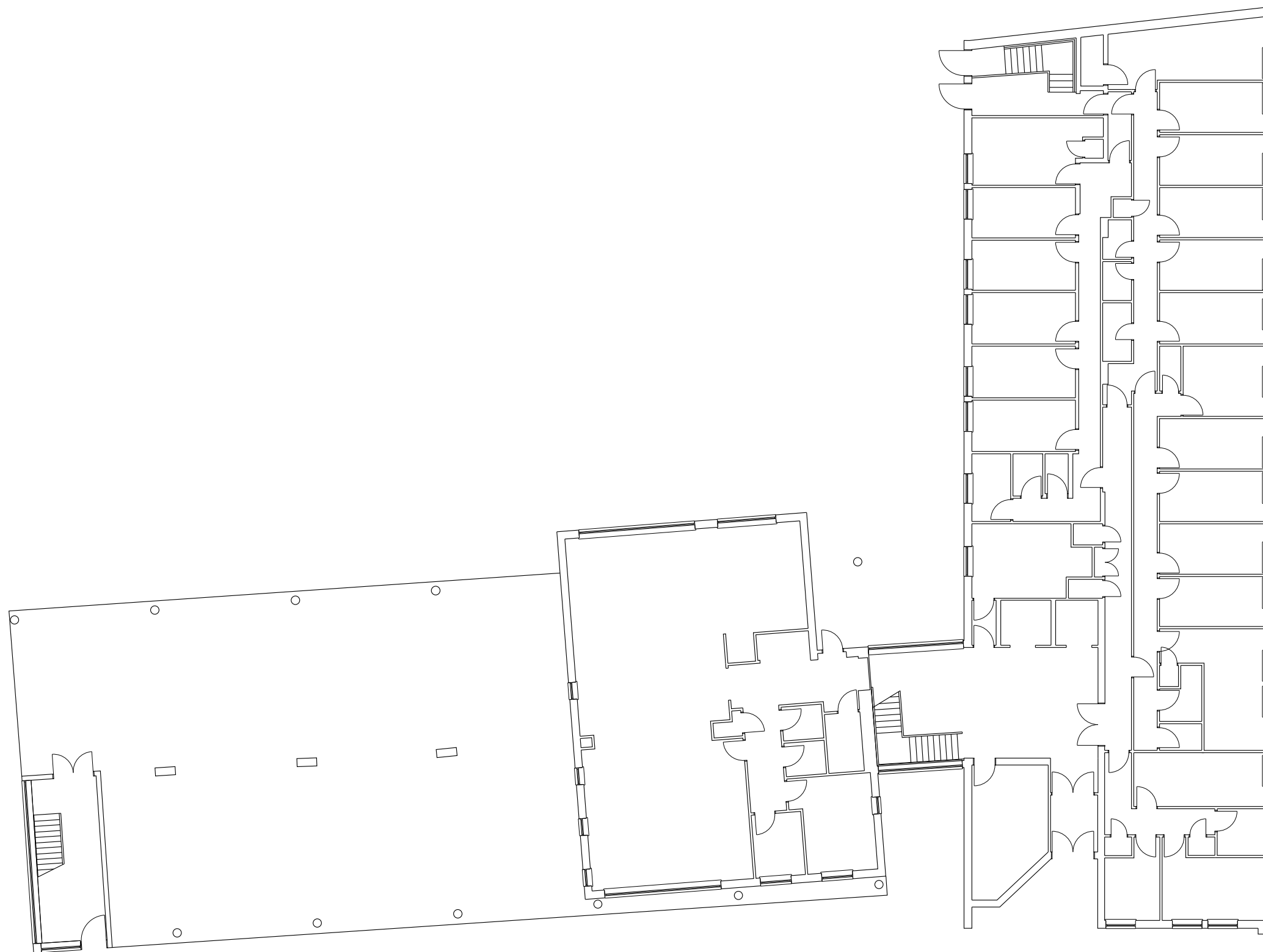
Table 5.3 – Permeable Paving Maintenance Schedule

6 SUMMARY AND CONCLUSION

- 6.1.1 The proposals seek permission to convert the existing building to provide 89 residential units including additional floors on the existing building. The majority of the site is currently hard surfaced, and the building is positively drained to the public surface water.
- 6.1.2 The proposed drainage strategy is to reuse the existing drainage network, for the proposed conversion. Additional soft landscape area will be provided in the existing hard surfaced parking areas which will result in a reduction in the volume and rate of discharge leaving the site.
- 6.1.3 The foul sewage currently serves 216 students, the proposed 89 apartments will generate 50% of the current discharge so there will no issue with the capacity for the proposed development.
- 6.1.4 The proposed development will result in a reduction in the discharge of both foul and surface water discharges from the development. For the reasons set out above, the proposed development is considered suitable for the development, as there will be no negative impacts on the public sewers or result in increased flood risk in the area.

APPENDIX A

Drawings FLU.1191.3.03 – 09 – Existing Floor Plans



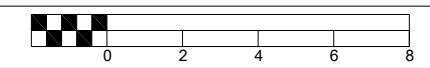
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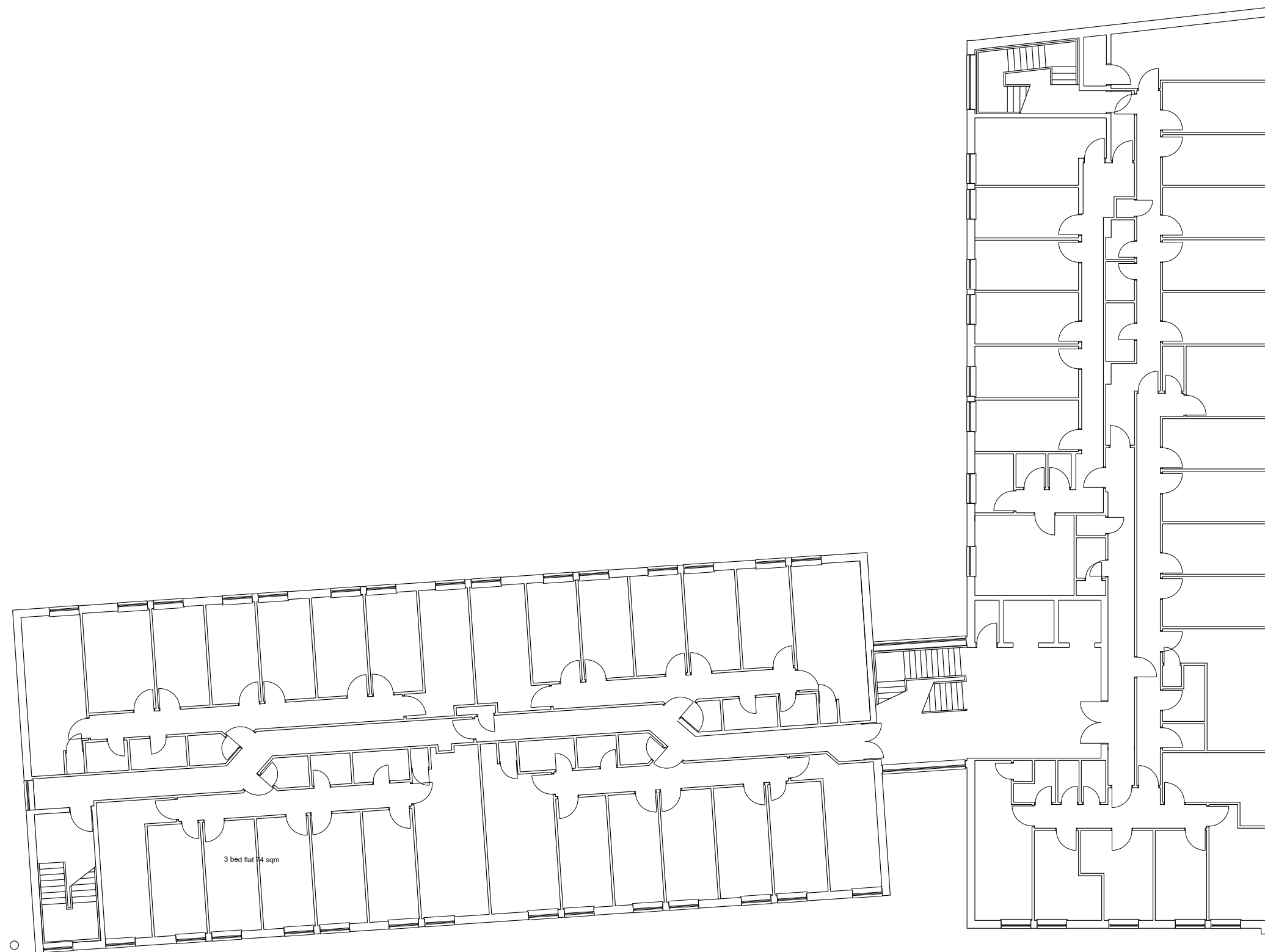
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Kingston Bridge House
Church Grove, Hampton Wick

