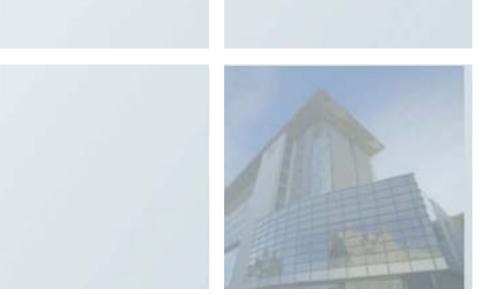
APPENDIX C GROUND CONDITIONS

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Manor Road, Richmond Ground Investigation Report July 2021







FAIRHURST



CONTROL SHEET

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1.0 INTRODUCTION AND OBJECTIVES

1.1 GENERAL

Fairhurst have been appointed by Avanton Richmond Development Limited (the 'client') to provide a Ground Investigation Report (GIR) for a site that currently comprises a warehouse structure occupied by a DIY and pets store, positioned to the west of Manor Road, Richmond (Post Code - TW9 1YB) (herein referred to as the 'site'). A site location plan is presented in **Appendix A**.

The purpose of the ground investigation and this report is to:

- Review the available environmental, historical and geological data;
- Characterise the soil and groundwater profile that exists at the site in a phased ground investigation;
- Confirm the findings of a Phase 1 Desk Stop Study carried out by Fairhurst (Geo-Environmental and Geotechnical Preliminary Risk Assessment, 126782, R1.6); and,
- Identify geotechnical site risks and constraints with respect to the proposed development.

This report includes the results of the first phase of ground investigation undertaken whilst the site remained operational. The second phase of investigation will be undertaken once demolition is complete. This Ground Investigation Report aims to confirm the findings of the Preliminary Risk Assessment in line with the Environment Agency's (EA) Land Contamination Risk Management (LCRM) Guidance and to inform the design under Building Regulations Part C with respect to contamination.

1.2 DEVELOPMENT PROPOSAL AND PLANNING CONTEXT

A planning application for the redevelopment of the Site was submitted to London Borough of Richmond Upon Thames (LBRuT) in February 2019 (ref. 19/0510/FUL) (the 'Original Proposed Development'), and was considered at LBRuT Planning Committee on 3 July 2019. The Planning Committee resolved that they were minded to refuse the Application, however on 29 July 2019 it was confirmed that the Greater London Authority (GLA) would act as the local planning authority for the purposes of determining the application.

Amendments to the Original Application

Following review of LBRuT's reasons for refusal and discussions with Officers at the GLA and Transport for London (TfL), the Applicant sought to review the scheme, with the principal aim of increasing the delivery of affordable housing through additional density and addressing other issues raised in the Mayor's Stage 2 Report.

The proposed changes necessitate an amendment to the Application's description of development. The revised description of development (hereafter referred to as the 'Amended Proposed Development') is as follows:

The current proposed development comprises the demolition of the current retail units and the development of 4 No. residential buildings, permeable paving, soakaway drainage and soft landscaping. Building A is a part 3, part 4, part 7 and 8 storey building, Building B is an 11 storey building, Building C is a part 8 and 10 storey building and Building D is a part 4, part 7 and part 8 storey building. In addition, a basement plant room is proposed beneath Building A.

The application is currently at assessment stage before it is due to go to committee.



Proposed development plans are presented in Appendix A.

1.3 SOURCES OF INFORMATION

The following existing information and documents relating to the site have been reviewed and form the basis of the assessment:

- British Geological Survey (BGS) Sheet 270 South London, Solid and Drift, 1:50,000 Scale (dated 2006);
- BGS Online Geology Viewer https://mapapps.bgs.ac.uk/geologyofbritain/home.html (accessed 19/05/2021);
- CIRIA C665 'Assessing risks posed by Hazardous Ground Gases to Buildings' (2007);
- BS8485(2015)+A1(2019) Code of Practice for the design of protective measures for methane and carbon dioxide ground gases for new buildings;
- CIRIA (1989) Special Publication 69 the Engineering Implications of Rising Groundwater Levels in the Deep Aquifer beneath London;
- DEFRA's MagicMaps website https://magic.defra.gov.uk/magicmap.aspx (accessed 09/04/2021);
- London Borough of Richmond upon Thames Planning Portal <u>Search planning applications</u> <u>London Borough of Richmond upon Thames</u> (accessed 19/05/2021);
- LMB Geosolutions, Factual Report on Ground Investigation (dated April 2021, see Appendix B);
- Series 600 Specification for Highway Works (dated 2017);
- SoBRA's Development of Generic Assessment Criteria for Assessing Vapour Risks to Human Health from Volatile Contaminants in Groundwater Version 1 (dated February 2017);

1.4 LEGISLATIVE CONTEXT

This assessment has been undertaken with due regard to Contaminated Land Guidance documents including the EA's LCRM. The methods used follow a phased, risk-based approach, with the potential environmental risk assessed qualitatively within the PRA, using the 'source-pathway-receptor contaminant linkage' concept. The S-P-R linkages are then targeted by intrusive investigation works, the results of which form the basis of a Generic Quantitative Risk Assessment presented herein. The reports aim to assess risks as introduced in Part IIA of the Environmental Protection Act 1990 (EPA, 1990) and Contaminated Land Statutory Guidance 2012.

1.5 CONFIDENTIALITY AND REPORTING LIMITATIONS

This report has been prepared for the sole use and reliance of Avanton Richmond Development Limited and should not be relied upon or transferred to any other parties without the express written authorisation of Fairhurst. Fairhurst accepts no duty or responsibility (including negligence) to any party other than the stated client and disclaims all liability of any nature whatsoever to any such party in respect of this report.



2.0 SITE SETTING AND HISTORY

This section provides a brief summary of pertinent information from Fairhurst's PRA. For a more detailed review the reader is referred to Fairhurst's Geo-Environmental and Geotechnical Preliminary Risk Assessment, 126782, R1.6.

2.1 SITE DESCRIPTION

The site is located approximately 0.6km north-east of Richmond town centre at approximate post code TW9 1YB and centred on National Grid Reference 518890, 175430. It is accessed via Manor Road, which bounds the site to the east. Railway lines form the north-western and southern boundaries.

The site is approximately triangular in shape, covering an area of about 1.8Ha and is currently occupied by a retail warehouse building (Homebase) in the central third, with associated car / bus parking in the north-eastern third and storage areas in the south-western third of the site.

2.2 SURROUNDING LAND USES

Surrounding land uses are mixed residential and commercial with the redeveloped Richmond gas works site located to the east.

2.3 SITE HISTORY

The site remained undeveloped until the late 1890s when a timber yard was noted on site. Railway siding were denoted to the south and west of the site by the early 20th century. A depot is shown on site by the 1960s together with railway cottages. By the 1970s on-site structures were redeveloped, with a possible warehouse style structure within the central portion. A fuel depot, electrical substation, coal hoppers and timber yard are detailed on site. Of these only the electrical substation is last identified in the early 1990s. By 1999 the site is presented in its current layout.

2.4 GEOLOGY

The 1:50,000 British Geological Survey (BGS) map for South London (Survey Sheet Number 270, dated 1998) and BGS online map viewer, including borehole records, have been reviewed to provide information on the published underlying geology and ground conditions.

The geology on site is summarised in Table 1.

Table 1 - Site Geology

Stratum	Area Covered	Estimated thickness (m)	Typical Description
Made Ground	Whole site	0.5m to 1.0m	Brown clayey silty sand with some gravel of brick, flint, concrete and clinker and some lenses of soft to firm brown sandy clay with occasional brick cobbles.
Kempton Park Gravel Member	Whole site	5.0m to 5.5m	The Kempton Park Gravel Member comprises sand and gravel, locally with lenses of silt, clay or peat.



Stratum	Area Covered	Estimated thickness (m)	Typical Description
London Cla Formation	Whole site	>50m	Silty sandy clay, described as stiff clay in borehole records TQ17NE436 and TQ17NE62.

Mining and Mineral Extraction

The Envirocheck identifies no records of mining, mineral sites or natural cavities within 250m of the site.

2.5 HYDROLOGY AND HYDROGEOLOGY

Hydrology

The River Thames is positioned approximately 1.4km to the north-west and 1.3km to the south-east at its closes positions, and generally flows in an easterly or south-easterly direction at these locations.

The nearest surface water feature is a pond, located circa 310m south of the site. The OS Water Network Map indicates the presence of possible field drains/ditches flowing in a southerly direction towards the pond, before trending to the east towards the River Thames.

Hydrogeology

The Environment Agency (EA) classifies the London Clay Formation as Unproductive Stratum whilst the Kempton Park Gravel Member is a Secondary A Aquifer.

The site and the surrounding area (<500m of the site boundary) are not situated within a groundwater source protection zone and there are no groundwater abstraction points situated within 500m of the site.

2.6 LANDFILL AND WASTE FACILITY RECORDS

According to the Envirocheck report, there are no current or historical landfills situated within 1,000 m of the site boundary.

2.7 RADON

A review of Public Health England's UK Maps of Radon website indicates the site to be situated within the lowest band of radon potential where less than 1% of homes are above the radon Action Level.



3.0 PERTINENT INFORMATION FROM THE PRA

The following section includes a summary of the Conceptual Site Model and the Geotechnical Engineering Assessment

3.1 Summary of Conceptual Site Model

Source Characterisation

A number of on-site and offsite potential sources of contamination were identified within the PRA. The most pertinent are summarised below:

On-site

- Made Ground
- · car parking facility and bus stand
- Electricity Substation
- Historical uses including car wash facility, timber yard, crane, railway sidings, fuel depot, coal hoppers, electrical substation and power station

Offsite

- Richmond gas works and associated activities, including railway sidings, gas holders, tank etc (1867 present), later detailed as works / depot
- Railway lines
- Historical uses including laundry, corporation depot, goods depot, later coach repair works, nursery, timber yard, smithy and 3no garages

Contaminants of concern associated with the selected sources outlined above are listed in Table 2 below. Whilst they have been withdrawn, Department of Environment (DoE) industry profiles have been utilised for reference, where available.

Table 2 - Contaminants of Concern

Source	Contaminants of Concern
On-site	
Made Ground	Metals, PAH, asbestos, TPH, ground gas
Former car wash	Metals, PAH , TPH, VOC, SVOC
Car parking	Metals, TPH, SVOC, VOC PAH
Historical uses including timber yard, crane, railway sidings, fuel depot, coal hoppers, electrical substation and power station	Metals, PAH, asbestos, TPH, PCB, VOC, SVOC, phenols, pesticides
Electrical substation	PCB, TPH, PAH, metals, VOC, SVOC
Off-site	



Source	Contaminants of Concern
Railway lines (1867 - present)	DoE industry profiles note the following potential contaminants in the vicinity of tracks: metals, VOC, PAH, pesticides
Richmond gas works and associated activities, including railway sidings, gas holders, tank etc (1867 - present)	Metals, TPH, PCB, PAH, VOC, SVOC, ammonia, phenols, cyanide (total), sulphates
Later detailed as works / depot (1975 - 1990s)	
Laundry (1896 - 1898)	VOC, solvents
Corporation depot (1896 - 2004)	Metals, TPH, VOC, SVOC, and PAH
Goods depot (1960 - 1970s). Later coach repair works (1970s - 1980s) and depot (late 1980s)	
Nursery (1896 - 1913)	Metals, pesticides
3no garages (1960 - 1990s)	Metals, TPH, VOC, SVOC, and PAH

Metals and inorganic compounds including but not limited to As, B, Cd, Cr total, Cr VI and III, Cu, Hg, Ni, Pb, Se, Zn/ phenols, cyanide (free and total), asbestos and sulphates / VOC: volatile organic compounds / SVOC: semi volatile organic compounds / PAH: polycyclic aromatic hydrocarbons / TPH CWG: total petroleum hydrocarbons / PCB: polychlorinated biphenyls / Ground gas including but not limited to CO2, CH4, CO, H2S

Potential Pathways

The potential pathways by which receptors might be exposed to contaminants (sources) at the site can vary depending on the proposed or current land use (i.e. commercial or residential land use). Noting that the proposed development is to include multi-storey apartment buildings and therefore assumed shared landscaping spaces the assessment has been based on a residential end use without plant uptake.

For humans, the following are considered plausible exposure pathways:

- Migration, accumulation and inhalation of soil gas / vapours via permeable soils and groundwater;
- Direct contact and ingestion / inhalation of contaminated soils in areas of soft landscaping; and
- Ingress of contaminants into conduits, contaminating drinking water supplies.

For building materials and below ground structures (including foundations and services), the following are considered plausible exposure pathways:

- Soil gas / vapour accumulation in confined spaces and voids within or beneath structures; and
- Direct contact of building fabric with contaminated soils.

For controlled waters, the following pathways may be present:

- Vertical leaching and migration of contaminants from the soil to groundwater; and
- Lateral migration of on-site groundwater off-site or from off-site groundwater on-site.



Potential Receptors

The following receptors are identified at the site:

- Human health: future site workers and residents and off-site adjacent land users, including neighbours and members of the public;
- Building materials and below ground structures (foundations and services); and
- Controlled waters: the underlying Secondary A Aguifer (Kempton Park Gravel Member).

The River Thames is not considered a receptor, noting its distance to the site (>1.6km). The off-site pond positioned circa 310m to the south is likely to be up hydraulic gradient and therefore is not considered to be a receptor from on-site contamination. Three groundwater abstractions were identified during council liaison, and were noted to be positioned west and north-west of the site and at closest 920m from the site; therefore, these have been discounted as they are not considered to be in hydraulic connectivity with the site.

It is assumed that appropriate Health & Safety measures, based upon a qualitative environmental risk assessment of site conditions by the contractor will be adopted during any future below ground maintenance works. This is likely to include personal protective equipment (PPE). It is considered that these measures will adequately mitigate the risk to construction and future maintenance workers from potential sources of contamination. Therefore, future construction and maintenance workers are not discussed further as part of this risk assessment.

Pollution linkages have not been identified to ecology as a Part IIA and Non-Part IIA Receptor at this stage. However, this report does not purport to be making ecological recommendations, for which a specialist should be consulted.

PRELIMINARY CONCEPTUAL SITE MODEL

Table 3 provides an evaluation of the potential contaminant linkages that were considered to be plausible on the basis of the information currently available.