Existing Ground Floor Plan



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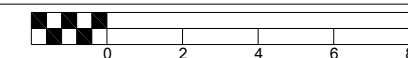


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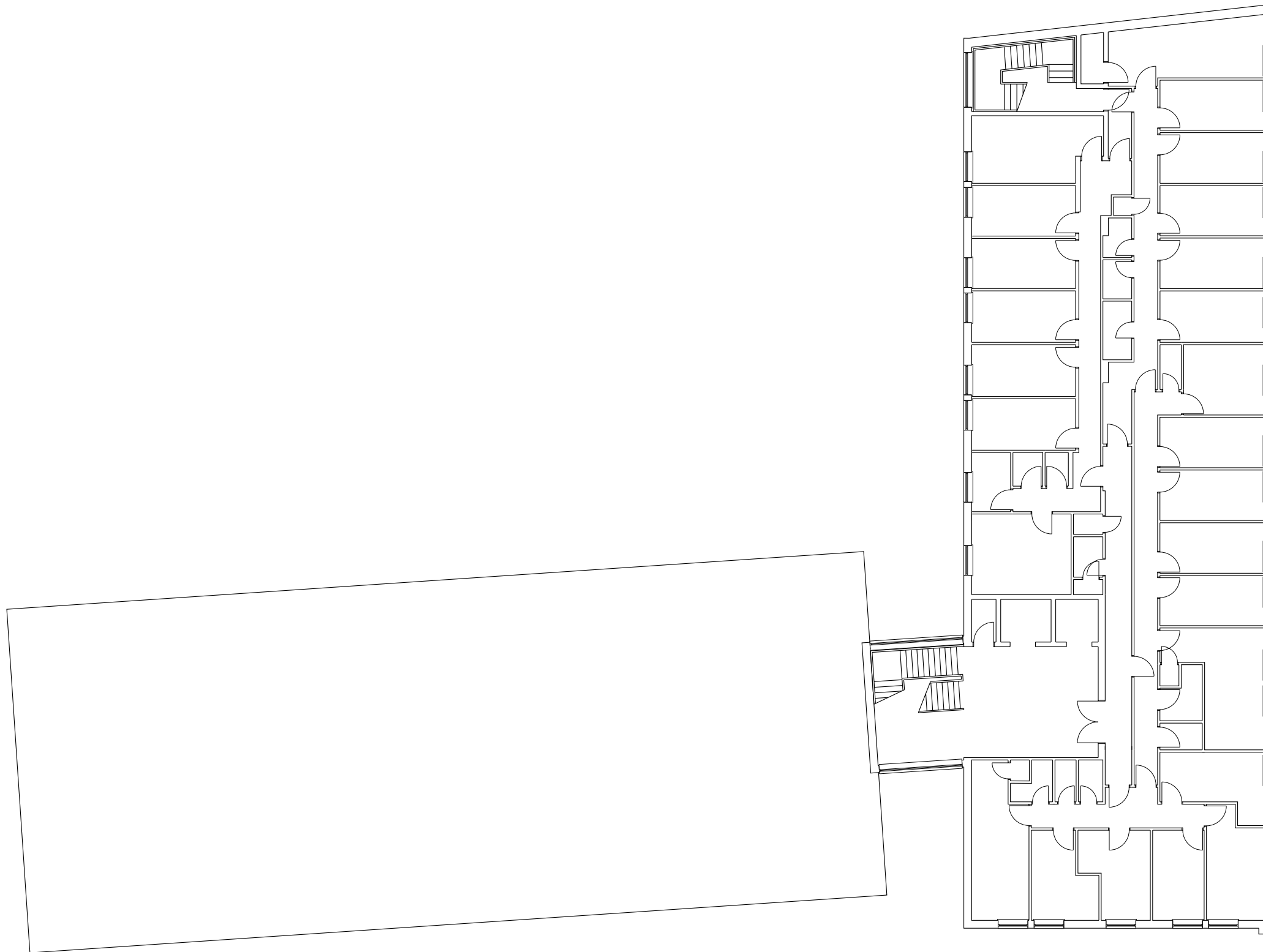
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Existing 1st, 2nd & 3rd Floor Plan



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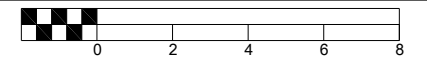


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Existing 4th, 5th & 6th Floor Plan



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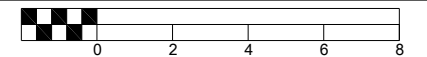
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Existing Front Elevation



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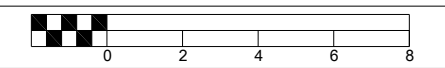