Table 3 – Preliminary Conceptual Site Model

Source	Potential contaminants	Potential Pathway (s)	Potential receptor (s)	Assessment	Potential Severity	Potential Probability	Risk Class
	Metals, TPH, PAH, PCB, VOC, SVOC, sulphates, asbestos, pesticides	Direct contact with contaminated solls Direct ingestion / inhalation of contaminated soils Ingestion of contaminated water from drinking water supply pipes	Human health (on-site)	Development proposals include areas of soft landscaping. Residential end use is more sensitive than existing commercial. Possible that new drinking water supply pipes are to be laid.	Medium	Likely	Moderate
	Ground gases / VOC	Inhalation of accumulated soil ground gases or vapours Lateral migration of volatile compounds in groundwater	Human health (on and off site)	Potential for ground gases to be present attributable to Made Ground or the degradation of organic contamination. Furthermore, potential sources of VOC identified.	Severe	Low	Moderate
sonıces	Ground gases / VOC	Soil gas / vapour accumulation in confined spaces and voids within or beneath structures	Buildings and service conduits (on and off-site)	As above.	Severe	Low	Moderate
əfis-nO	Metals, TPH, PAH, PCB, VOC, SVOC, pesticides Sulphates in London Clay Formation	Direct contact of building fabric with contaminated soils	Buildings and service conduits (on and off-site)	Sources of contamination identified on-site and associated with historic uses which may result in aggressive chemical conditions within Made Ground. Soft landscaping associated with proposed development may increase vertical leaching of contamination. Plausible that proposed building foundations are positioned beneath the groundwater table.	Mild	Likely	Moderate / Low
	Metals, TPH, PAH, PCB, VOC, SVOC, asbestos, pesticides	Vertical leaching and migration of contaminants from the soil to groundwater Lateral migration of groundwater off-site	Secondary A Aquifer (on and off-site)	Soft landscaping associated with proposed development may increase vertical leaching of contamination. Whilst no abstractions / surface water features are identified, the resource potential of the Secondary A Aquifer should be considered. Shallow groundwater was encountered from 1.5m bgl within BGS borehole records in the Kempton Park Gravel Formation.	Medium	Likely	Moderate



Source	Potential contaminants	Potential Pathway (s)	Potential receptor (s)	Assessment	Potential Severity	Potential Probability	Risk Class	
	Ground gases /	Inhalation of accumulated soil ground gases or vapours	health	Potential sources of ground gases identified and there is potential for these to migrate in the unsaturated zone onto site.	Severe	<u>80</u>	Moderate	
	VOC	Lateral migration of volatile compounds in groundwater	(aus-10)	Potential for VOCs to migrate in groundwater or via the unsaturated zone to beneath the site.) 5 6 9			
onices	Ground gases / VOC	Soil gas / vapour accumulation in confined spaces and voids within or beneath structures	Buildings and service conduits (on - site)	As above.	Severe	Low	Moderate	I .
s ətis-ttO	Metals, TPH, PCB, PAH, VOC, SVOC, ammonia, cyanide (total), sulphates, pesticides	Contact with building structures and services with contaminated groundwater migrating onto site	Buildings and service conduits (on - site)	Plausible that proposed building foundations will be positioned beneath the groundwater table and therefore in contact with contaminated groundwater migrating onto site.	Mild	Likely	Moderate / Low	T
	Metals, TPH, PCB, PAH, VOC, SVOC, ammonia, cyanide	Vertical leaching and migration of contaminants from the soil to groundwater	ondary A fer (on-	Potential for contaminated groundwater to be flowing onto site. No abstractions identified on-site; however, resource	Medium	Likely	Moderate	ı
	(total), sulphates, pesticides	Lateral migration of off-site groundwater on-site	site)	potential could be impacted.				



3.2 Engineering Assessment

The proposed development, summarised in **Section 1.2**, is understood to comprise No.6 residential apartment blocks ranging between six and eight storeys with a small single-level basement. The basement will be approximately 3m below the proposed ground-floor level, located near the centre of the site and covers a smaller area than the above-ground structure. It is envisaged to be used as a plant room. A temporary sheet pile wall will be required to facilitate the construction of the basement.

Ancillary structures including the bin store and external masonry walls are proposed in the eastern part of the site, which could potentially be likely to be founded on shallow footings within the London Clay or Kempton Park Gravel Member. The external areas will be at similar levels to the existing ground surface.

POTENTIAL GEOTECHNICAL CONSTRAINTS

The following geotechnical considerations are associated with the development.

Basement Development

Consultation with LBRuT identifies that the basement development would be subject to Policy LP 11 from their local plan, which details:

- A. The Council will resist subterranean and basement development of more than one storey below the existing ground level to residential properties or those which were previously in residential use.
- B. Proposals for subterranean and basement developments will be required to comply with the following:
- 1. Extend to no more than a maximum of 50% of the existing garden land or more than half of any other undeveloped garden area (this excludes the footprint of the original building);
- 2. Demonstrate the scheme safeguards the structural stability of the existing building, neighbouring buildings and other infrastructure, including related to the highway and transport; a Structural Impact Assessment will be required where a subterranean development or basement is added to, or adjacent to, a listed building.
- 3. Use natural ventilation and lighting where habitable accommodation is provided;
- 4. Include a minimum of 1 metre naturally draining permeable soil above any part of the basement beneath the garden area, together with a minimum 200mm drainage layer, and provide a satisfactory landscaping scheme;
- 5. Demonstrate that the scheme will not increase or otherwise exacerbate flood risk on the site or beyond, in line with policy LP 21 Flood Risk and Sustainable Drainage;
- 6. Demonstrate as part of a Construction Management Statement that the development will be designed and constructed so as to minimise the impact during construction and occupation stages (in line with the Local Environmental Impacts, Pollution and Land Contamination policy of this Plan);
- C. Proposals for subterranean and basement developments, including extensions, as well as light-wells and railings, will be assessed against the advice set out in the Council's SPDs relating to character and design as well as the relevant Village Planning Guidance



and the forthcoming SPD on Basements and Subterranean Developments. Applicants will be expected to follow the Council's Good Practice Guide on Basement Developments.

Furthermore, the LBRuT planning advice note 'Good Practice Guide on Basement Developments', May 2015 Consultation with LBRuT identifies that the basement development would be subject to Policy LP 11 from their local plan, which details potential requirements including: contacting utilities, Network Rail and Transport for London to confirm that works will not interfere with their infrastructure; flood risk assessment taking consideration of groundwater and potential groundwater flooding; assessment of land stability; structural assessment taking consideration of ground conditions and groundwater, existing trees and infrastructure and drainage; site investigation; and assessment of ground movements.

On the basis of the foregoing, it is considered that a Basement Impact Assessment and/or Ground Movement Assessment may be required to confirm the absence of adverse impacts to existing offsite infrastructure assets or neighbouring structures, subject to the development details/design and liaison with TFL/LUL and/or Network Rail and the Local Planning Authority.

Below Ground Structures and Utilities

Council correspondence notes that the site is within a National Grid safeguard zone (unknown if this relates to the former Richmond Gas Works) and a possible Southern Gas Network structure was identified in the south-eastern corner of the site. Furthermore, existing utilities are likely to be present on-site associated with the existing development, including the electrical substation. Existing services may require removal, capping and diversion associated with the development. Furthermore, it is recommended that full service plans are obtained in advance of any below ground investigation works;

Relic foundations and structures may be present associated with the historic development of the site. Obstructions may need to be delineated and grubbed out as part of any future earthworks at the site; and

The site is within a high risk area with respect to unexploded ordnance. It is recommended that a UXO specialist is contacted prior to any below ground works.

Foundation Appraisal

There is the potential for aggressive sulphates in the Made Ground and the London Clay Formation, which may impact buried concrete and as such will require further consideration as part of any ground investigation;

Trees are present bounding the site and pre-application consultation with LBRuT identifies that there are tree preservation orders at the site. It is considered probable that existing tree roots are present in the Kempton Park Gravel Formation and should this formation prove to be cohesive in nature the volume change potential should be considered where trees are to be removed or planted associated with the development;

Structural loads are unknown at this stage. Noting proposed development heights of potentially up to 10 No. storeys (plus ground and basement floors), it is considered that loads may exceed traditional shallow foundations (i.e. pads and strips) and foundations may need to be piled. Following ground investigation consideration could be given to shallow foundations and raft basement slabs, dependent on settlement tolerances and the thickness and density of the Kempton Park Gravels;



Based on BGS borehole records, it is considered that a piled foundation solution would extend into the London Clay Formation. Foundation design will be subject to structural loads and ground investigation findings.

Adjacent Railways

District, Overground and National Rail tracks bound the site to the south and the west. It is likely that additional assessments will be required to confirm the potential impacts of development on these assets, including during ground investigation and future development.

Further Considerations

Soakaways may be feasible within the granular Kempton Park Gravel Formation; however, given the potential for contamination identified, further risk assessments may be required to ensure that these do not result in increased mobilisation of potential contamination. Furthermore, BGS borehole logs have identified a groundwater table from circa 1.5m bgl and the shallow depth to groundwater may preclude the use of soakaway drainage.



4.0 GROUND INVESTIGATION

Following development of the preliminary CSM and assessment of engineering considerations, Fairhurst scoped and designed a preliminary intrusive ground investigation targeted to refine the understanding of the site for geo-environmental and geotechnical purposes.

The preliminary ground investigation was carried out by LMB Geosolutions Ltd, under the management of Fairhurst, between the 21st April and 27th April 2021 with three post-fieldwork monitoring visits between the 5th and 26th May 2021. All exploratory locations were surveyed for below ground utilities and inspection hand pits were dug to 1.2 m bgl prior to drilling.

The original scope of works comprised the following:

- 5 No. rotary percussive boreholes to 15 m bgl (BH101 to BH105);
- 3 No. trial pits to 5 m bgl (TP01 to TP03);
- Standard Penetration testing in all boreholes;
- Logging of soils in accordance with BS 5930:2015+A1:2020;
- · Geotechnical and geo-environmental soil sampling;
- Field screening of soil samples using a photo-ionisation detector (PID);
- Installation of groundwater and ground gas monitoring installations at all borehole locations;
- 3 No. return monitoring visits across a 1 month period;
- 1 No. round of groundwater sampling; and,
- Geotechnical and geo-environmental laboratory testing.

The ground investigation was specified in accordance with the ICE UK Specification for Ground Investigation (2012) and was undertaken in general accordance with techniques outlined in Eurocodes 7 - BS EN 1997-1:2004, BS 5930:2015+A1:2020 Code of Practice for Ground Investigations and BS 10175:2011+A2:2017 Investigation of Potentially Contaminated Sites. The works were carried out by LMB Geosolutions with a part-time site presence from Fairhurst.

An exploratory hole location plan is shown on **Figure 2**, **Appendix A**. The LMB Factual Report which includes a detailed account of the ground investigation including the methodologies undertaken, is presented within **Appendix B**, should be referred to for the logs and all details relating to ground investigation techniques and methods adopted.

4.1 PRELIMINARY GROUND INVESTIGATION STRATEGY

Access was severely restricted due to ongoing commercial activities at the site. Exploratory hole locations were chosen on the basis of accessibility, geotechnical and geo-environmental requirements as set out in Table 4.



Table 4 - Ground Investigation Strategy

Exploratory	Rationale	Samples / Installation	Comment
Hole Location			
BH101	Geotechnical requirements – proposed building footprint Geo-environmental – in the vicinity of former car wash and assessment of Made Ground	Geotechnical and environmental soil samples Groundwater and ground gas installations	Groundwater sampling to target possible off-site sources: Richmond gas works and associated activities, including railway sidings, gas holders, tank etc (1867 - present) Later detailed as works / depot (1975 - 1990s) North / east of the site
BH102	Geotechnical requirements – proposed building footprint Geo-environmental – in the vicinity of former railway sidings and coal hoppers, assessment of Made Ground	Geotechnical and environmental soil samples Groundwater and ground gas installations	Groundwater sampling to assess onsite conditions
BH103	Geotechnical requirements – proposed building footprint Geo-environmental – assessment of Made Ground and car parking	Geotechnical and environmental soil samples Groundwater and ground gas installations	Groundwater sampling to target possible off-site sources: Richmond gas works and associated activities, including railway sidings, gas holders, tank etc (1867 - present) Later detailed as works / depot (1975 - 1990s) North / east of the site
BH104	Geotechnical requirements – proposed building footprint Geo-environmental – Ground and car parking	Geotechnical and environmental soil samples Groundwater and ground gas installations	Centre of the site, groundwater sampling to assess onsite conditions
BH105	Geotechnical requirements – proposed building footprint Geo-environmental – Ground and car parking	Geotechnical and environmental soil samples Groundwater and ground gas installations	Groundwater sampling to target possible off-site sources: Corporation depot (1896 - 2004), Goods depot. Later coach repair works (1970s - 1980s) and depot (late 1980s)
TP101	Geotechnical requirements – proposed building footprint Geo-environmental – in the vicinity of Former coal hopper, crane, substation	Geotechnical and environmental soil samples	Assessment of shallow ground conditions



	and assessment of Made Ground		
TP102	Geotechnical requirements – proposed building footprint Geo-environmental – assessment of Made Ground and in the vicinity of former depot	Geotechnical and environmental soil samples	Assessment of shallow ground conditions
TP103	Geotechnical requirements – proposed building footprint Geo-environmental – assessment of Made Ground and in the vicinity of former railway siding	Geotechnical and environmental soil samples	Assessment of shallow ground conditions

4.2 Limitations

As the ground investigation was undertaken during the operational phase of the site, the locations selected did not target all identified on-site and off-site sources of contamination and some areas of the site remain un-investigated (principally areas beneath exiting building footprints). As previously stated, the GI Strategy set out above is the first phase in a two-stage investigation. The next phase of ground investigation is planned post demolition and will likely comprise the drilling and installation of additional boreholes and trial pitting to fill the data gaps.



5.0 GROUND CONDITIONS ENCOUNTERED

The following section of this report describes the ground conditions encountered during the Fairhurst ground investigation works on the subject site.

5.1 GEOLOGY

The ground conditions encountered on site were consistent with the BGS records.

A summary of the ground conditions encountered during the ground investigation is presented in **Table 5**, exploratory hole logs are presented in LMB's Factual Report in **Appendix B**.

Table 5 – Summary of Ground Conditions Encountered

LITHOLOGY	PROVEN BASE OF LITHOLOGY (m bgl)	PROVEN BASE OF LITHOLOGY (m AOD)	THICKNESS (m)	TYPICAL DESCRIPTION
Surfacing	0.1 – 0.3	6.02 – 6.48	0.1 – 0.3	Brick paving and subbase (0.1m bgl) and tarmac over roadstone gravel (0.3m bgl)
Made Ground	0.65 – 2.2	4.05 – 5.64	0.4 – 2.0	Brown to dark brown clayey gravelly sand with occasional brick and concrete cobbles. Gravel sub-angular fine to coarse flint, brick, concrete and rare clinker.
Kempton Park Gravel Member	4.3 – 7.0	-0.3 – 1.95	1.55 - 4.1	Brown to orange slightly gravelly medium to coarse SAND. Gravel sub-angular to rounded medium to coarse flint.
London Clay Formation	Proven to 7.45 – 25.00	Proven to - 1.7518.75	Proven thickness 1.35 – 20.70	Dark grey to grey brown CLAY. Closely fissured.

5.2 SURFACING

Brick paving over a sub-base was encountered in boreholes BH102, BH103, BH104 and trial pits TP101 and TP102. Trial pit TP102 also reported a weak concrete layer between 0.3m and 0.5m bgl.

The surfacing in trial pit TP103 was reportedly reinforced concrete, which was present to a depth of 0.24m bgl. Borehole BH105 was covered with 0.18m of asphalt over road-stone gravel the base of which was recorded at 0.30m bgl.

Grass over clayey sand with rootlets was encountered at ground level within BH101 and recorded to a depth of 0.25m bgl.



5.3 MADE GROUND

Made Ground was encountered at all exploratory hole locations beneath the surfacing. The top of the Made Ground deposits were encountered at depths of between 0.06m and 0.25m bgl (6.02m and 6.45m AOD), with the base of this material recorded at depths of between 0.65m and 2.20m bgl (5.61m and 4.05m AOD). The maximum recorded thickness of Made Ground was within BH104, were these deposits were 2.14m thick, with this sampling position undertaken towards the centre of the site.

The Made Ground was generally granular in nature, typically comprising of brown to dark brown clayey gravelly SAND with occasional brick and concrete cobbles. Gravel was described as sub-angular fine to coarse flint, brick, concrete, glass and clinker. Ash deposits were encountered within the Made Ground in BH101, with this material present at depths of between 0.70m and 1.70m bgl.

Made Ground was inherently heterogeneous and varied in composition locally and reference should be made to the borehole logs for further details.

5.4 KEMPTON PARK GRAVEL MEMBER (RIVER TERRACE DEPOSITS)

Superficial deposits interpreted as the Kempton Park Gravel Member were encountered at all exploratory hole locations beneath the Made Ground. This material was predominantly granular in nature, however, locally with BH01, TP01 and TP02 cohesive material was initially encountered and proven to depths of between 1.80m and 3.00m bgl (3.70m and 4.40m AOD).

The top of the strata was encountered at depths of between 0.65m and 2.20m bgl (5.61m and 4.05m AOD), and where the base of the superficial deposits was proven, BH101 to BH105, this was recorded at depths of between 4.30m and 7.00m bgl (1.95m and -0.30m AOD). The base of the strata was not proven within three exploratory positions, TP101, TP102 and TP103, with these exploratory positions terminated within this strata, all at a depth of 2.50m bgl. The maximum thickness was 5.30m encountered at BH101, which was undertaken in the north east of the site.

The granular deposits were generally found to comprise of brown to orange slightly gravelly to gravelly SAND, whilst the cohesive material was typically described as brown to orange sandy CLAY.

5.5 LONDON CLAY FORMATION

London Clay Formation was encountered within five of the eight exploratory positions, the five exploratory boreholes, with the top of the stratum encountered between 4.30m (BH104) and 7.00m (BH101) bgl (1.95m and -0.30m AOD). The base of the London Clay Formation was not proven within any of the exploratory boreholes which encountered this stratum, but it was recorded to extend to depths of between 7.45m and 25m (-1.05m and -18.75m AOD), with a maximum thickness of 20.70m recorded at BH104.

The London Clay was generally described as dark grey to grey brown CLAY with a weathered layer at the top of the stratum described as brown with orange brown mottling CLAY with sand and rare flint gravel. The weathered layer was limited in thickness ranging between 0.05m (BH105) and 0.10m (BH101, BH102, BH103 and BH104).

A Mudstone band, comprising very weak to moderately weak grey mudstone, was encountered within the London Clay Formation at one exploratory position, namely BH104, at a depth of 12.70m bgl (-



6.45m AOD) and the base of this was proven at a of depth of 13.00m bgl (-6.75m AOD), with a thickness of 0.30m.

Shell fragments were encountered at 6.00m bgl (0.25m AOD) in BH104 and 13.00m bgl (-6.75m AOD) in BH105 whilst pyrite nodules were encountered at 13.90m and 23.00m bgl (-7.65m and -16.75m AOD) in BH105. Additionally, bioturbation traces were recorded within BH103 at 8.00m bgl (-1.46m AOD) and within BH104 at 6.00m bgl and 8.00m bgl (0.25m and -1.75m AOD).

5.6 GAS AND GROUNDWATER INSTALLATIONS

All boreholes were installed with 50mm diameter combined ground gas and groundwater monitoring wells within the Made Ground, Kempton Park Gravel Member and London Clay Formation. A summary of the monitoring wells installed is presented in **Table 6** below with a full list presented on the exploratory hole logs within LMB's Factual Report (**Appendix B**).

Table 6- Summary of Monitoring Wells

EXPLORATORY HOLE LOCATION (SURFACE ELEVATION – m AOD)	TOP AND BASE OF RESPONSE ZONE (m bgl)	TOP AND BASE OF RESPONSE ZONE (m AOD)	STRATA TARGETED
BH101 (6.70)	1.50 to 8.45	5.20 to -1.75	MG / KPGM / LCF
BH102 (6.40)	1.00 to 7.00	5.40 to -0.60	MG / KPGM / LCF
BH103 (6.54)	2.00 to 8.00	4.54 to -1.46	KPGM / LCF
BH104 (6.25)	1.00 to 5.00	5.25 to 1.25	MG / KPGM / LCF
BH105 (6.25)	1.00 to 6.00	5.25 to 0.25	MG / KPGM / LCF

MG – Made Ground, KPGM – Kempton Park Gravel Member, LCF – London Clay Formation

Three return visits for ground gas / groundwater level monitoring were completed by LMB on the 5th, 13th and 26th May 2021.

The wells were developed on 29th April 2021 and groundwater was sampled on 5th May 2021 using low-flow sampling techniques with a peristatic pump so groundwater was micro-purged and directed via a flow cell installed with a multi-parameter probe so that indicator parameters (i.e. pH, electrical conductivity, oxidation reduction potential, dissolved oxygen and temperature) could be monitored to ensure stability prior to sampling into dedicated sampling containers.

The combined groundwater and ground gas monitoring visits included the use of a PID to provide a semi-quantitative measure of Volatile Organic Compounds (VOC), an interface probe to measure product thickness, if and where present, and groundwater level, and measurement of ground gases including carbon monoxide (CO), carbon dioxide (CO₂), hydrogen sulphide (H₂S), methane (CH₄) and oxygen (O₂). Atmospheric pressure readings and gas flow rates were also undertaken.

5.7 GROUNDWATER

Groundwater strikes reported by LMB during the ground investigation are summarised in Table 7.

Table 7 – Summary of Groundwater Strikes Recorded During Investigation



Borehole	Groundw	ater Level	Stratum	
Borenoie	m bgl m AOD		Suatum	
BH101	4.50 rising to 4.30	2.20 rising to 2.40	Kempton Park Gravel Member	
BH102	4.50 rising to 3.90	1.90 rising to 2.50	Kempton Park Gravel Member	
BH103	3.50 rising to 2.80	3.04 rising to 3.74	Kempton Park Gravel Member	
BH104	3.20 rising to 2.60	3.05 rising to 3.65	Kempton Park Gravel Member	
BH105	2.80 rising to 3.00	3.45 rising to 3.25	Kempton Park Gravel Member	

In order to understand the groundwater regime more comprehensively following the completion of site investigation work, groundwater monitoring rounds were undertaken on 5th, 13th and 26th May 2021 as summarised in **Table 8**.

Table 8 – Summary of Groundwater Levels Recorded During Return Visit Monitoring

BOREHOLE	RESPO	NSE ZONE	GROUNDWATER LEVEL		
	m bgl STRATA		m bgl	m AOD	
BH101	1.50 to 8.45	MG / KPGM / LC	2.46 to 2.56	4.24 to 4.14	
BH102	1.00 to 7.00	MG / KPGM / LC	2.32 to 2.41	4.08 to 3.99	
BH103	2.00 to 8.00	KPGM / LC	3.88 to 4.26	2.66 to 2.28	
BH104	1.00 to 5.00	MG / KPGM / LC	3.66 to 4.68	2.59 to 1.57	
BH105	1.00 to 6.00	MG / KPGM / LC	3.45 to 4.03	2.80 to 2.22	

MG – Made Ground, KPGM – Kempton Park Gravel Member, LC – London Clay Formation

The findings suggest that a shallow body of groundwater is situated within the Kempton Park Gravel Member. The Kempton Park Gravel Member was saturated at varying levels across the site with recorded groundwater levels at between 4.24m to 3.99m AOD in the north of the site (BH101 and BH102) lowering to between 2.80m to 1.57m AOD in the central and southern area of the site (BH103, BH104 and BH105).

It should be noted that groundwater levels are subject to variation as a result of seasonal changes and during/following periods of adverse weather conditions.

5.8 VISUAL OR OLFACTORY EVIDENCE OF CONTAMINATION

Visual and olfactory evidence of potential contamination was recorded by the logging engineer in the following positions:

- Black ashy material and clinker recorded at BH101 between 0.70m and 1.70m bgl;
- Grey colouration and a hydrocarbon odour recorded at BH102 at a depth of 4.00m bgl;
- A strong hydrocarbon odour recorded at BH104 between 2.20m and 4.30m bgl; and,
- Black colouration was recorded at TP101 between 0.60m and 0.90m bgl.



5.9 LABORATORY TESTING

Geotechnical and geo-environmental laboratory testing was scheduled by Fairhurst and undertaken by i2 Analytical, an MCERTS and UKAS accredited laboratory.

Selected geo-environmental soil samples were tested for the following analytes:

- Metals arsenic, boron, cadmium, chromium, hexavalent chromium, copper, lead, mercury, nickel, selenium and zinc;
- Organic Compounds TPH criteria working group (CWG), BTEX, phenols, PAHs, VOCs and SVOCs;
- Inorganic Compounds asbestos identification, total cyanide, total sulphate, total organic carbon (TOC), organic matter, sulphide and pH;
- · Polychlorinated biphenyls (PCBs); and,
- Waste Acceptance Criteria (WAC) testing.

Selected groundwater samples were tested for the following analytes:

- Metals antimony, arsenic, barium, beryllium, boron, cadmium, calcium, chromium, hexavalent chromium, cobalt, copper, iron, lead, magnesium, manganese, mercury, molybdenum, nickel, phosphorus, potassium, selenium, sodium, tin, vanadium and zinc;
- Organic Compounds TPH criteria working group (CWG), BTEX, phenols, PAHs, VOCs and SVOCs;
- Inorganic Compounds total cyanide, free cyanide, complex cyanide, sulphate as SO4, TOC, sulphide and pH; and,
- Other electrical conductivity at 20 C, pesticide screen.

Selected geotechnical samples were scheduled for the following tests:

- Particle size distribution;
- Atterberg limit testing;
- Moisture content;
- Dry density / moisture content relationship light compaction;
- California Bearing Ratio (CBR);
- Unconsolidated undrained single stage triaxial compression tests; and,
- One-dimensional oedometer consolidation tests.

5.10 SAMPLE DEVIATIONS

Several samples obtained during the investigation were recorded as deviated, details of which are provided below:

 Sample holding times were exceeded for soil samples BH101 (0.80 m bgl), BH102 (0.50 m bgl), BH103 (0.20 m bgl) and TP103 (0.50m bgl) for sulphide and total cyanide analysis; and,



 Sample holding times were exceeded for soil samples TP103 (0.50m bgl) for BTEX and MTBE (Monoaromatics) analysis.



6.0 GEOTECHNICAL TESTING & ENGINEERING PROPERTIES

The following section discusses the ground conditions determined from the Fairhurst ground investigation and laboratory testing described in **Section 5**.

The below figures should be referenced accordingly in relation to the field and laboratory testing results:

- Figure 3 Moisture Content versus Depth
- Figure 4 Casagrande Plasticity Chart
- Figure 5 SPT-N₆₀ versus Depth

The laboratory data referred to is provided within the LMB Factual Report (**Appendix B**). A summary of the tests undertaken during the Fairhurst ground investigation is presented in **Table 9**.

Table 9 - Summary of Geotechnical Testing Undertaken

GEOTECHNICAL TEST		SURFACING	MADE GROUND	KEMPTON PARK GRAVELS	LONDON CLAY FORMATION
In-situ	Standard Penetration Tests (SPT)	-	3	16	22
	Moisture Content Determination	-	4	9	19
	Atterberg Limit Determination	1	-	2	12
	Particle Size Distribution (PSD) Analysis	-	-	4	-
Laboratory	California Bearing Ratio (CBR)	-	-	1	-
	Undrained Unconsolidated (UU) Single Stage Triaxial Testing	-	-	-	10
	Dry Density / Moisture Content Relationship Light Compaction	-	-	1	-
	One-dimensional Oedometer Consolidation Testing	-	-	-	5
	BRE Sulphate Suite D	-	2	4	13

6.1 SURFACING

Due to the limited thicknesses of the surfacing encountered on site and the nature of the proposed development, no geotechnical in-situ or laboratory testing was undertaken on the surfacing material.



6.2 MADE GROUND

Moisture Content and Classification Testing

Four Moisture Content tests were undertaken on samples of the granular Made Ground from between 0.75m (BH101 and BH105) to 1.20m (BH101) bgl. The moisture contents of the samples ranged between 7% (BH105, 0.75m bgl) and 19% (BH101, 0.80m bgl), with an average moisture content of 12.4%.

Strength and Stiffness

Three SPTs were undertaken within the Made Ground, with corrected N₆₀ values of between 8 and 9. However, due to the lack of data collected from the stratum and the inherent variability within Made Ground, caution should be exercised when determining parameters for design purposes.

Chemical Analysis and Sulphate Classification

Two soil samples of the Made Ground were scheduled for Suite D aggressive chemical environment for concrete (ACEC) classification in accordance with BRE Special Digest 1:2005, Concrete in Aggressive Ground.

The concentrations of Water Soluble Sulphate were 223mg/l (BH101 at 1.2m) and 30.5mg/l (BH103 at 0.80m), with the pH for the soil being 9.3 and 8.5, respectively. The groundwater analysis recorded a maximum sulphate (as SO₄) value of 156 mg/l and the characteristic pH is 7.26, giving a Design Sulphate Class of DS-1 and ACEC class of AC-1, assuming mobile groundwater conditions.

6.3 KEMPTON PARK GRAVEL MEMBER

Particle Size Distribution

Particle Size Distribution testing was undertaken on four samples of the Kempton Park Gravel Member deposits at depths of between 2.00m (TP103) and 6.00m (BH101) bgl. Three samples, recovered from BH101, 6.00m bgl, BH102, 4.00m bgl and TP102, 2.30m bgl, were described as slightly clayey slightly silty gravelly SAND whilst the sample recovered from TP103, 2.00m bgl was described as slightly clayey slightly silty very sandy GRAVEL. The samples were shown to comprise between 15% and 49% of gravel, between 46% and 81% of sand and between 4% and 10% of fines (silt and clay), which is consistent with the on-site descriptions

The initial PSD testing suggests that the material might be suitable as a Class 1 material for re-use as general fill, in accordance with Series 600 Specification for Highway Works. However, further testing should be undertaken under a site-specific earthworks specification when the requirements for material re-use are confirmed.

Moisture Content and Classification Testing

Nine Moisture Content and two Atterberg Limit tests were undertaken on samples of the Kempton Park Gravel Member. The moisture contents of the samples ranged between 5.4% and 18%, with an average moisture content of 9.5%. The Atterberg Limit tests presented liquid limit results of 29% and 35% and plasticity index results of 16% and 17%, consistent with low to medium plasticity clays. The modified plasticity indices are between 8% and 9 %, described as having a medium to high volume change potential in accordance with NHBC Standards Chapter 4.2 (2017).



Using the relationship between moisture content and Atterberg limits proposed by Driscoll (1983), one of the samples appeared to have been desiccated at the time of sampling.

Compaction Testing

One California Bearing Ratio (CBR) test was undertaken on the Kempton Park Gravel Member from a sample recovered from BH101 at a depth of 3.00m bgl which recorded a result of 17%.

One compaction test to determine the moisture content to dry density relationship has been undertaken on Kempton Park Gravel Member from a sample recovered from TP103 at a depth of 1.00m bgl. The test recorded an optimum moisture content of 9.2% and a maximum dry density of 2.16Mg/m³. The average moisture content for the Kempton Park Gravel Member is consistent with the optimum moisture content.

Strength and Stiffness

A total of sixteen SPTs were undertaken within the Kempton Park Gravel Member, with corrected N_{60} values ranging from 15 (BH102, 5.0m bgl) to >50 (BH105, 2.0m bgl), correlating to medium dense to very dense.

Chemical Analysis and Sulphate Classification

Four soil samples of the Kempton Park Gravels were scheduled for Suite D aggressive chemical environment for concrete (ACEC) classification in accordance with BRE Special Digest 1:2005, Concrete in Aggressive Ground.

The two maximum concentrations of Water Soluble Sulphate were 1,840mg/l (BH101 at 2.0m) and 463mg/l (BH101 at 2.75m), with the pH for the soil being 7.8 for both, giving a Design Sulphate Class of DS-3 and ACEC class of AC-3, assuming mobile groundwater conditions.

6.4 LONDON CLAY FORMATION

Moisture Content and Classification Testing

Nineteen Moisture Content and twelve Atterberg Limit tests were undertaken on samples of the London Clay Formation. The moisture contents of the samples ranged between 18% and 32%, with an average moisture content of 27%. The Atterberg Limit tests presented liquid limit results that ranged from 55% to 81% and plasticity index results from 32% to 50%, consistent with high to very high plasticity clays. The modified plasticity indices are between 9% and 50 %, described as having a low to high volume change potential in accordance with NHBC Standards Chapter 4.2 (2017). In the majority of occasions the volumetric change potential would be high.

Unit Weight

The bulk unit weight of the London Clay Formation has been determined from eleven triaxial tests and three one-dimensional oedometer tests. The results ranged from 1.88 to 2.21 Mg/m³.

Strength and Stiffness

A total of twenty-two SPTs were undertaken within the London Clay Formation, with corrected N_{60} values ranging from 8 (BH103 and BH105, both at 5.0m bgl) to >50 (BH104, 12.50m bgl). Using the correlation proposed by Stroud (1974) and an f_1 factor of 4.5, the undrained shear strength of the material is derived to be between 36.45kPa and >225kPa, corresponding to low to very high strength material in accordance with BS EN ISO 14688-2:2018.



The undrained shear strength of the material was further assessed by ten laboratory triaxial tests on undisturbed soil samples from 6.50m (BH102) to 23.00m (BH104) bgl. The results of the triaxial tests ranged from 93kPa to 338kPa, indicative of high to extremely high strength clay in accordance with BS EN ISO 14688-2:2018.