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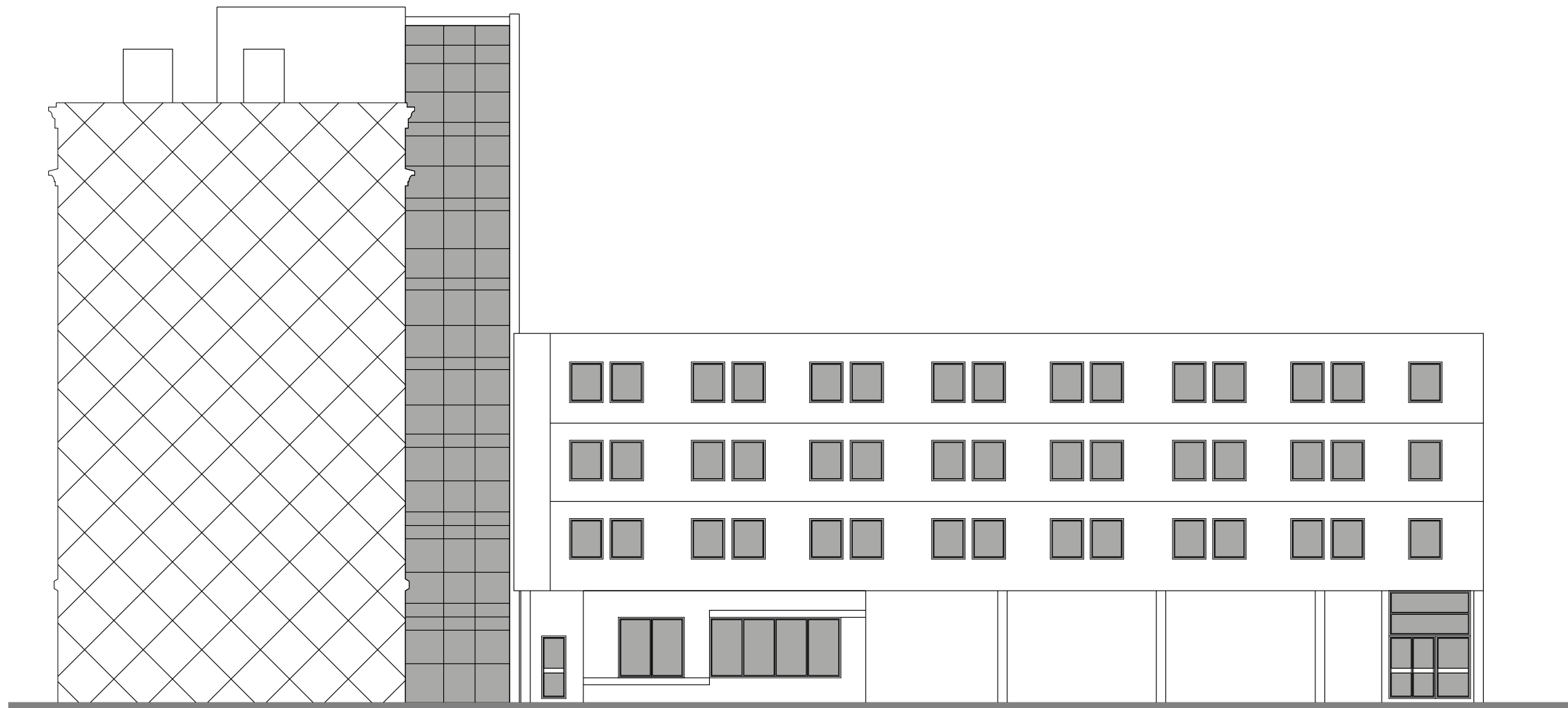
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Existing Side Elevation



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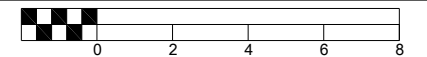


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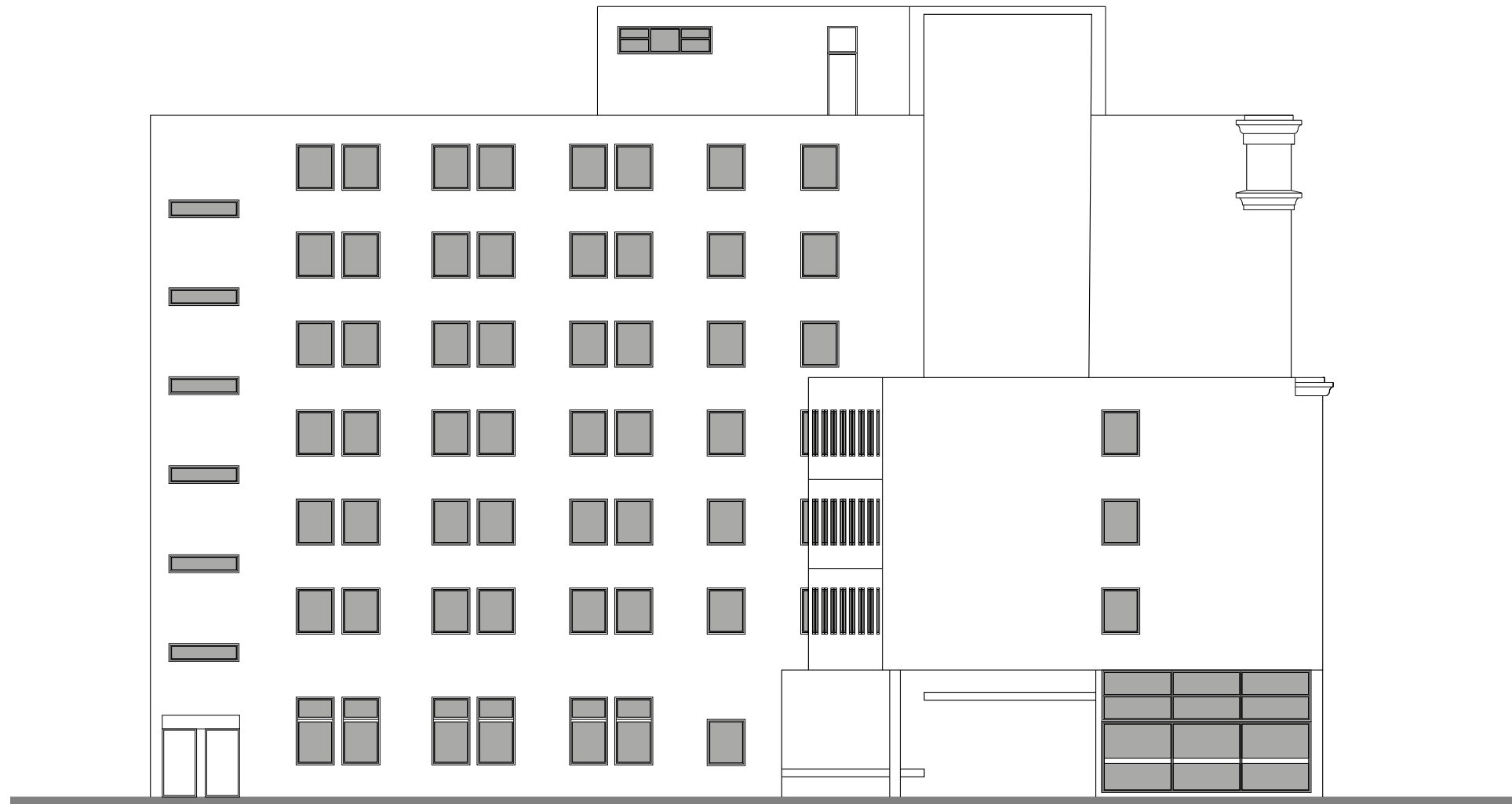
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Existing Rear Elevation