One-dimensional Consolidation Properties

Five one-dimensional oedometer consolidation tests were undertaken on samples taken from 6.50m (BH104) to 18.50m (BH105) bgl within the London Clay Formation. The tests consisted of four to five load stages and one unloading stage, applying pressures of between 65kPa and 2,960kPa. The coefficient of volume compressibility, m_{ν} , ranged between 0.031 and 0.31 m^2 /MN, indicative of a very low to medium compressibility soil.

Chemical Analysis and Sulphate Classification

Thirteen soil samples of the London Clay were scheduled for Suite D aggressive chemical environment for concrete (ACEC) classification in accordance with BRE Special Digest 1:2005, Concrete in Aggressive Ground.

The two maximum concentrations of Water Soluble Sulphate were 777mg/l (BH105 at 23.0m) and 671mg/l (BH105 at 17.0m), with the pH for the soil being 8.5 for both, giving a Design Sulphate Class of DS-2 and ACEC class of AC-2, assuming mobile groundwater conditions.



8.0 GEOTECHNICAL ASSESSMENT

8.1 GENERAL

The proposed development, summarised in **Section 1.2**, is understood to comprise a series of residential apartment blocks ranging in storey heights from three to eleven, with locally a single-level basement in one of the blocks. The basement will be approximately 3m below the proposed ground-floor level, located near the centre of the proposed building and covers a smaller footprint area than the above-ground structure. It is envisaged to be used as a plant room.

The site is generally flat and situated at an elevation between 33.78m and 33.95m above ordnance datum (AOD). There is a retaining wall adjacent to Manor Road on the eastern site boundary. Furthermore, trees and vegetation were noted during Fairhurst's walkover along the western and northwestern site boundaries.

8.2 GEOTECHNICAL CONSIDERATIONS

Presence of Existing Services

Council correspondence notes that the site is within a National Grid safeguard zone (unknown if this relates to the former Richmond Gas Works) and a possible Southern Gas Network structure was identified in the south-eastern corner of the site. Furthermore, existing utilities are likely to be present on-site associated with the existing development, including the electrical substation. Existing services may require removal, capping and diversion associated with the development. Furthermore, it is recommended that full service plans are obtained in advance of any below ground investigation works

Unexploded Ordnance (UXO)

The site is within a high risk area with respect to unexploded ordnance. An UXO specialist watching brief is required for any below ground works.

Variability of Made Ground and Natural Deposits

The presence of Made Ground has been proven through the intrusive ground investigation. The Made Ground was generally found to be between 0.41m and 2.14m thick and comprised both cohesive and granular material with local variations in composition.

Although limited geotechnical testing was undertaken on the Made Ground, the material is anticipated to be variable in nature and comprise predominantly low strength material. As such, it is not considered to be a suitable founding strata.

With regard to the superficial deposits, these belonged to the Kempton Park Gravel Member, which were predominantly granular in nature, although locally cohesive in the upper parts. With regard to the cohesive deposits, this was visually assessed as being of firm consistency, and would be expected to have an undrained shear strength in the order of 50 kPa.

With respect to the granular deposits, these were typically medium dense to very dense soils, with the average 'N' value being 30, which is the boundary of medium dense / dense in accordance with BS 5930:2015. Generally, these deposits were found to directly underlie the Made Ground and the depths stated above; however, locally underling the cohesive deposits at depths of 1.80m to 3.00m. For initial design purposes these deposits would be expected to support an Allowable Bearing Capacity of in the order of 175 kN/m².



The top of the London Clay Formation was encountered at depths of between 4.30m and 7.00m bgl, with this encountered in all five borehole locations. The thickness of the weathered horizon also appears to be consistent across these locations, and relatively limited in terms of thickness. The laboratory testing within this material have shown that this is of high to very high strength, with undrained shear strengths typically in excess of 100 kPa.

Relic Foundations and Underground Structures / Obstructions

Due to the historical and present site use, existing foundations of the former buildings on site will likely be encountered during the construction process with these resulting in constraints to the new proposed development. The nature of below ground structures associated with existing structures, which are in uninvestigated areas of the site, is unknown and requires further supplementary investigation.

Further, there is a retaining wall adjacent to Manor Road on the eastern boundary of the site which requires investigation to determine the existing foundation depths and toe width.

Cohesive Deposits with Volume Change Potential

Cohesive Made Ground and the London Clay Formation have been determined to have a low to high volume change potential in accordance with NHBC Standards Chapter 4.2 (2017). Any proposals for new vegetation near structures should be reviewed in order to assess the risk of seasonal shrink/ swell movement in cohesive soils leading to differential settlement. Should any ground-bearing or shallow foundations be proposed, reference should be made to the NHBC recommendations regarding the minimum foundation depths required in shrinkable soils.

Foundation Options

Given the proposed development and the anticipated high magnitude loading, non-standard foundations in the form of piled foundations would be a feasible foundation solution, with the piles terminated within the London Clay Formation. Based on the ground conditions encountered on site and its proximity to nearby structures, rotary bored or continuous flight auger (CFA) piles are considered to be the most suitable pile types. However, consideration should be given to the potential presence of mudstone bands within the London Clay Formation and shallow groundwater ingress. Advice should be obtained from specialist piling contractors in terms of whether the higher torque CFA rigs would be able to penetrate the identified mudstone bands at the depths where these have been identified to be present.

Ancillary structures including the bin store and external masonry walls are proposed in the eastern part of the site, which are likely to be founded on shallow traditional footings. The external areas will be at similar levels as existing ground as no significant changes in levels are believed to be proposed.

Given the proximity of the railway land, consultation with the rail operator will be required before any intrusive works.

It should be noted that at the detailed design stage an assessment should be undertaken to ascertain whether the granular Kempton Park Gravel Member deposits would provide a suitable bearing stratum for some of the structures on site. Given the depths these deposits were encountered at, they would be present within the realms of conventional foundation, albeit locally with trench fill methods required. Due consideration should be given to the shallow groundwater, which may preclude trench foundations being a viable technical option.



With respect to the cohesive Kempton Park Gravel Member deposits these are only likely to provide a suitable bearing stratum for the Ancillary structures including the bin store and external masonry walls are proposed in the eastern part of the site, albeit this will require to be reviewed at detailed design stage.

Proposed Basements

The anticipated basement depth is 3m bgl, which is within the Kempton Park Gravels and beneath the resting groundwater table; however, there may be further excavation beneath this depth for proposed foundations.

Floor Slab

Due to variability in the composition and geotechnical properties of the Made Ground, it is not recommended to found the proposed ground level floor slab on the material. It is recommended that either a pile supported suspended floor slabs or suspended floor should the detailed design indicate that the granular deposits provide a suitable bearing stratum.

External Paved Areas

One laboratory California Bearing Ratio (CBR) tests was undertaken on the Kempton Park Gravel Member deposits from a sample recovered from BH101 at a depth of 3.00m bgl which recorded a result of 17%.

Where undertaken formation levels for access roads and road pavements within the external area of the proposed development are likely to be within the Made Ground deposits. Due to the variable composition and thickness of the Made Ground deposits across the site, further investigations in the form of CBR testing may be required for the determination of capping thicknesses and pavement design due to their variability, albeit typically provision of a sufficiently thick capping layer should be assumed to be required. The use of geogrid reinforcement may also permit a reduced capping layer to be adopted.

Earthworks and Material Re-use

Preliminary geotechnical laboratory testing suggests that the granular deposits would be classified as Class 1A/B material for re-use as general fill, in accordance with Series 600 Specification for Highway Works. A proportion of the as-dug materials are likely to require some degree of natural or artificial drying out, where wetter than optimum, in order to permit their placement at or close to the optimum moisture content. Further testing will be required in order to provide recommendations on the classification and placement of the material under a site-specific earthworks specification.

Groundwater and Excavations

Groundwater monitoring from the ground investigation suggests that groundwater may be encountered below 2.32m bgl during excavations. Close side support (temporary shoring) and appropriate dewatering measures, such as provision of sump and pump type water control systems may be required during the excavation of the basement and service corridors. It is understood that a temporary sheet pile wall will be installed to facilitate the construction of the basement which should provide groundwater cut-off.

The main contractor should provide details on how they intend to ensure the safety and short to medium duration stability of excavations. Safety precautions in accordance with industry best practice should be adopted for entry into excavations.



Drainage and Soakaways

Following excavation, soakaway testing was completed in general accordance with BRE 365 (2016). The trial pits were surcharged with water from a bowser and the fall in depth of water with time recorded. Due to the relatively slow rate of infiltration repeat tests were not feasible.

The infiltration test data was assessed in accordance with the methodology outlined in BRE Digest 365 (2016): Soakaway Design. This document describes 'design and construction procedures for soakaways explains how to calculate rainfall design values and soil infiltration rates and gives design examples.'

None of the soakaway infiltration tests proceeded through the required 75% to 25% effective depth of the excavation in accordance with BRE365.

Based on the cohesive and low permeability nature of the ground conditions on site and the expected shallow groundwater levels, the use of soakaways is unlikely to be feasible.

Chemical Attack on Buried Concrete

Based on chemical laboratory test results, the stratum beneath the site has been categorised as Design Sulphate Class and ACEC Class in accordance with BRE Special Digest 1:2005, Concrete in Aggressive Ground, as follows:

- Groundwater testing was consistent with a Design Sulphate Class DS-1 AC-1s;
- Made Ground testing was consistent with a Design Sulphate Class DS-1 AC-1;
- Kempton Park Gravels testing was consistent with a Design Sulphate Class DS-3 AC-3;
- London Clay testing was consistent with a Design Sulphate Class DS-2 AC-2.

It is recommended that a concrete classification of DS-3 AC-3 should be specified in the design and construction of substructures.



9.0 GEO-ENVIRONMENTAL RISK ASSESSMENT

9.1 SUMMARY OF ANALYTICAL LABORATORY TESTING UNDERTAKEN

Geo-environmental laboratory testing was undertaken on soil samples collected during the ground investigation, as summarised in **Table 10**:

Table 10 - Geo-environmental Laboratory Soil Testing

GEO-ENVIRONMENTAL TEST	NO. SCHEDULED
Metals – arsenic, boron, cadmium, chromium, hexavalent chromium, copper, lead, mercury, nickel, selenium and zinc	8
TPHCWG, PAHs, Phenols and BTEX	8
SVOCs and VOCs	2
Asbestos screen	8
SOM	2
TOC, pH, sulphide, total cyanide and total sulphate	8
PCBs	1
Waste Acceptance Criteria Testing (BSEN 12457/3)	1

Table 11 provides a summary of the chemical analysis completed on the groundwater samples which was collected from the 5 No. installations on 5th May 2021.

Table 11 – Geo-environmental Laboratory Groundwater Testing.

GEO-ENVIRONMENTAL TEST	NO. SCHEDULED
Metals – antimony, arsenic, barium, beryllium, boron, cadmium, calcium, chromium, hexavalent chromium, cobalt, copper, iron, lead, magnesium, manganese, mercury, molybdenum, nickel, phosphorus, potassium, selenium, sodium, tin, vanadium and zinc	5
TPHCWG, PAHs, Phenols and BTEX	5
SVOCs and VOCs	5
PCBs	1
Pesticide screen	3
Total cyanide, free cyanide, complex cyanide, sulphate as SO4, TOC, sulphide, electrical conductivity at 20 C and pH	5

9.2 ASSESSMENT CRITERIA AND APPROACH - RISKS TO HUMAN HEALTH

For the assessment of the soil analytical results, with the exception of Lead, the human health assessment criteria applied are the 2015 LQM/CIEH 'Suitable 4 Use Levels' (S4ULs) for residential without plant uptake end use. The S4ULs are based on Health Criteria that represent minimal or



tolerable levels of risks to health as described in the EA's SR2 guidance, ensuring that the resulting assessment criteria are 'suitable for use' under planning.

For Lead, S4ULs are not available and the 'low risk' Category 4 Screening Level (C4SL) for the residential without home grown produce land-use was adopted.

The assessment criteria outlined above have been used to provide a Generic Quantitative Risk Assessment (GQRA). No statistical analysis has been completed and recorded concentrations have been compared directly to the applied S4ULs / C4SL.

SOM testing was undertaken on 2 No. samples with recorded values of 1.10% and 1.80%. As such, adoption of criteria on the basis of a 1% SOM has been undertaken as a conservative approach.

The most sensitive human health receptors on-site are considered to be future residents within the apartment buildings. No formal residential gardens are proposed therefore the home grown vegetables or fruit pathways is excluded. Laboratory results have been compared against assessment criteria derived for this land-use. Should any future development plans vary from this end use, the contaminated land risk assessment should be revised.

Individual laboratory results are presented in the LMB Factual Report (**Appendix B**). A summary of soil laboratory test results in comparison to assessment criteria is included within **Appendix C** of this report.

Soil Contamination

Utilising the assessment criteria discussed above, the results of the 8 No. samples sent for geoenvironmental soil laboratory analysis have been compared against the relevant assessment criteria. **Table 12** provides a summary of human health exceedances with respect to a Residential (without plant uptake) end use at 1 % SOM.

Table 12 – Summary of Human Health Screening Criteria Exceedances

EXPLORATORY HOLE	SAMPLE DEPTH (m bgl)	CONTAMINANT	GAC (mg/kg)	CONCENTRATION (mg/kg)
		Dibenzo(a,h)anthracene	0.31	0.54
BH101	0.80	Arsenic	40	65
		Lead	310	440
TP101	0.80	Total Cyanide	34 (free)	120
		1,3-Dichlorobenzene	0.44	0.5
TP102	0.40	Asbestos	Detection	Chrysotile, 0.005%
TP103	0.50	Asbestos	Detection	Chrysotile, 0.008%

An exceedance above the relevant screening criteria with regard to arsenic has been recorded within a sample recovered from BH101 at a depth of 0.80m bgl. Additionally within this sample, the concentrations of Dibenzo(a,h)anthracene and lead have been recorded above the relevant screening criteria. The exceedances are likely linked to the 'black ashy material' described in the borehole log to be present between 0.7m and 1.7m bgl.



An exceedance of the relevant screening criteria with regard to total cyanide has been recorded within a sample recovered from TP101 at a depth of 0.80m bgl. Additionally, concentrations of 1,3-Dichlorobenzene have been recorded to exceed the relevant screening criteria within this sample. The TP101 trial pit log reports black colouration between 0.6 and 0.9m bgl. The exceedances are likely linked to historic contamination of the shallow ground.

Asbestos has been detected within two samples, recovered from TP102 at a depth of 0.40m bgl and TP103 0.50m bgl. Subsequent asbestos quantification has confirmed the presence of chrysotile asbestos at concentrations of 0.005% and 0.008% respectively. The presence of further asbestos across the site cannot be ruled out.

According to the proposed development plans (presented in **Appendix A**), the site encompasses the residential building footprints and hardstanding therefore direct contact between impacted soils and future site users / maintenance workers is considered unlikely in these areas. In areas of proposed soft landscaping, in proximity to the impacted soils, the placement of an engineered environmental capping layer formed from imported, chemically 'clean' soils should act as a pathway break between impacted soils and the future site user. An engineered anti-dig layer comprising a high visibility geotextile separator or geogrid should also be incorporated within the capping layer, within soft landscaped areas. Appropriate maintenance regimes should be in place to ensure that this capping layer remains functional. In addition, its presence should be recorded within the health and safety file for the site.

During excavation and other earthwork activities as part of the future development, adequate dust suppression and stockpile management is anticipated to mitigate likely dust generation which may migrate onto neighbouring residential and commercial properties, particularly given the presence of asbestos.

On completion of the remedial works it is considered that the risks to future site users, maintenance workers and third party neighbours from impacted soils on-site are considered to be **Low**.