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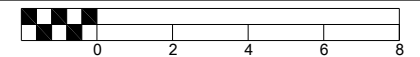


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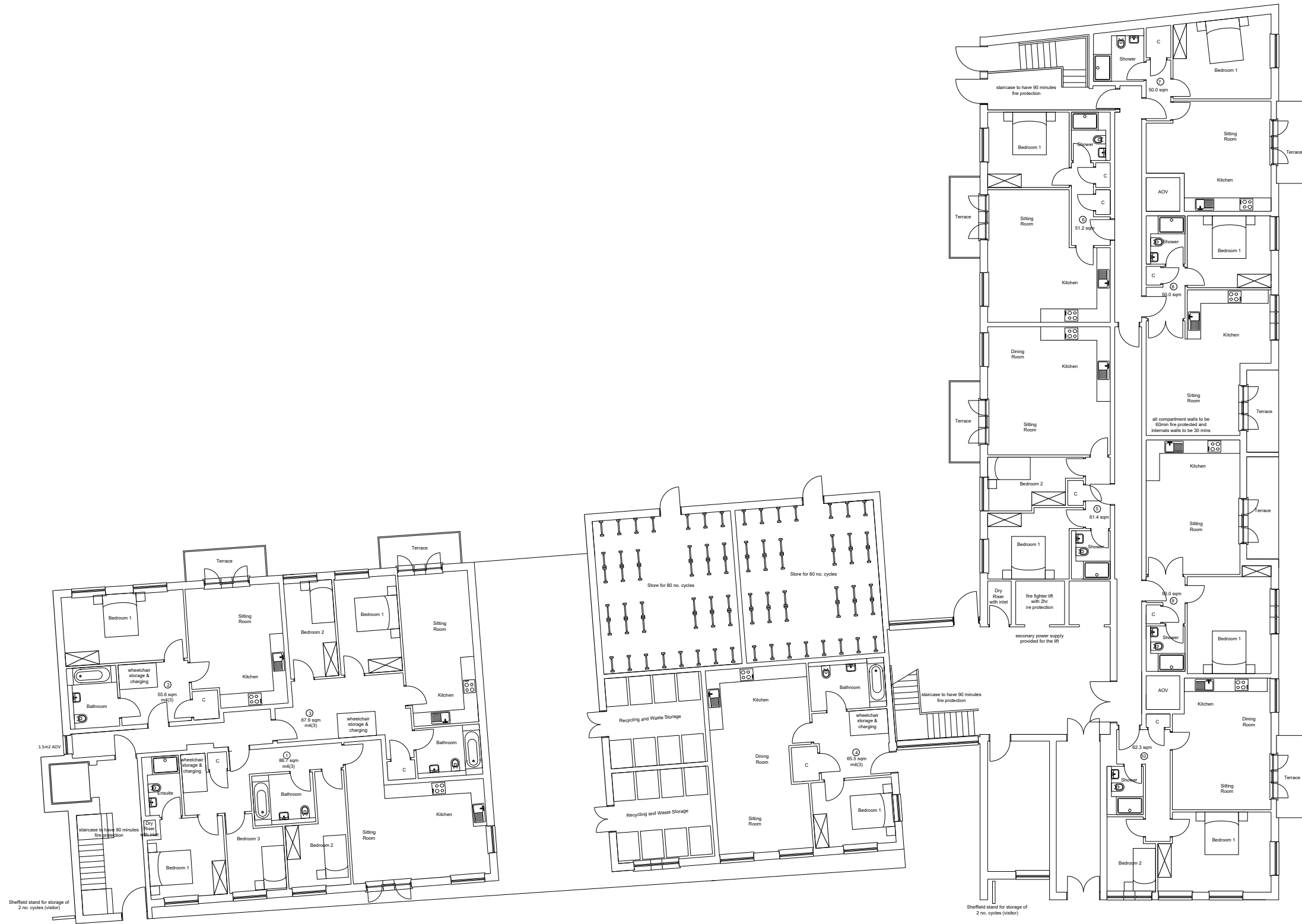
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Existing Side Elevation



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Drawings FLU.1191.3.11 – 17 – Proposed Floor Plans



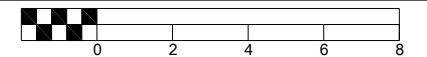
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Proposed Ground Floor Plan



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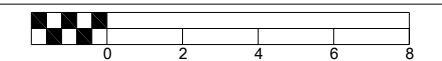
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Proposed First Floor Plan



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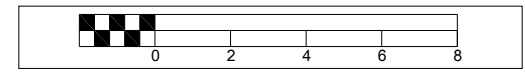
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Kingston Bridge House
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Proposed Second Floor Plan



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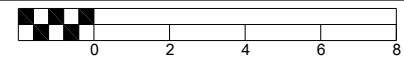
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Kingston Bridge House
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Proposed Third Floor Plan



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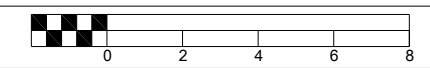
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Kingston Bridge House
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Proposed Fourth Floor Plan



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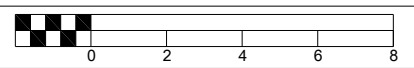
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Kingston Bridge House
Church Grove, Hampton Wick

Proposed Fifth Floor Plan



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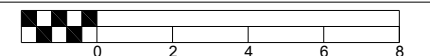


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Kingston Bridge House
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Proposed Sixth Floor Plan

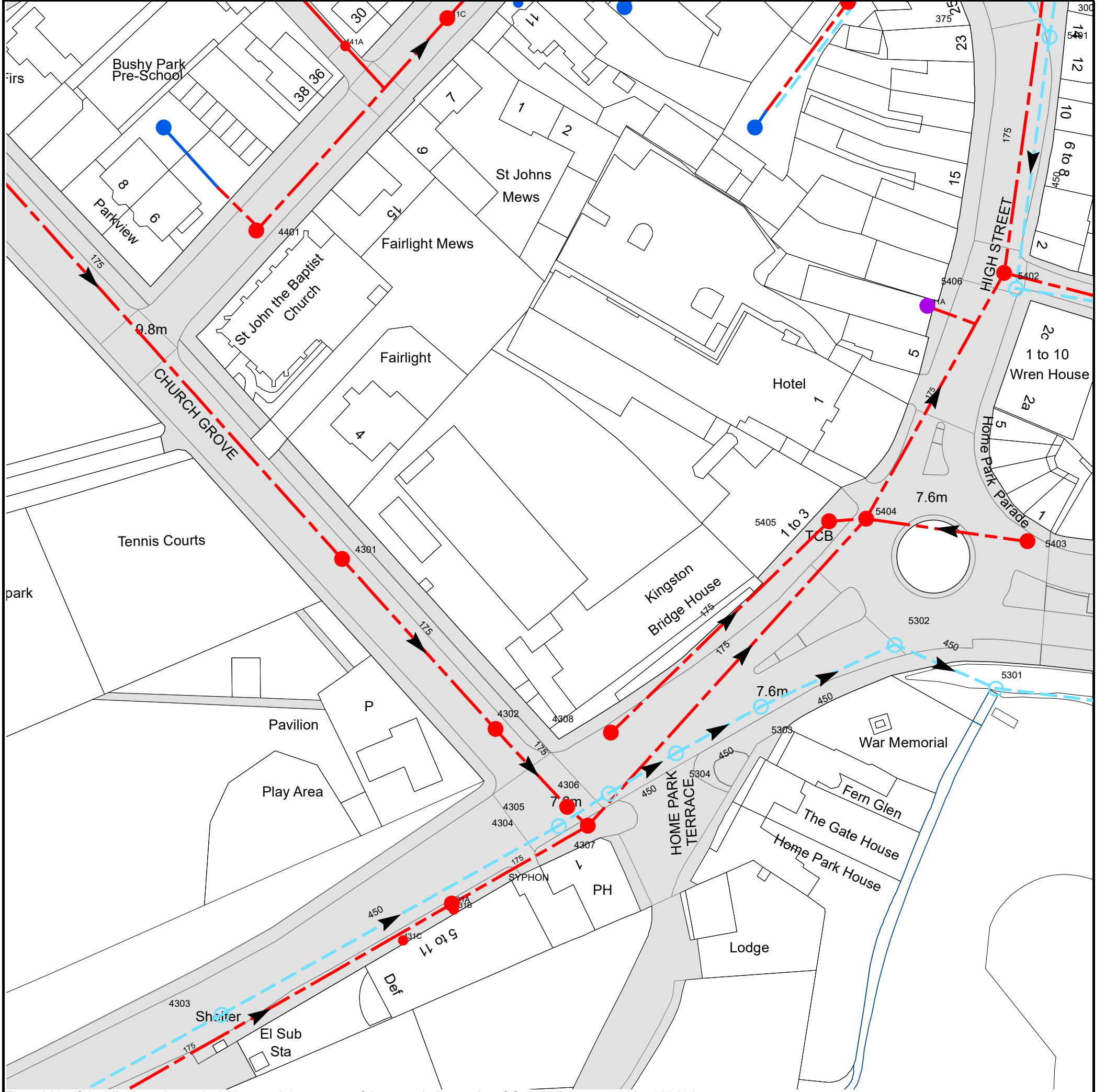


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APPENDIX B

Thames Water Record Drawings

Asset Location Search Sewer Map - ALS/ALS Standard/2020 4287326



The width of the displayed area is 200 m and the centre of the map is located at OS coordinates 517487,169400

The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.

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NB. Levels quoted in metres Ordnance Newlyn Datum. The value -9999.00 indicates that no survey information is available



















Manhole Reference	Manhole Cover Level	Manhole Invert Level
441C	n/a	n/a
541A	n/a	n/a
4301	n/a	n/a
5403	n/a	n/a
5405	n/a	n/a
5404	n/a	n/a
5402	n/a	n/a
5406	n/a	n/a
4401	n/a	n/a
44ZY	n/a	n/a
54ZT	n/a	n/a
441A	n/a	n/a
5401	n/a	n/a
54ZY	n/a	n/a
441B	n/a	n/a
54ZR	n/a	n/a
4303	7.23	5.39
431C	n/a	n/a
431B	n/a	n/a
431A	n/a	n/a
4307	7.55	5.43
4304	7.46	5.19
4305	n/a	n/a
4306	7.49	5.18
5304	7.53	5.11
4308	7.61	5.48
4302	n/a	n/a
5303	n/a	n/a
5301	n/a	n/a
5302	n/a	n/a

The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.








ALS Sewer Map Key

Public Sewer Types (Operated & Maintained by Thames Water)

-  **Foul:** A sewer designed to convey waste water from domestic and industrial sources to a treatment works.
-  **Surface Water:** A sewer designed to convey surface water (e.g. rain water from roofs, yards and car parks) to rivers or watercourses.
-  **Combined:** A sewer designed to convey both waste water and surface water from domestic and industrial sources to a treatment works.
-  **Trunk Surface Water**
-  **Trunk Foul**
-  **Storm Relief**
-  **Trunk Combined**
-  **Vent Pipe**
-  **Bio-solids (Sludge)**
-  **Proposed Thames Surface Water Sewer**
-  **Proposed Thames Water Foul Sewer**
-  **Gallery**
-  **Foul Rising Main**
-  **Surface Water Rising Main**
-  **Combined Rising Main**
-  **Sludge Rising Main**
-  **Proposed Thames Water Rising Main**
-  **Vacuum**





Sewer Fittings

A feature in a sewer that does not affect the flow in the pipe. Example: a vent is a fitting as the function of a vent is to release excess gas.

-  Air Valve
-  Dam Chase
-  Fitting
-  Meter
-  Vent Column




Operational Controls

A feature in a sewer that changes or diverts the flow in the sewer. Example: A hydrobrake limits the flow passing downstream.

-  Control Valve
-  Drop Pipe
-  Ancillary
-  Weir






End Items

End symbols appear at the start or end of a sewer pipe. Examples: an Undefined End at the start of a sewer indicates that Thames Water has no knowledge of the position of the sewer upstream of that symbol, Outfall on a surface water sewer indicates that the pipe discharges into a stream or river.

-  Outfall
-  Undefined End
-  Inlet






Other Symbols

Symbols used on maps which do not fall under other general categories








-  /  Public/Private Pumping Station
-  Change of characteristic indicator (C.O.C.I.)
-  Invert Level
-  Summit

Areas

Lines denoting areas of underground surveys, etc.

-  Agreement
-  Operational Site
-  Chamber
-  Tunnel
-  Conduit Bridge

Other Sewer Types (Not Operated or Maintained by Thames Water)

-  Foul Sewer
-  Surface Water Sewer
-  Combined Sewer
-  Gully
-  Culverted Watercourse
-  Proposed
-  Abandoned Sewer

Notes:

- 1) All levels associated with the plans are to Ordnance Datum Newlyn.
- 2) All measurements on the plans are metric.
- 3) Arrows (on gravity fed sewers) or flecks (on rising mains) indicate direction of flow.
- 4) Most private pipes are not shown on our plans, as in the past, this information has not been recorded.
- 5) 'na' or '0' on a manhole level indicates that data is unavailable.
- 6) The text appearing alongside a sewer line indicates the internal diameter of the pipe in millimetres. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology present on the plan, please contact a member of Property Insight on 0845 070 9148.