9.3 CONTAMINATION IN SOILS - RISKS TO DRINKING WATER SUPPLY PIPES

A preliminary assessment of the recorded soil concentrations against the UKWIR threshold concentrations for PE or PVE pipes (the water supply pipes most susceptible to the migration of contaminants migrating through the pipe material) has been undertaken. The results indicate that concentrations of PAHs and mineral oil across the site are above the threshold values for PE and PVC water piping. It should be noted that the laboratory limit of detection for phenols and also, cresols and chlorinated phenols is recorded to be elevated above the UKWIR threshold for PVC pipe. Given that this pipe material was deemed unsuitable due to elevated PAH concentrations exceeding the UKWIR thresholds, it was not considered necessary to undertake any additional testing in relation to phenols, cresols and chlorinated phenols. In addition, pH and electrical conductivity values recorded indicate that the use of wrapped steel and copper pipe material may not be suitable and therefore specialist drinking water supply pipes, such as barrier pipes (PE-AI-PE), may be required in the future development.

9.4 CONTROLLED WATERS - RISK TO GROUNDWATER

A series of groundwater samples were collected, and screened for the contaminants stated in Table 11. Chemical laboratory analytical results from the investigation have been screened against controlled



waters generic assessment criteria (GAC) selected in view of the site setting where the River Terrace Gravel Member is classified by the EA as Secondary (A) Aquifer.

The generic assessment criteria have been selected to be protective of aquifer resource potential. Whilst Drinking Water Standards (DWS) have been used for the assessment criteria with respect to aquifer protection it is recognised that these are overly stringent as the closest groundwater abstraction, located 920m west, is used for irrigation purposes. Where UK Drinking Water Standards are not available for specific determinands, World Health Organisation (WHO) guidelines values were adopted.

Overall concentrations were found to comply with the screening criteria, however limited exceedances of manganese and antimony were noted in boreholes BH101, BH102 and BH103.

Table 13 - Summary of EQS Exceedances

LOCATION	ANALYTE	DWS (ug/l)	CONCENTRATION (ug/l)
BH101	Manganese	50	93
BH102	Manganese	50	96
BH103	Manganese	50	380
BH104	Antimony	5	23

Manganese concentrations are highest in borehole BH103 and dissipate quickly across the site based on a north easterly groundwater flow direction. Antimony exceedance was noted in one borehole only and does not appear to be widespread across the site.

Based on the above, it is considered that the site poses a **LOW risk** to the shallow groundwater within the Kempton Park Gravel Member.

9.5 GROUNDWATER VAPOUR RISK

Groundwater data obtained during this investigation was assessed against residential end-use groundwater vapour criteria set out in SoBRA's Development of Generic Assessment Criteria for Assessing Vapour Risks to Human Health from Volatile Contaminants in Groundwater Version 1, dated February 2017. Concentrations of contaminants above the assessment criteria were not identified. Therefore risks to future residents, maintenance workers and buildings from volatile vapours originating from impacted groundwater are considered to be **Low**.

9.6 GROUND GAS RISK ASSESSMENT

3 No. rounds of ground gas monitoring were undertaken between 5th and 26th May 2021 by LMB, a full record of which is provided in the LMB Factual Report (**Appendix B**). The available results have been assessed using the methods described in CIRIA Report C665 and BS8485:2015 +A1: 2019. Atmospheric pressures are presented in **Table 14** below:



Table 14 – Atmospheric Pressure Summary

MONITORING ROUND	ATMOSPHERIC PRESSURE RANGE (m bar)	PRESSURE TREND
5 th May 2021	1009	Steady
13 th May 2021	1006 to 1004	Falling
26 th May 2021	1015	Steady

Ground gas monitoring results are summarised in **Table 15** below:

Table 15 - Ground Gas Summary

MONITORING WELL	STRATUM MONITORED	MAX FLOW RATE (I/hr)	MAX METHANE (% v/v)	MAX CARBON DIOXIDE (% v/v)	METHANE GSV* (I/hr)	CARBON DIOXIDE GSV# (I/hr)
BH101	MG / RTC	0.1*	0.1*	4.80	0.0001	0.0048
BH102	MG / RTC	0.1*	0.1*	1.90	0.0001	0.0019
BH103	RTC	0.1*	0.1*	3.30	0.0001	0.0033
BH104	MG	0.1*	0.1*	0.20	0.0001	0.0002
BH105	RTC	0.1*	0.1*	0.10*	0.001	0.0001

MG – Made Ground, RTD – River Terrace Deposits, LC – London Clay Formation

#GSV = Gas Screening Value, in accordance with BS 8485:2015

In accordance with BS8485:2015 +A1:2019, the hazardous Gas Screening Value (GSV) is based on a 'worst-case' scenario whereby GSV equals the maximum hazardous gas concentration multiplied by the maximum flow rate recorded in any of the monitored boreholes. Based on the results in **Table 15**, GSVs for the site are below the threshold of 0.07 l/hr for Characteristic Situation (CS) 1 (**Very Low Risk**). Classification of the site as CS1 is also indicated by the maximum carbon dioxide concentration and methane concentration being below the limiting values of 5 % and 1 %, respectively.

Other ground-gas constituents, such as hydrogen sulphide, carbon monoxide, methane and PID (photo-ionisation detector, which measures VOCs) results were equally found at low levels (generally at or below detection limits). The low PID results are consistent with the absence of VOCs above detection limits in soil or groundwater samples analysed and, as such, risks from VOCs are considered to be **Very Low**.

RB17 Risk Assessment

To supplement the limited ground gas data available and to provide further lines of evidence, a RB 17 Ground Gas Risk Assessment was undertaken. CL:AIRE research bulletin RB17 - A Pragmatic Approach to Ground Gas Risk Assessment, dated November 2012 describes specific, practical aspects of research which have direct application in "real world environments". The purpose of the document is to provide a more robust assessment of the risk posed by the presence of ground gas, which is achieved by considering information and data that allows the likely gas generation from source to be estimated. The document proposes a pragmatic approach to ground gas assessment using data that can be collected quickly and at low cost.

^{*}detection limit of the monitoring equipment utilised.



RB17 and Section D within BS8485:2015 +A1:2019, state that where Made Ground soils are present, determination of the characteristic situation is based on the total organic carbon (TOC) content of the material, where a TOC content of <1% corresponds to a Characteristic Situation 1 scenario. Other aspects to consider are the age and the average thickness of the Made Ground. It was stated, soils that have been in place for 20 years or more are likely to have a low gassing potential and Made Ground with an average thickness of 3m and maximum thickness of 5m are unlikely to generate significant quantities of ground gas.

The historical map review indicated the site was in its current day layout by 1999 with no discernible changes to the site since. This would suggest that soils have been in place for more than 20 years and their gassing potential is likely low. A review of the logs indicate Made Ground soils on-site (not including surfacing) to have an average thickness of 1.20m. The Made Ground stratum across the site is therefore of insufficient thickness to generate significant ground gas volumes. Organic material has been recorded within the Made Ground deposits at two locations TP101 and TP102 which was noted on the logs as organic material at TP101 (0.60m to 0.90m bgl) and relict topsoil, wood and large roots at TP102 (0.70m to 1.40m bgl). TOC testing undertaken on the Made Ground samples collected were generally low (TOC < 1 %) with the exception of samples at BH101, 0.80m bgl (TOC of 5.0%) and TP101, 0.80m bgl (TOC of 1.1%). The sample taken from the Made Ground within BH101 was described as ashy slightly gravelly sand within the logs, which may explain the relatively high TOC concentrations. According to RB17 ash inclusions can give high TOC results that are not representative of the gas emissions risk. The sample taken from the Made Ground within TP101 was noted to include organic material within the logs at this depth. Therefore, a CS1 (**Very Low risk**) classification is considered to be applicable for the site following a qualitative RB17 risk assessment.

In addition to the above and as additional line of evidence, groundwater underlying the site was recorded at a relatively shallow depth, limiting the unsaturated zone through which soil gases migrate.

10.0 REVISED CONCEPTUAL SITE MODEL

The findings of the Fairhurst ground investigation have been used to update the preliminary conceptual site model discussed in **Section 3.0** of this report. This has been revised in the context of risks to Human Health (residential end use without plant uptake) and buildings based on the data gathered during the course of the investigation. The revised CSM is presented in **Table 16** below.



Table 16 – Updated Conceptual Site Model

	SOURCE				RISK ASSESSMENT (FOLLOWING CIRIA C552)	T (FOLLOWING C	IRIA C552)	
ORIGIN	CONTAMINANT OF CONCERN	ZONE AFFECTED	РАТНМАУ	HAZARD TO RECEPTOR	CONSEQUENCE	PROBABILITY	RISK	COMMENT ON HAZARD REALISATION
Made Ground	Arsenic, Lead, Dibenzo(a,h)ant hracene in borehole BH101, Total cyanide and 1,3- Dichlorobenze	Site-wide	Direct / dermal contact, ingestions and inhalation of soil bound	Health impact to future residents, maintenance workers and third party	Medium	Unlikely	Low	Impacted Made Ground soils were recorded across the site. Exposure to future residents are limited to areas of soft landscaping. However due to the site predominantly comprising building cover and hardstanding and the anticipated import of chemically clean' soils within the limited soft landscaped areas providing a pathway break between underlying impacted soils and residents / maintenance workers at the surface, risks to future residents and maintenance workers were considered to be Low .
	TP101 and asbestos fibres in trial pits TP102 and TP103		dust.	neighbours.				Furthermore, during excavation and other earthworks activities as part of the future development, adequate dust suppression and stockpile management is anticipated to mitigate likely dust generation which may migrate onto neighbouring residential and commercial properties. Therefore risks to third party neighbours are considered to be Low.
Made Ground	PAHs and mineral oils	Made Ground on site	Direct contact with water supply pipes	Contaminant migration through pipe material and subsequent health impact to end-user	Medium	Unlikely	Low	Based on the ground conditions recorded, the use of barrier pipes is recommended. This approach should be agreed with the local water company.
Made Ground	Ground gas / Vapours	Site-wide	Migration into buildings, followed by inhalation of accumulated ground gases and / or soil vapours	Health impact to future residents and explosive risk to buildings.	Medium	Unlikely	Low	Assessment of the limited monitoring results indicated a Characteristic Situation CS1 (Very Low risk) for the site. A qualitative RB17 assessment also concludes a CS1 (Very Low risk). A vapour risk assessment was undertaken and concluded a low risk to site end-users.
Risk Ratings.								

Risk Ratings:

- High The available information indicates a significant possibility of harm to a receptor requiring further investigation, assessment or treatment.
- Moderate The available information indicates a potential for significant harm to a receptor requiring further investigation and assessment.
- Low The available information does not indicate a significant potential for harm to a receptor requiring further investigation. This does not indicate zero risk.



11.0 CONCLUSIONS AND RECOMMENDATIONS

11.1 GENERAL

The current proposed development comprises the demolition of the current retail units and the development of 4 No. residential buildings, permeable paving, soakaway drainage and soft landscaping. Building A is a part 3, part 4, part 7 and 8 storey building, Building B is an 11 storey building, Building C is a part 8 and 10 storey building and Building D is a part 4, part 7 and part 8 storey building. In addition, a basement plant room is proposed beneath Building A. The basement will be approximately 3 m below the proposed ground-floor level, located near the centre of the proposed building and covers a smaller area than the above-ground structure. It is envisaged to be used as a plant room. A temporary sheet pile wall will be required to facilitate the construction of the basement.

Ancillary structures including the bin store and external masonry walls are proposed in the eastern part of the site, which are likely to be founded on shallow footings. The external areas will be at similar levels as existing ground.

11.2 GROUND AND GROUNDWATER CONDITIONS

The intrusive ground investigation undertaken generally confirmed the anticipated ground conditions based on the British Geological Society (BGS) mapping.

The ground investigation indicated that the ground conditions on-site comprise hardstanding over Made Ground with a maximum thickness of 2.14m, underlain by the Kempton Park Gravel Member, with a maximum proven thickness of 5.30m. The Kempton Park Gravel Member was recorded to be underlain by the London Clay Formation; the base of this strata was not proven in this investigation.

The post-fieldwork groundwater monitoring results record groundwater to be encountered within the Kempton Park Gravel Member from 2.32m to 4.68m bgl. However, the potential for groundwater seepage in the sandy layers and claystone bands at greater depths within the London Clay Formation should not be discounted.

11.3 GEOTECHNICAL ASSESSMENT

The site is within a high risk area with respect to unexploded ordnance. An UXO specialist watching brief is required for any below ground works.

Although limited geotechnical testing was undertaken on the Made Ground, the material is anticipated to be variable in nature and comprise predominantly low strength material. As such, it is not considered to be a suitable founding strata. Preliminary geotechnical laboratory testing suggests that the granular deposits would be classified as Class 1A/B material for re-use as general fill, in accordance with Series 600 Specification for Highway Works, though these deposits are likely to require drying out to permit their placement at or close to the optimum moisture content.

With regard to the superficial deposits, these belonged to the Kempton Park Gravel Member. The cohesive deposits would be expected to have an undrained shear strength in the order of 50 kPa. The granular deposits would be expected to support an Allowable Bearing Capacity of in the order of 175 kN/m².



The London Clay Formation has been encountered as high to very high strength, with undrained shear strengths typically in excess of 100 kPa.

Cohesive Made Ground and the London Clay Formation have been determined to have a low to high volume change potential in accordance with NHBC Standards Chapter 4.2 (2017). Any proposals for new vegetation near structures should be reviewed in order to assess the risk of seasonal shrink/swell movement in cohesive soils leading to differential settlement. Should any ground-bearing or shallow foundations be proposed, reference should be made to the NHBC recommendations regarding the minimum foundation depths required in shrinkable soils.

Given the proposed development and the anticipated high magnitude loading, non-standard foundations in the form of piled foundations would be a feasible foundation solution, with the piles terminated within the London Clay Formation. Based on the ground conditions encountered on site and its proximity to nearby structures, rotary bored or continuous flight auger (CFA) piles are considered to be the most suitable pile types. However, consideration should be given to the potential presence of mudstone bands within the London Clay Formation and shallow groundwater ingress. Advice should be obtained from specialist piling contractors in terms of whether the higher torque CFA rigs would be able to penetrate the identified mudstone bands at the depths where these have been identified to be present.

Ancillary structures including the bin store and external masonry walls are proposed in the eastern part of the site, which are likely to be founded on shallow traditional footings.

The anticipated basement depth is 3m bgl, which is within the Kempton Park Gravels and beneath the resting groundwater table; however, there may be further excavation beneath this depth for proposed foundations. Close side support (temporary shoring) and appropriate dewatering measures may be required during the excavation of the basement and service corridors. It is understood that a temporary sheet pile wall will be installed to facilitate the construction of the basement which should provide groundwater cut-off.

Based on the cohesive and low permeability nature of the ground conditions on site and the expected shallow groundwater levels, the use of soakaways is unlikely to be feasible.

Council correspondence notes that the site is within a National Grid safeguard zone (unknown if this relates to the former Richmond Gas Works) and a possible Southern Gas Network structure was identified in the south-eastern corner of the site. Furthermore, existing utilities are likely to be present on-site associated with the existing development, including the electrical substation.

Buried obstructions such as existing foundations of the buildings on-site are likely to be encountered during construction. Contingency measures should be in place should they impact on the design and construction of the foundations and basement excavation. The nature of below ground structures associated with existing structures, which are in uninvestigated areas of the site, is unknown and requires further supplementary investigation.

It is recommended that a concrete classification of DS-3 AC-3 should be specified in the design and construction of substructures.

11.4 GEO-ENVIRONMENTAL ASSESSMENT

The purpose of the geo-environmental assessment was to confirm the findings of a Preliminary Risk Assessment carried out by Fairhurst and develop the conceptual site model.



Potential on-site sources of contamination included Made Ground, a former car wash, historical uses including timber yard, crane, railway sidings, fuel depot and coal hoppers and an electrical substation. Various off-site sources including, but not limited to, railway sidings and the Richmond Gas Works were also identified.