APPENDIX C

Drawings FLU.1191.3.10 – Proposed Site Layout

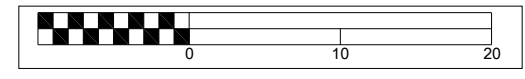


Rev	Date	Description

FLUENT
ARCHITECTURAL DESIGN SERVICES
69-71 WINDMILL ROAD, SUNBURY,
MIDDLESEX, TW16 7DT
TEL: 0800 0438838
E-MAIL: INFO@FLUENT-ADS.CO.UK
WEB: FLUENT-ADS.CO.UK

Kingston Bridge House
Church Grove, Hampton Wick

Proposed Site Plan

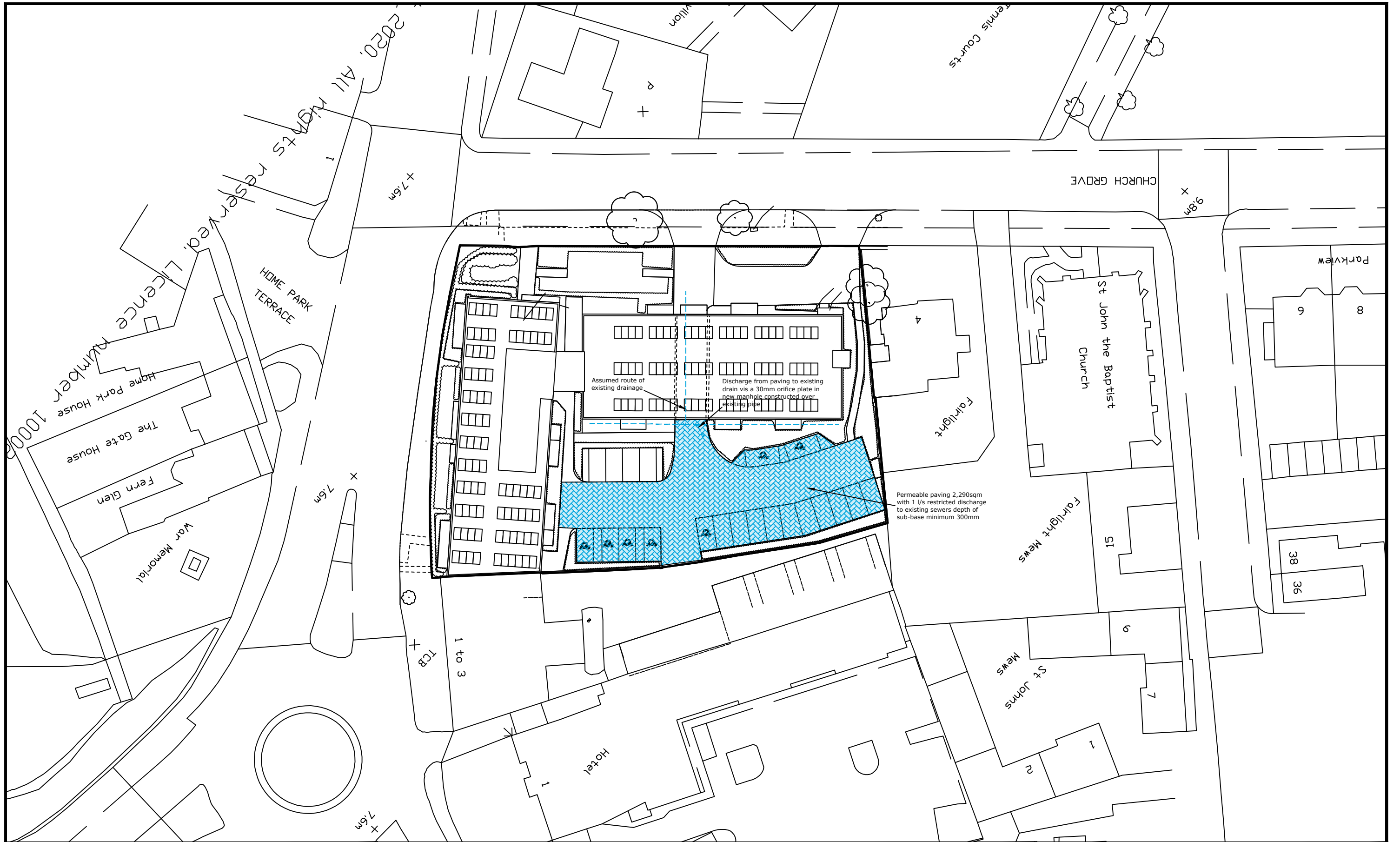


Scale 1:500 @ A3	Dwg No. FLU.1191.3.10
Date 07.10.20	Rev H
Drawn N.Millin	

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APPENDIX D

Drawing 201345/DS/01 – Propsoed SuDS layout



Westcombe
Group

Kingston Bridge House
Hampton Wick

Drainage
Strategy

LANMOR Consulting
Civil Engineers & Transport Planning

Thorogood House, 34 Tolworth Close, Surbiton, Surrey, KT6 7EW
Telephone: 0208 339 7899 Fax: 0208 339 7898
E-mail: info@lanmor.co.uk
www.lanmor.co.uk

SCALE 1:200

DRAWN BY MK

PRJ No. 201345

DWG No. 201345/DS/01

Microdrainage Calculations

Summary of Results for 100 year Return Period (+40%)

Half Drain Time : 1382 minutes.

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max Σ Outflow (l/s)	Max Volume (m ³)	Status
15 min Summer	99.607	0.157	0.0	0.7	0.7	47.9	O K
30 min Summer	99.634	0.184	0.0	0.8	0.8	65.8	O K
60 min Summer	99.660	0.210	0.0	0.8	0.8	83.8	O K
120 min Summer	99.685	0.235	0.0	0.9	0.9	101.2	O K
180 min Summer	99.699	0.249	0.0	0.9	0.9	110.3	O K
240 min Summer	99.707	0.257	0.0	0.9	0.9	116.0	Flood Risk
360 min Summer	99.717	0.267	0.0	0.9	0.9	122.8	Flood Risk
480 min Summer	99.723	0.273	0.0	1.0	1.0	126.8	Flood Risk
600 min Summer	99.726	0.276	0.0	1.0	1.0	128.9	Flood Risk
720 min Summer	99.727	0.277	0.0	1.0	1.0	129.8	Flood Risk
960 min Summer	99.727	0.277	0.0	1.0	1.0	129.5	Flood Risk
1440 min Summer	99.723	0.273	0.0	1.0	1.0	127.1	Flood Risk
2160 min Summer	99.716	0.266	0.0	0.9	0.9	122.4	Flood Risk
2880 min Summer	99.709	0.259	0.0	0.9	0.9	117.0	Flood Risk
4320 min Summer	99.692	0.242	0.0	0.9	0.9	105.8	O K
5760 min Summer	99.677	0.227	0.0	0.9	0.9	95.3	O K
7200 min Summer	99.663	0.213	0.0	0.8	0.8	85.8	O K
8640 min Summer	99.651	0.201	0.0	0.8	0.8	77.3	O K
10080 min Summer	99.639	0.189	0.0	0.8	0.8	69.6	O K
15 min Winter	99.618	0.168	0.0	0.7	0.7	55.1	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Time-Peak (mins)
15 min Summer	139.469	0.0	46.1	19
30 min Summer	91.145	0.0	55.0	34
60 min Summer	56.713	0.0	85.7	64
120 min Summer	34.093	0.0	104.6	124
180 min Summer	24.982	0.0	114.0	184
240 min Summer	19.920	0.0	118.8	242
360 min Summer	14.430	0.0	124.0	362
480 min Summer	11.481	0.0	126.8	482
600 min Summer	9.608	0.0	128.3	602
720 min Summer	8.303	0.0	129.1	720
960 min Summer	6.590	0.0	129.1	936
1440 min Summer	4.752	0.0	125.7	1154
2160 min Summer	3.421	0.0	189.8	1536
2880 min Summer	2.707	0.0	197.9	1956
4320 min Summer	1.944	0.0	199.3	2768
5760 min Summer	1.535	0.0	214.2	3576
7200 min Summer	1.278	0.0	217.6	4392
8640 min Summer	1.099	0.0	219.3	5184
10080 min Summer	0.968	0.0	219.8	5944
15 min Winter	139.469	0.0	50.2	19

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (1/s)	Max Control (1/s)	Max Σ Outflow (1/s)	Max Volume (m ³)	Status
30 min Winter	99.647	0.197	0.0	0.8	0.8	75.1	O K
60 min Winter	99.677	0.227	0.0	0.9	0.9	95.4	O K
120 min Winter	99.706	0.256	0.0	0.9	0.9	115.0	Flood Risk
180 min Winter	99.721	0.271	0.0	1.0	1.0	125.5	Flood Risk
240 min Winter	99.730	0.280	0.0	1.0	1.0	132.0	Flood Risk
360 min Winter	99.742	0.292	0.0	1.0	1.0	140.1	Flood Risk
480 min Winter	99.749	0.299	0.0	1.0	1.0	144.9	Flood Risk
600 min Winter	99.753	0.303	0.0	1.0	1.0	147.7	Flood Risk
720 min Winter	99.756	0.306	0.0	1.0	1.0	149.3	Flood Risk
960 min Winter	99.757	0.307	0.0	1.0	1.0	150.0	Flood Risk
1440 min Winter	99.752	0.302	0.0	1.0	1.0	146.6	Flood Risk
2160 min Winter	99.742	0.292	0.0	1.0	1.0	140.3	Flood Risk
2880 min Winter	99.732	0.282	0.0	1.0	1.0	132.9	Flood Risk
4320 min Winter	99.708	0.258	0.0	0.9	0.9	117.0	Flood Risk
5760 min Winter	99.686	0.236	0.0	0.9	0.9	101.9	O K
7200 min Winter	99.667	0.217	0.0	0.8	0.8	88.4	O K
8640 min Winter	99.649	0.199	0.0	0.8	0.8	76.4	O K
10080 min Winter	99.634	0.184	0.0	0.8	0.8	65.9	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Time-Peak (mins)
30 min Winter	91.145	0.0	58.5	33
60 min Winter	56.713	0.0	97.2	64
120 min Winter	34.093	0.0	116.4	122
180 min Winter	24.982	0.0	123.9	180
240 min Winter	19.920	0.0	128.0	240
360 min Winter	14.430	0.0	132.8	356
480 min Winter	11.481	0.0	135.4	472
600 min Winter	9.608	0.0	136.8	586
720 min Winter	8.303	0.0	137.5	700
960 min Winter	6.590	0.0	137.4	922
1440 min Winter	4.752	0.0	133.9	1324
2160 min Winter	3.421	0.0	215.2	1644
2880 min Winter	2.707	0.0	223.8	2104
4320 min Winter	1.944	0.0	217.7	2984
5760 min Winter	1.535	0.0	245.0	3864
7200 min Winter	1.278	0.0	249.7	4680
8640 min Winter	1.099	0.0	252.6	5448
10080 min Winter	0.968	0.0	254.3	6248

Thorogood House
 34 Tolworth Close
 Surbition Surrey KT6 7EW



Date 22/08/2022 13:49
 File

Designed by Kunal
 Checked by

XP Solutions

Source Control 2015.1

Rainfall Details

Rainfall Model	FSR	Winter Storms	Yes
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	20.000	Shortest Storm (mins)	15
Ratio R	0.411	Longest Storm (mins)	10080
Summer Storms	Yes	Climate Change %	+40

Time Area Diagram

Total Area (ha) 0.229

Time (mins) Area
From: To: (ha)

0 4 0.229

Thorogood House
34 Tolworth Close
Surbition Surrey KT6 7EW



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Checked by

XP Solutions Source Control 2015.1

Model Details

Storage is Online Cover Level (m) 100.000

Porous Car Park Structure

Infiltration Coefficient Base (m/hr) 0.00000	Width (m) 26.0
Membrane Percolation (mm/hr) 1000	Length (m) 88.0
Max Percolation (l/s) 635.6	Slope (1:X) 500.0
Safety Factor 2.0	Depression Storage (mm) 5
Porosity 0.30	Evaporation (mm/day) 3
Invert Level (m) 99.450	Cap Volume Depth (m) 0.300