Laboratory results of soil samples were compared against residential generic assessment criteria (residential without plant uptake). Exceedances of the relevant GAC were recorded for arsenic, lead and Dibenzo(a,h)anthracene within a sample recovered from BH101 at a depth of 0.80m bgl and exceedances of the relevant GAC were recorded for total cyanide and 1,3-Dichlorobenzene within a sample recovered from TP101 at a depth of 0.80m bgl. Furthermore, asbestos has been detected within the two samples tested recovered from TP102 at a depth of 0.40m bgl and TP103 0.50m bgl. Subsequent asbestos quantification has confirmed the presence of chrysotile asbestos at concentrations of 0.005% and 0.008% respectively.

Due to the site predominantly comprising building cover and hardstanding direct contact between impacted soils and future site users / maintenance workers is considered unlikely in these areas. In areas of proposed soft landscaping the placement of an engineered environmental capping layer formed from imported, chemically 'clean' soils should act as a pathway break between impacted soils and the future site user. An engineered anti-dig layer comprising a high visibility geotextile separator or geogrid should also be incorporated within the capping layer, within soft landscaped areas. Appropriate maintenance regimes should be in place to ensure that this capping layer remains functional. In addition, its presence should be recorded within the health and safety file for the site.

On completion of the remedial works it is considered that the risks to future site users, maintenance workers and third party neighbours from impacted soils on-site are considered to be **Low**.

Furthermore, during excavation and other earthworks type activities as part of the future development, adequate dust suppression and stockpile management is anticipated to mitigate likely dust generation which may migrate onto neighbouring residential and commercial properties. Therefore risks to third party neighbours are considered to be **Low**.

Groundwater samples analysed from across the site were largely compliant with UKDWS with limited exceedances for manganese and antimony noted in boreholes BH101, BH102, BH103 and BH104. Given the stringent nature of the assessment criteria selected, the localised nature of antimony exceedance coupled with a likely off-site source of manganese, it is considered that the site poses a **Low risk** to the shallow groundwater within the Kempton Park Gravel Member.

Assessment of the ground gas monitoring results indicated a Characteristic Situation (CS) 1 (**very low risk**) for the site. This was confirmed by an RB17 assessment. An assessment of concentrations of volatile contaminants within groundwater samples indicated a **Low risk** to future residents, maintenance and buildings from volatile contaminants in shallow perched water.

A preliminary assessment of the recorded soil concentrations against the UKWIR assessment criteria for PE or PVE pipes (the water supply pipes most susceptible to the migration of contaminants migrating through the pipe material) has been undertaken. The results indicate that concentrations of PAHs and mineral oil across the site are above the threshold values for PE and PVC water piping. In addition, pH and electrical conductivity values recorded indicate that the use of wrapped steel and copper pipe material may not be suitable and therefore specialist drinking water supply pipes, such as barrier pipes (PE-AI-PE), are recommended in the future development.



11.5 RECOMMENDATIONS

Based upon the results of the investigation the following recommendations are made:

- The inspection of asbestos registers of the buildings is recommended to ensure all asbestos in buildings is removed by a licensed asbestos contractor prior to demolition. Asbestos clearance certificates should be held on file;
- The possibility of processing and re-using demolition material on site under the WRAP protocol should be explored;
- Fairhurst to provide a watching brief during all earthworks and removal of the existing slabs;
- It is anticipated that a Remediation Strategy will be required for the proposed development, which will comprise the following;
 - o In areas of proposed soft landscaping, an engineered environmental capping layer formed from imported, chemically 'clean' soils should act as a pathway break between impacted soils and the future site user. An engineered anti-dig layer comprising a high visibility geotextile separator or geogrid should also be incorporated within the capping layer, within soft landscaped areas. Appropriate maintenance regimes should be in place to ensure that this capping layer remains functional. In addition, its presence should be recorded within the health and safety file for the site;
 - The use of barrier pipes for drinking water supply is recommended and should be agreed with the local water company prior to installation;
 - Based on the ground gas risk assessment, the site is classed as a Characteristic Situation (CS)1 (very low risk) site, where no gas protection measures are required;
 - Risks from volatiles in soils and shallow groundwater were classed as low, no special protection measures are required;
 - A concrete classification of DS-3 AC-3 should be specified in the design and construction of substructures.

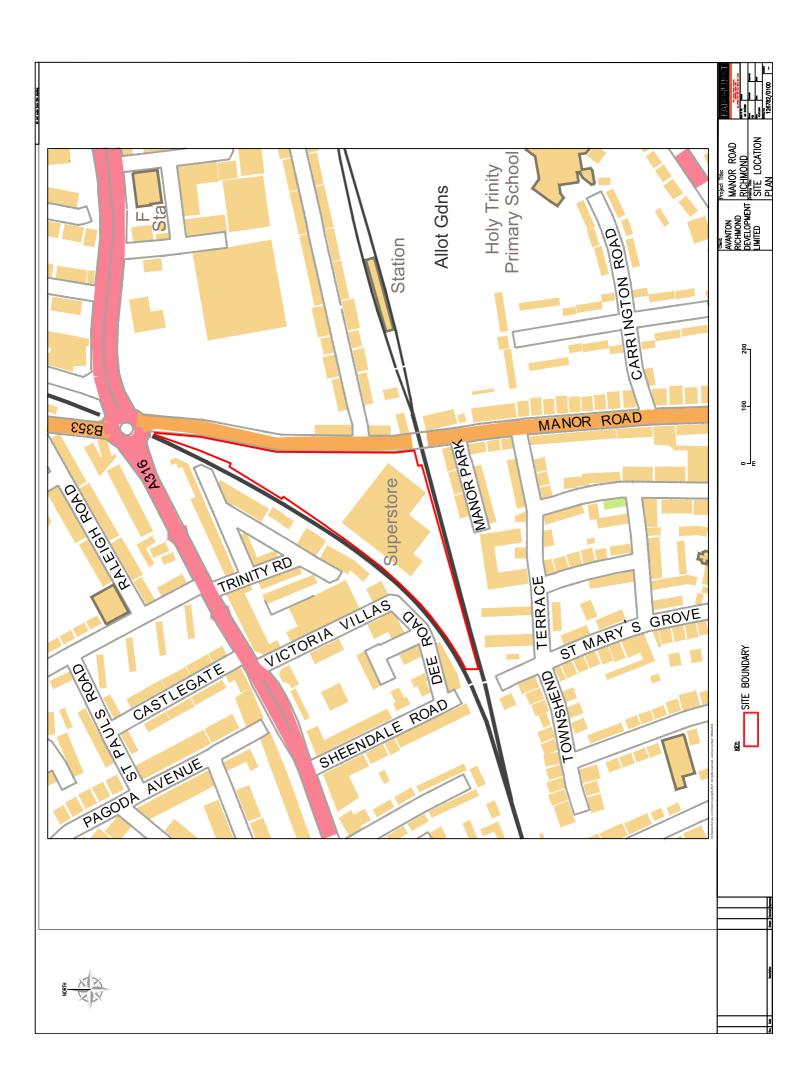
A second phase of ground investigation is planned once the buildings and associated structures have been demolished to supplement the existing data and should include, but not be limited to, the following;

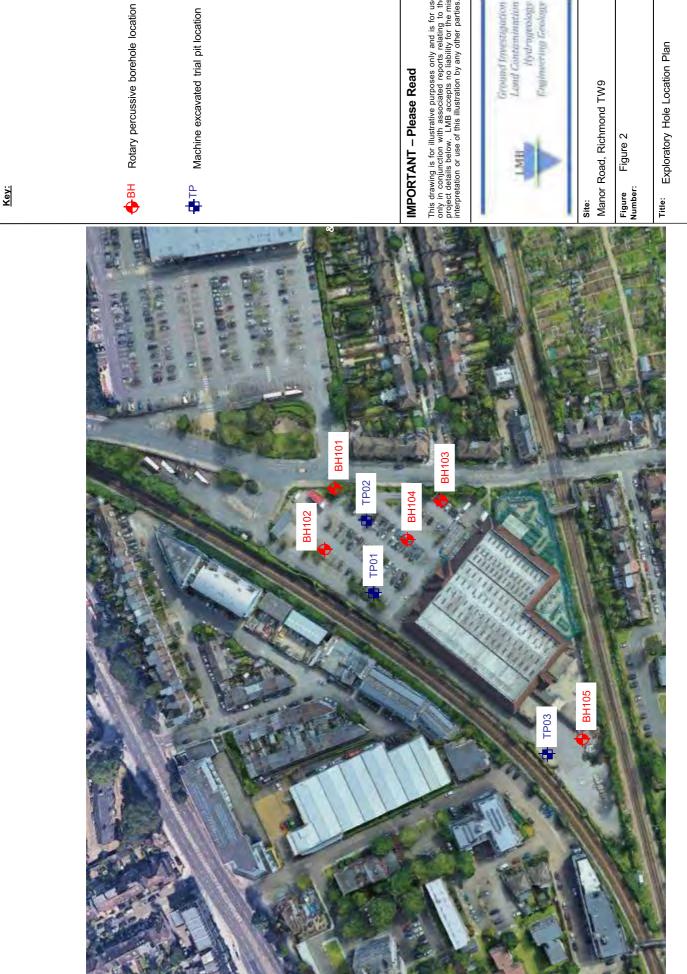
- Additional Cable Percussive boreholes (to depths of c. 25m) and Window Sample boreholes (to depths of c. 5m) in previously inaccessible areas;
- Trial pitting along the eastern boundary of the site to investigate the existing retaining wall along
 Manor Way and across the site to identify the location of services entering the site;
- Installation of gas and groundwater monitoring wells and 3 No. additional return monitoring rounds to confirm the low ground gas risk and groundwater risk;
- Additional geo-environmental sampling and testing to inform any subsequent contamination remediation measures;
- Additional geotechnical in-situ and laboratory testing for design.

Prior to the supplementary ground investigation the Local Planning Authority should be consulted and any associated Planning Conditions reviewed against the proposed scope of investigation.



APPENDIX A - FIGURES





Machine excavated trial pit location

This drawing is for illustrative purposes only and is for use only in conjunction with associated reports relating to the project details below. LMB accepts no liability for the misinterpretation or use of this illustration by any other parties.



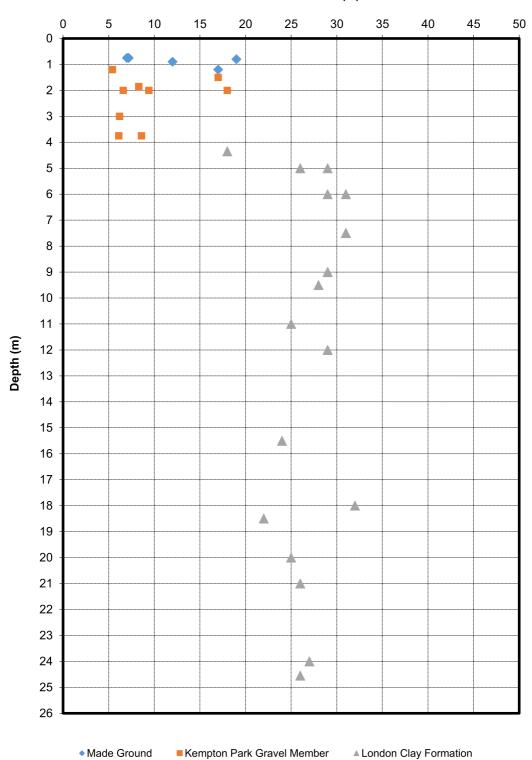
Date: May 2021 Created By: PIL Project No:

Client: Avanton Richmond Development Ltd

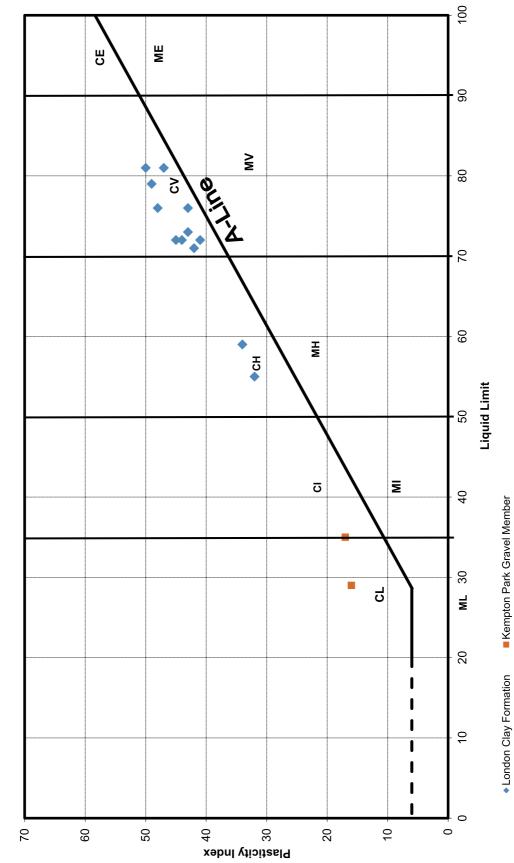


Moisture Content vs Depth Plot for all Strata

Moisture Content (%)

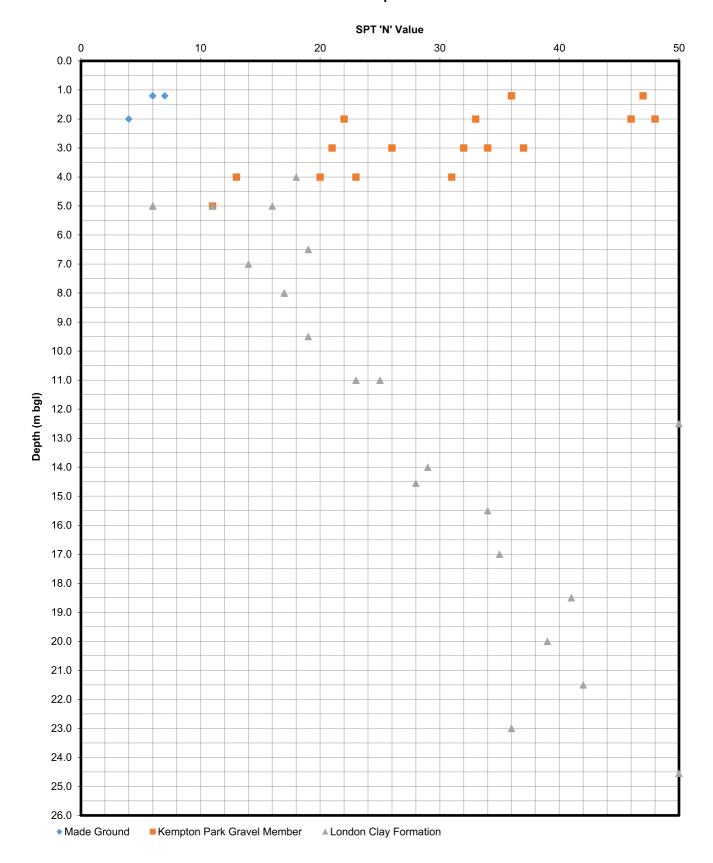


Plasticity Chart for all Strata





SPT 'N' Value vs Depth Plot for All Strata





APPENDIX B – LMB FACTUAL REPORT



LMB GEOSOLUTIONS LTD

FACTUAL REPORT FOR GROUND INVESTIGATION 84 MANOR ROAD, RICHMOND TW9

June 2021

DOCUMENT RECORD

Document Title	Factual Report for Ground Investigation
Site	84 Manor Road, Richmond TW9 1YB
Document Date	21st June 2021
Document Version	Issue 2: Updated following comments from Fairhurst
Document Authorisation	Philip Lewis BSc (Hons), MSc, CGeol, FGS
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Company No. 8303397

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INTRODUCTION

Introduction

AUTHORISATION

LMB Geosolutions Ltd (LMB) was instructed by Fairhurst (Consultant Engineers) on behalf of Avanton Richmond Development Ltd (the Client) to undertake ground investigation works to support the proposed development at 84 Manor Road, Richmond TW9 1YB (the Site).

PROJECT AND SITE DETAILS

Site Address	84 Manor Road, Richmond TW9 1YB. A Site Location Plan is provided as Figure 1 .
Site/Building Area	The site currently comprises an operational retail park that includes Homebase, Pets at Home, We Buy any Car along with car parking and a rear service yard.
Proposed Development	Information provided by the Consultant Engineers indicates that the site is to be redeveloped to include residential apartment buildings with a basement car park, commercial areas, soft landscaping and infrastructure.