Orifice Outflow Control

Diameter (m) 0.030 Discharge Coefficient 0.600 Invert Level (m) 99.450

Thorogood House
34 Tolworth Close
Surbition Surrey KT6 7EW

Date 22/08/2022 13:49
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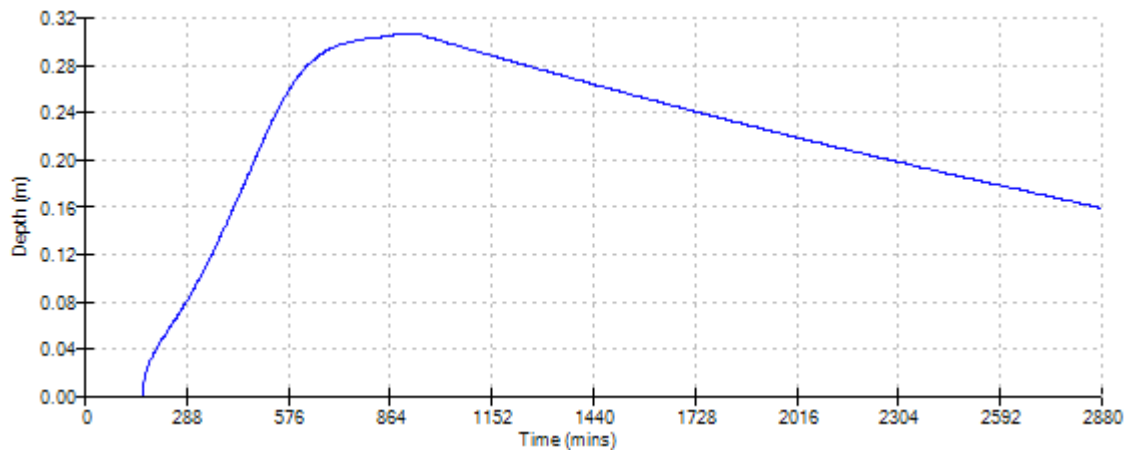
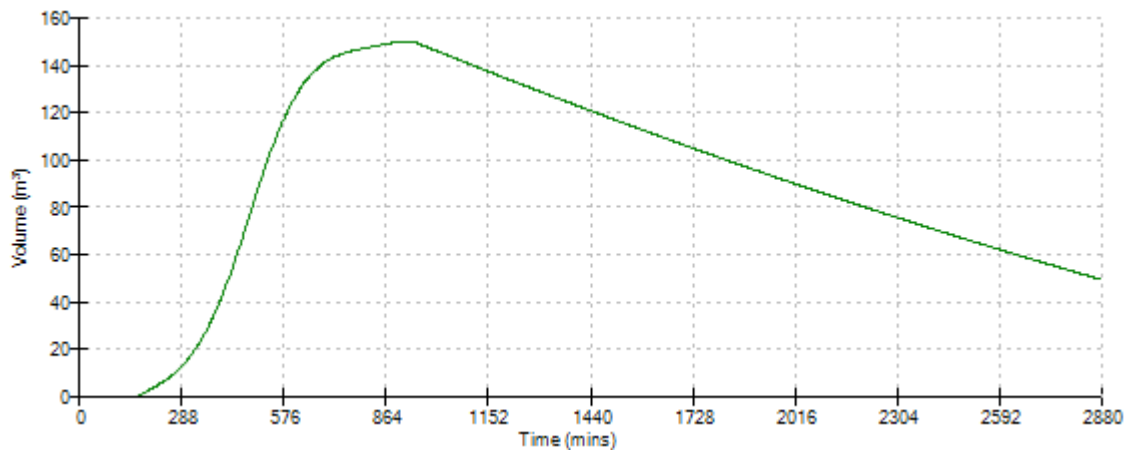
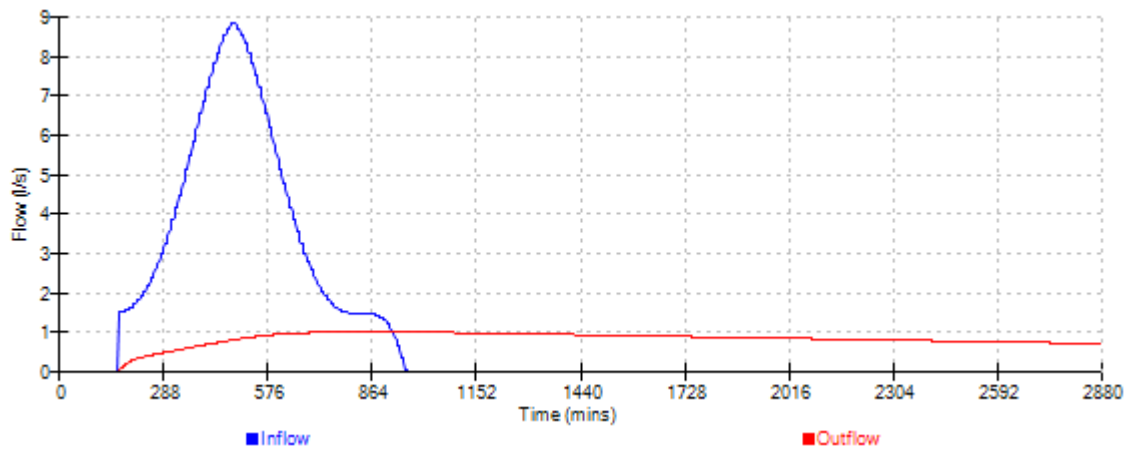
Designed by Kunal
Checked by



XP Solutions

Source Control 2015.1

Event: 960 min Winter



Drainage Proforma

1. Project & Site Details	Project / Site Name (including sub-catchment / stage / phase where appropriate)	Kingston Bridge House
	Address & post code	at Kingston Bridge house, Church Road, Hampton Wick, KT1 4AG
	OS Grid ref. (Easting, Northing)	E 517487 N 169400
	LPA reference (if applicable)	
	Brief description of proposed work	Conversion of existng building from student accomodation to residential apartments
	Total site Area	2,780 m ²
	Total existing impervious area	2,550 m ²
	Total proposed impervious area	2,200 m ²
	Is the site in a surface water flood risk catchment (ref. local Surface Water Management Plan)?	no
	Existing drainage connection type and location	Sewer
	Designer Name	
	Designer Position	
Designer Company		

2. Proposed Discharge Arrangements	2a. Infiltration Feasibility		
	Superficial geology classification	Kempton Park Gravels	
	Bedrock geology classification	London Clay	
	Site infiltration rate	m/s	
	Depth to groundwater level	m below ground level	
	Is infiltration feasible?		
	2b. Drainage Hierarchy		
		<i>Feasible (Y/N)</i>	<i>Proposed (Y/N)</i>
	1 store rainwater for later use	N	N
	2 use infiltration techniques, such as porous surfaces in non-clay areas	Y	Y
	3 attenuate rainwater in ponds or open water features for gradual release	N	N
	4 attenuate rainwater by storing in tanks or sealed water features for gradual release	N	N
	5 discharge rainwater direct to a watercourse	N	N
	6 discharge rainwater to a surface water sewer/drain	Y	Y
7 discharge rainwater to the combined sewer.	N	N	
2c. Proposed Discharge Details			
Proposed discharge location	Existing conection to sewer		
Has the owner/regulator of the discharge location been consulted?	No		

3a. Discharge Rates & Required Storage				
	Greenfield (GF) runoff rate (l/s)	Existing discharge rate (l/s)	Required storage for GF rate (m ³)	Proposed discharge rate (l/s)
Q _{bar}	n/a	 	 	
1 in 1	n/a	n/a	n/a	n/a
1 in 30	n/a	n/a	n/a	n/a
1 in 100	n/a	n/a	n/a	n/a
1 in 100 + CC	 	 	n/a	n/a
Climate change allowance used		40%		
3b. Principal Method of Flow Control				
3c. Proposed SuDS Measures				
	Catchment area (m ²)	Plan area (m ²)	Storage vol. (m ³)	
Rainwater harvesting	0	 	0	
Infiltration systems	0	 	0	
Green roofs	0	0	0	
Blue roofs	0	0	0	
Filter strips	0	0	0	
Filter drains	0	0	0	
Bioretention / tree pits	0	0	0	
Pervious pavements	2290	2290	200	
Swales	0	0	0	
Basins/ponds	0	0	0	
Attenuation tanks	0	 	0	
Total	2290	2290	200	

4a. Discharge & Drainage Strategy	Page/section of drainage report
Infiltration feasibility (2a) – geotechnical factual and interpretive reports, including infiltration results	section 2.2
Drainage hierarchy (2b)	section 4.2
Proposed discharge details (2c) – utility plans, correspondence / approval from owner/regulator of discharge location	Appendix B
Discharge rates & storage (3a) – detailed hydrologic and hydraulic calculations	section 4
Proposed SuDS measures & specifications (3b)	section 4.2
4b. Other Supporting Details	Page/section of drainage report
Detailed Development Layout	Appendix C
Detailed drainage design drawings, including exceedance flow routes	n/a
Detailed landscaping plans	Appendix C
Maintenance strategy	section 5
Demonstration of how the proposed SuDS measures improve:	n/a
a) water quality of the runoff?	
b) biodiversity?	
c) amenity?	