SCOPE OF WORKS

The scope of works has been completed:

- Site set up including liaison with the Client and appointment of sub-contractors;
- Mobilisation to site and transport of the rigs and plant to the proposed locations;
- Completion of a service avoidance, level and positioning survey at proposed exploratory hole locations:
- Concrete coring/breaking at proposed locations;
- Completion of five rotary percussive borehole to depths of between 7.45m and 25.00m below ground level (bgl) with insitu SPT and collection of disturbed and undisturbed samples for laboratory testing;
- Completion of three machine excavated trial pits to depths of 2.50m bgl with collection of disturbed samples for laboratory testing;
- Completion of three infiltration soakaway tests within trial pit locations in general accordance with BRE 365;
- Installation of monitoring wells within all borehole locations to depths of between 6.00m and 8.00m bgl;
- Field headspace screening of soil samples using a photo-ionisation detector (PID);
- Supervision and geological logging (in general accordance with BS 5930) by an appropriately experienced geo-environmental engineer;
- Well development and groundwater sampling using low flow techniques;

INTRODUCTION

- Return monitoring of groundwater and ground gas levels on 6no. occasions¹;
- Geotechnical and chemical laboratory testing of soil and groundwater samples;
- Completion of a factual report that will include;
 - Details of the ground and groundwater conditions encountered.
 - Geological logs (AGS format if required) and foundation sections.
 - Presentation of chemical analytical.
 - Presentation of geotechnical laboratory testing results.

PUBLISHED GEOLOGY

Reference to the relevant British Geological Survey map for the area and information provided by the Consultant Engineers indicates that the ground conditions at the site comprise Made Ground overlying the Kempton Park Gravel Member which in turn overlies the London Clay Formation.

LIMITATIONS

LMB has prepared this report solely for the use of the named Client and those parties with whom a warranty agreement and/or assignment has been agreed. Should any third party wish to use or rely upon the contents of the report, written approval must be sought from LMB and the Client.

LMB accepts no responsibility or liability for:

- a) the consequences of this document being used for any purpose or project other than for which it was commissioned, and
- b) issue of this document to any third party with whom an agreement has not been executed.

The information provided, among other things, take in to consideration currently available guidance and best available techniques. No liability can be accepted for the retrospective effects of any future changes or amendments to best available techniques.

¹ This version of the report includes 3no. rounds and an updated report will be issued following completion of the full 6no. rounds.

Ground Investigation

INTRODUCTION

The ground investigation works were undertaken between 21st April and 27th April 2021 and comprised the progression of 5no. rotary percussive boreholes and 3no. machine excavated trial pits, with sampling of soil for laboratory testing and geological logging.

An exploratory hole location plan produced by the Consultant Engineers is provided as Figure 2.

Ground gas and groundwater monitoring was undertaken following completion of the fieldworks between 5^{th} May and 26^{th} May 2021.

Monitoring well development was completed on 29^{th} April 2021 and groundwater sampling was completed on 5^{th} May 2021.

Details of the ground investigation completed, along with the findings of the investigation, are provided in the following sections. The exploratory hole logs laboratory results and monitoring results are presented in **Appendix A, B, C** and **D** respectively.

Guidance Documents

Details of the best practice guidance documents and reference information used in undertaking the ground investigation and assessment are provided at the end of this report (see REFERENCES & GUIDANCE).

Investigation Strategy

The ground investigation was designed based on the requirements and specification from the Consultant Engineers. All works and exploratory holes were supervised and logged by an appropriately experienced geoenvironmental engineer.

FIELDWORKS

Concrete Breaking & Coring

Hard surfacing and concrete surfacing were removed from exploratory hole locations with the use of a concrete coring rig and/or a hydraulic breaker.

Rotary Percussive Boreholes

Five rotary percussive boreholes were completed to depths of between 7.45 and 25.00m bgl. Environmental, disturbed and/or bulk samples were generally collected at regular intervals.

Standard Penetration Tests (SPTs) were generally completed at alternate 1.0m intervals in the upper 5.0m and at alternating 1.50m thereafter. Undisturbed samples were generally collected at alternate1.0m intervals in the upper 5.0m and at alternating 1.50m thereafter where cohesive soils were encountered.

In Situ Tests

Standard Penetration Tests (SPTs) were carried out in the rotary percussion boreholes in accordance with BS EN ISO 22476-3. A calibration certificate for the SPT hammer is provided in **Appendix E**.

Machine Excavated Trial Pits

Three machine excavated trial pits were completed to depths of 2.50m bgl using a 3cx excavator. Environmental and disturbed samples were generally collected at regular intervals for laboratory testing.

An exploratory hole location plan provide by the consultant engineers is presented as Figure 2.

Soakaway Testing

Following excavation, soakaway testing was completed in general accordance with BRE 365 (2016). The trial pits were surcharged with water from a bowser and the fall in depth of water with time recorded. Due to the relatively slow rate of infiltration repeat tests were not feasible.

The infiltration test data was assessed in accordance with the methodology outlined in BRE Digest 365 (2016): Soakaway Design. This document describes 'design and construction procedures for soakaways explains how to calculate rainfall design values and soil infiltration rates and gives design examples.'

The methodology applied in this assessment specifically relates to the calculation of soil infiltration rate within the unsaturated zone.

None of the soakaway infiltration tests proceeded through the required 75% to 25% effective depth. The soakaway infiltration test worksheets are provided in **Appendix F**.

Field Headspace Testing

Field screening of soil samples was undertaken using a *mini-rae Lite* photo ionisation detector (PID). Soil arisings were collected at regular intervals and placed in sealed plastic bags, agitated by shaking and left for a minimum of 30 minutes prior to testing of 'headspace' using the PID.

Details are provided on the exploratory hole logs presented in **Appendix A** and calibration certificates are provided in **Appendix G**.

Groundwater Sampling

Groundwater samples were collected from all monitoring wells. All wells were developed using an electrical submersible pump, washed and cleaned between holes on 29th April 2021.

Monitoring wells were sampled on 5^{th} May 2021 using low flow sampling techniques with a peristaltic pump so groundwater was micro-purged and directed via a flow cell installed with a multi-parameter probe so that indicator parameters (i.e. pH, EC, ORP, DO and temperature etc) could be monitored to ensure stability prior to sampling into dedicated sampling containers.

Low flow sampling indicator parameters are provided in **Appendix D**.

Monitoring

Three rounds of monitoring have been completed at the site. Ground gas monitoring was undertaken using a GFM 436 gas analyser, volatile vapour monitoring was completed using a mini-rae Lite PID.

Calibration certificates are provided in Appendix G.

Visual and Olfactory Evidence of Contamination

Observations of potential visual and/or olfactory evidence of contamination observed during the fieldworks were limited to the following:

- BH01 (0.70-1.70m) black ashy material and clinker.
- BH02 (4.00m) grey colouration and hydrocarbon odour.
- BH04 (2.20-4.30m) strong hydrocarbon odour.
- TP01 (0.60-0.90m) black colouration

However, Made Ground was observed in all exploratory hole locations and can be indicative of the presence of contamination.

LABORATORY TESTING

Soil and groundwater samples were submitted to the UKAS and MCERTS accredited laboratories of i2 Analytical for chemical and geotechnical testing.

Laboratory test results are provided in **Appendix B** and **C**.

SUMMARY OF GROUND & GROUNDWATER CONDITIONS

Ground Conditions

The table below provides a summary of ground conditions encountered with full descriptions provided in the associated exploratory hole logs provided in **Appendix A**:

Strata	Depth Range to Top (m bgl)	Depth Range to Base (m bgl)
Made Ground	Ground Level	0.65 – 2.20
Kempton Park Gravel Member ⁽¹⁾	0.65 – 2.20	4.30 - 7.00
London Clay Formation (2)	4.30 - 7.00	7.45 – 25.00

- (1) Base not determined in all locations.
- (2) Base not determined.

Groundwater Observations

During the ground investigation works water strikes were recorded at the following depths:

Location	Depth to Water Strike (m bgl)
BH101	4.50
BH102	4.30
BH103	3.50
BH104	3.20
BH105	3.00

During return monitoring groundwater was recorded at depths of between 1.57m and 4.24m bgl.

Details are provided on the exploratory hole logs presented in $Appendix\ A$ and the monitoring results presented in $Appendix\ D$.

MONITORING AND INSTRUMENTATION

Dual groundwater and ground gas monitoring wells were installed in all borehole locations. Details of the monitoring well installations can be viewed on the exploratory hole logs in **Appendix A**.

REFERENCES & GUIDANCE

REFERENCES & GUIDANCE

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- 18. Environment Agency (2004), Model Procedures for the Management of Land Contamination, September 2004, ISBN: 1844322955;
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- 21. Environment Agency (2009b), CLEA Software (Version 1.04) Handbook (and Software), Report SC050021/SR4, January 2009;
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 $^{^2}$ This document has been withdrawn but is considered to remain useful in proving technical background for designing ground investigation works.

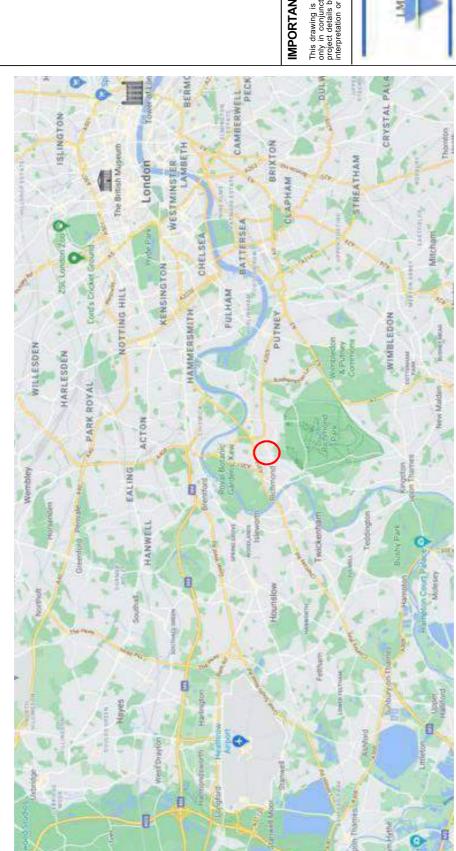
 $^{^3}$ This document has been withdrawn but is considered to remain useful in proving technical background for designing ground investigation works.

REFERENCES & GUIDANCE

- 23. Environment Agency (2009d), A Review of Body Weight and Height Data Used in the CLEA Model, Report SC050021/Final Technical Review 1, January 2009;
- 24. Nathanial et. al., (2009), The LQM/CIEH Generic Assessment Criteria for Human Health Risk Assessment (2nd edition), Land Quality Press, Nottingham, ISBN 0-9547474-7-X
- 25. USEPA (2004), User's Guide for Evaluating Subsurface Vapour Intrusion into Buildings
- 26. Environment Agency (2013). Groundwater Protection: Principles and Practice (GP3)
- 27. Water Framework Directive (2000/60/EC)
- 28. Groundwater Regulations (2009).
- 29. Drinking Water Quality Standards England & Wales 2000 (Amended 2004, DWS).
- 30. World Health Organisation (WHO) Petroleum Products in Drinking Water.
- 31. Environmental Quality Standards (EQS). The River Basin Districts Typology, Standards and Groundwater Threshold Values (Water Framework Directive) (England and Wales) Directions 2010.
- 32. Environment Agency (2006). Remedial Targets Methodology. Hydrogeological Risk Assessment for Land Contamination.
- 33. Environment Agency (2013). Technical Guidance WM2 (v3). Interpretation of the definition and classification of hazardous waste.

FIGURES

FIGURES



IMPORTANT - Please Read

Approximate Site Location

Key:

This drawing is for illustrative purposes only and is for use only in conjunction with associated reports relating to the project details below. LMB accepts no liability for the misniterpretation or use of this illustration by any other parties.



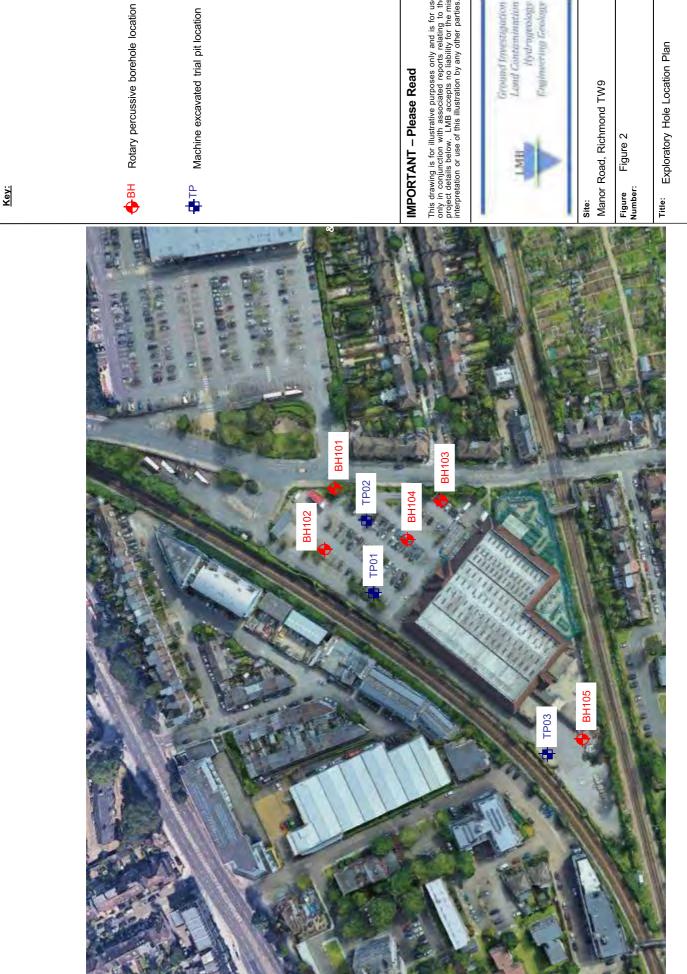
Manor Road, Richmond TW9 Site:

Figure 1 Figure Number: Title:

Site Location Plan

May 2021 Date: Created By: PIL Project No:

Client: Avanton Richmond Development Ltd



Machine excavated trial pit location

This drawing is for illustrative purposes only and is for use only in conjunction with associated reports relating to the project details below. LMB accepts no liability for the misinterpretation or use of this illustration by any other parties.



Date: May 2021 Created By: PIL Project No:

Client: Avanton Richmond Development Ltd

APPENDICES

Appendices

APPENDIX A EXPLORATORY HOLE LOGS

Percussion Drilling Log Project Name: Manor Road Client: Avanton Richmond Development Ltd Date: 21/04/2021 Location: 84 Manor Rd. Richmond TW9 Contractor: SADS Co-ords: E518965.60 N175513.60 Crew Name: Project No.: LMB_Manor **Drilling Equipment:** Borehole Number Hole Type Level Logged By Scale Page Number BH101 RO 6.70m AoD PIL 1:50 Sheet 1 of 1 Sample and In Situ Testing Water Depth Level Legend Stratum Description Strikes (m) (m) Depth (m) Туре Results MADE GROUND: grass over brown slightly clayey 0.10 0.10 PID PID=0.20 sand with rootlets and occasional flint and rare brick 0.25 6.45 0.25 D 0.40 D MADE GROUND: grey to grey brown gravelly sand 0.40 ES concrete and occasional brick cobbles. 0.70 6.00 0.40 PID PID=0.20 MADE GROUND: grey to black ashy slightly gravelly 0.50 sand. Gravel sub-angular fine to coarse flint, brick, D 0.75 D clinker and glass. Rare brick cobbles. 0.80 D 0.80 ES 0.80 PID PID=0.20 1.00 D 1.70 5.00 Brown to orange brown sandy CLAY with occasional 1.20 D fine to medium flint gravel. (KEMPTON PARK 1.20 SPT N=6 (1,1/2,1,1,2) GRAVEL MEMBER). 1.50 ES 1.50 PID PID=0.40 1.85 D 1.90 ES 1.90 PID PID=0.30 2.00 3.00 3.70 2.00 SPT N=22 (4,5/5,5,6,6) 3 Brown to orange brown slightly clayey slightly gravelly 2.75 D to gravelly medium SAND. Gravel sub-angular to 3.00 D rounded fine to coarse flint (damp). KEMTPON PARK 3.00 - 4.00 В GRAVEL MEMBER). 3.00 SPT N=26 (4,5/6,6,7,7) 3.10 ES 3.10 PID PID=0.30 4 3.75 D 4.00 D 4.00 ES 4 50 2 20 4.00 SPT N=18 (4,5/5,5,4,4) Brown slightly gravelly medium to coarse SAND. 4.00 PID PID=0.40 Gravel sub-angular to rounded medium to coarse flint. 4.60 ES (KEMTPON PÄRK GRAVEL MEMBER) 4.60 PID PID=1.00 4.75 5.00 D 5.00 - 6.00 В 5.00 SPT N=16 (3,3/3,4,4,5) 6.00 D 6 6.00 - 7.00В 6.50 D 6.50 ES 6.50 SPT N=19 (3,4/4,4,5,6) 6.50 PID PID=0.20 7.00 -0.30 brown with orange brown mottling CLAY - recovered 7.05 ES 7.10 -0.40 with sand and rare flint gravel possibly pulled down from overlying granular deposits. (LONDON CLAY 7.05 PID PID=0.10 7.50 FORMATION) dark grey to grey brown CLAY. Closely fissured. 7.50 7.50 PID PID=0.10 (LONDON CLAY FORMATION). 8.00 8 8.45 -1.75 End of Borehole at 8.450m 9 10 Hole Diameter Chiselling Inclination and Orientation Casing Diameter Depth Base Depth Base Depth Top Depth Base Depth Top Depth Base Inclination Remarks

Percussion Drilling Log Project Name: Manor Road Client: Avanton Richmond Development Ltd Date: 21/04/2021 - 27/04/2021 Location: 84 Manor Rd. Richmond TW9 Contractor: SADS Co-ords: E518938.50 N175516.76 Project No.: LMB_Manor Crew Name: **Drilling Equipment:** Borehole Number Hole Type Level Logged By Scale Page Number BH102 RO 6.40m AoD PIL&DN 1:50 Sheet 1 of 1 Sample and In Situ Testing Water Depth Level Legend Stratum Description Strikes (m) (m) Depth (m) Type Results Brick paving. MADE GROUND: yellow brown slightly gravelly sand 0.06 6.34 0.15 0.13 6.27 0.15 PID PID=0.10 0.25 6.15 0.50 D MADE GROUND: red brown to grey gravelly sand / 0.50 ES sandy raodstone gravel (sub-base) 0.75 5.65 PID=0.30 0.50 PID MADE GROUND: grey to brown gravelly sand with 0.90 D concrete and occasional brick cobbles. Gravel sub-0.90 ES angular fine to coarse brick, flint and concrete. 1.20 5.20 PID=0.30 0.90 PID MADE GROUND: brown to dark brown clayey gravelly 1.20 D sand with occasional brick and concrete cobbles. SPT N=7 (1,1/1,1,4,1) 1.20 Gravel sub-angular fine to coarse flint, brick, concrete 1.60 4.80 ES 1.50 PID=0.40 1.50 PID MADE GROUND: dark brown clay with fine to medium 1.85 D brick and concrete gravel and slate fragments. 2.00 FS brown to orange brown gravelly medium SAND / 2.00 - 2.45 R sandy sub-angular to rounded fine GRAVEL. Gravel 2.00 SPT N=33 (6,4/8,8,9,8) sub-angular to rounded fine flint. (KEMPTON PARK PID 2.00 PID=0.30 GRAVEL MEMBER). 2.75 D 3.00 3 3.00 SPT N=21 (3,4/5,5,6,5) 3.75 D 4.00 - 4.45 В 4.00 2.40 brown to orange brown gravelly medium SAND with N=20 (3,4/5,4,5,6) 4.00 SPT occasional to frequent flint cobbles. Gravel sub-PID PID=18.60 4.00 angular to rounded fine to coarse flint. (KEMPTON 4 50 FS PARK GRAVEL MEMBER) PID=3.70 PID 4 50 4.0m grey colouration and hydrocarbon odour. 4.75 D 5.00 D 5 SPT N=11 (2,3/2,3,3,3) 5.00 PID 5.00 PID=0.80 PID PID=0.60 5.50 5.70 0.70 brown to orange brown gravelly medium to coarse 5.80 FS 5.70 0.70 SAND with occasional to frequent flint cobbles. Gravel PID=0.20 5.80 PID 5.80 0.60 sub-angular to rounded fine to coarse flint. (KEMPTON 6.00 PARK ĞRAVEL MEMBER). 4.0m grey colouration and hydrocarbon odour. 6.50 U brown with occasional orange brown mottling CLAY. (LONDON CLAY FORMATION). dark grey brown CLAY. Closely fissured with 7.00 occasional grey silty partings. (LONDON CLAY FORMATION). 7.00 N=14 (2,3/3,3,3,5) 7.45 -1.05 8 End of Borehole at 8,450m 9 10 Hole Diameter Chiselling Inclination and Orientation Casing Diameter Depth Base Depth Base Depth Top Depth Base Depth Top Depth Base Inclination Remarks

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Percussion Drilling Log Project Name: Manor Road Client: Avanton Richmond Development Ltd Date: 21/04/2021 - 23/04/2021 Location: 84 Manor Rd. Richmond TW9 Contractor: SADS Co-ords: E518946.14 N175473.06 Crew Name: Project No.: LMB_Manor **Drilling Equipment:** Borehole Number Hole Type Level Logged By Scale Page Number BH104 RO 6.25m AoD PIL 1:50 Sheet 1 of 3 Sample and In Situ Testing Depth Water Level Legend Stratum Description Strikes (m) (m) Depth (m) Туре Results Brick paving. MADE GROUND: yellow brown medium sand (sub-0.06 6.19 6.15 0.10 0.35 5.90 0.40 D MADE GROUND: red brown sandy roadstone gravel 0.40 ES 0.65 5.60 (sub-base) PID=0.20 0.40 PID MADE GROUND: grey brown gravelly sand with brick 0.90 D and concrete cobbles 0.90 ES MADE GROUND: brown to grey brown gravelly sand. PID=0.20 0.90 PID Gravel sub-angular to rounded fine to coarse flint, 1.20 D brick and concrete. 1.20 SPT N=6 (2,1/2,2,1,1) 1.30 ES 1.30 PID PID=0.70 1.80 ES 2 1.80 PID PID=1.60 2.20 4.05 1.85 D Grey slightly silty gravelly fine to medium and locally 2.00 D medium SAND with strong hydrocarbon odour. Gravel SPT 2.00 N=4 (1,2/1,1,1,1) sub-angular to rounded fine to coarse flint. (KEMPTON ES 2.30 PARK GRAVEL MEMBER). 2.30 PID PID=10.50 2.70 ES 3 PID=21.30 2.70 PID 2.75 D 3.00 D SPT 3.00 N = 34(3,4/8,7,9,10)3 70 FS PID 3.70 PID=5.40 3.75 D 4.30 1.95 D 4.00 Brown with occasional orange brown and grey mottling 4 40 1 85 SPT N=13 (5,6/4,3,3,3) 4.00 CLAY with rare rounded flint gravel. (LONDON CLAY D 4.35 FORMATION). 4.35 FS Dark grey brown CLAY. Closely fissured. (LONDON PID=1.80 4.50 PID CLAY FORMATION). 5 4.75 D 5.00 D SPT N=11 (2,3/2,3,3,3) 5.00 5.50m becomes very closely fissured. 6.00 D 6.0m very rare fine white shell fragments and rare bioturbation traces. 6.50 U D 7.50 ES 7.50 PID PID=0.20 8.00 8.0m frequent grey veining and bioturbation traces. 9.00 D 9 9.0m becomes extremely closely fissured with 9.00 ES some lamination. PID 9.00 PID=1.10 9.50 D 9.50 N=19 (1,3/4,4,5,6) 10.00 D 10 Hole Diameter Chiselling Inclination and Orientation Casing Diameter Depth Base Depth Base Depth Top Depth Base Depth Top Depth Base Inclination Remarks

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Well	Water Strikes			Situ Testing		Depth (m)	Level (m)	Legend		Stratum Desc	cription		
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	Hole Diam	eter	Casing	g Diameter			Chisell	ing		Inclir	nation and Orio	entation	30 —
Depth	Base [Diameter	Depth Base		Dep	th Top Dep		Duration	Tool	Depth Top Depth 6		nation	Orientation
Rema	arks												

Percussion Drilling Log Project Name: Manor Road Client: Avanton Richmond Development Ltd Date: 23/04/2021 - 26/04/2021 Location: 84 Manor Rd, Richmond TW9 Contractor: SADS Co-ords: E518843.62 N175378.64 Project No.: LMB_Manor Crew Name: Drilling Equipment: Borehole Number Hole Type Level Logged By Scale Page Number 6.25m AoD BH105 RO PIL&NB 1:50 Sheet 1 of 3 Sample and In Situ Testing Water Depth Level Well Legend Stratum Description Strikes (m) (m) Depth (m) Results Type Tarmac. 0.18 6.07 0.20 MADE GROUND: red brown to red grey sandy 0.30 5.95 PID=1.30 0.20 PID roadstone gravel 0.25 D MADE GROUND: brown to grey brown gravelly sand 0.50 D with concrete and brick cobbles (very compact). 0.50 FS 0.50 PID PID=0.90 0.75 D 1.10 5.15 brown to orange brown gravelly medium to medium 0.90 ES coarse SAND with occasional flint cobbles. D D 1.00 (KEMPTON PARK GRAVEL MEMBER). 1.20 N=47 SPT 1.20 (6,9/10,12,12,13) 1.50 FS 2 1.50 PID PID=0.60 1.85 D 2.00 D SPT N=48 2.00 (8,10/12,12,12,12) 2.75 D 3 3.00 D FS 3 00 SPT 3.00 N = 32(4,5/6,8,8,10) PID 3.00 PID=0.50 D 3.75 4.00 D 4.00 SPT N=31 (3,5/6,8,8,9) 4.20 ES PID PID=0.70 4.20 4.75 D 4.75 1.50 brown with occasional orange brown mottling CLAY. 4.80 ES 4.85 1.40 (LONDON CLAY FORMATION). 5 ח 5.00 Stiff dark grey to grey brown CLAY. Closely fissured. ES 5.00 (LONDON CLAY FORMATION). N=6 (1,2/1,2,1,2) PID=0.10 SPT 5.00 PID 5.00 6.00 D 6 6.50 U >7.0m occasional fine whitish inclusions. 7.50 D 7.90 - 8.00 8 7.90 PID PID=0.00 8.00 D SPT N=17 (3,4/4,4,4,5) 8.00 9.00 D 9 9.50 U 10 Hole Diameter Casing Diameter Chiselling Inclination and Orientation Depth Top Depth Base Depth Base Depth Base Depth Top Depth Base Inclination Remarks

					F	Perc	cus	sion	Drill	ling L	_og			
Projec	ct Name	: Manor R	oad		Client	: Avantoi	n Richmo	ond Develo	pment Ltd	Date: 23/0	4/2021 - 26	6/04/2021		
-		Лanor Rd,		d TW9		actor: SA			<u>'</u>			N175378.6	 64	
		 _MB_Man			Crew	Name:				Drilling Eq	uipment:			
Bor	ehole N BH10			le Type RO		Level 6.25m A	oD		ged By _&NB		cale :50		Number	
Well	Water Strikes			Situ Testino		Depth	Level	Legend		Stratui	m Descripti	on		
	Strikes	Depth (r	m) Type	Results	3	(m)	(m)		Stiff dark g	rey to grey b	rown CLAY. (Closely fissur	ed.	-
		10.50 10.90 - 11	.00 ES						(LONDON	CLAY FORM	IATION).			- - - - -
		10.90 11.00 11.00	PID D SPT						>11.0m be micaceous	comes sligh s.	tly sandy. S —	Sand fine ar	ıd	11 -
		12.00 12.50												12 -
		12.50	D						>13.0m oc	casional find	e shell fragi	ments.		13 -
		13.50 13.90 - 14 13.90 14.00	I.00 ES PID D						13.90m da nodule.	rk grey coai	rse gravel s —	ized pyrite		14 -
		15.00 15.50							>15.0m be	ecomes very	stiff.			15 -
														16 -
		16.50 16.90 - 17 16.90 17.00 17.00	7.00 ES PID D	PID=0.30 N=35 (5,6/7,9,9,										17 -
		18.00 18.50												18 -
		10.00												19 -
		19.50 20.00	D											20 —
Depth	Hole Diam Base [eter Diameter	Casin, Depth Base	g Diameter Diameter	Depth	Top Dep	Chis oth Base	elling Duration	Tool	Depth Top	Inclination Depth Base	and Orientation Inclination	Orient	ation
Rema	arks				1									

						Perd	cuss	sion	Drill	ing L	_og			
Projec	ct Name	: Manor Ro	oad		Clier	nt: Avanton	Richmor	nd Develo	pment Ltd	Date: 23/0)4/2021 - 26	5/04/2021		
Locat	ion: 84 N	Manor Rd,	Richmond	tW9	Conf	tractor: SA	DS			Co-ords: E	518843.62	N175378.6	 64	
Proje	ct No. : I	 _MB_Mand	or		Crev	v Name:				Drilling Eq	uipment:			
Воі	ehole N BH10	5	F	e Type RO		Level 6.25m Ac	DD		ged By .&NB	1	cale :50		e Numbe	
Well	Water Strikes			Situ Testing		Depth (m)	Level (m)	Legend		Stratur	m Descripti	on		
Well	Strikes		n) Type 10 ES SPT PID B U		12)	25.00	-18.75	Legend	(LONDON	rey to grey bi	rown CLAY. (Closely fissu	red.	21 - 22 - 23 - 24 - 25 - 26 - 26 - 26 - 26 - 26 - 26 - 26
														27 —
														28 -
														29
														30 -
Depth			Casing Depth Base	Diameter Diameter	Dep	th Top Dept	Chisellth Base [ing Duration	Tool	Depth Top	Inclination Depth Base	and Orientation	Orient	
0111	0													

March Marc							<u> </u>		
Character 2010 Char	Project Name: Manor Road			<u> </u>	ant: Avanton Richmond Dev	relopment Ltd		Date: 26/04/2021	
Continue	in: 84 Manor Rd, Richmα	1W9		00	ntractor: SADS			Co-ords: E518915.06 N175486.16	
Particular Par	Project No. : LMB_Manor							Equipment: 3CX	
1,000 1,00	Location Numb TP101	er	Location Ty _l	90	Level 6.20m AoD		Logi 1		Page Number Sheet 1 of 1
1.00 1.00	Well Strikes		ample and In Situ Tes		Depth (m)	Level (m)	Legend		
1.00 1.00					0.10	6.10		Brick paving over sand MADE GROUND: red to red brown sandy roadstone gravel (sub-base).	
1.55					0:30	5.90		MADE GROUND: brown to grey brown gravelly sand with brick and concrete cobble.	s and whole brick.
1,50 1,50		0.50	ES PID	PID=0.30	09:0	5.60		MADE GROUND: black gravelly sand. Gravel rounded fine to coarse flint, fine brick, fragments.	organic material and chalk/lime
1.50 EB PD-0.10		0.80	ES PID	PID=0.10	06:0	5.30		Firm brown with grey mottling sandy gravelly CLAY. Gravel sub-angular to rounded fi	ine to coarse flint. (KEMPTON
2.000 E.B. PICHOLINE PRODUCTION PARK GRAVEL MEMBER). 2.000 B.B. PICHOLINE PRODUCTION PARK GRAVEL MEMBER PRODUCTION PARK GRAVEL PR		1.50 1.50 1.50	o	PID=0.10					
2.50 3.70 End of Boronce at 2.50 End o		2.00 2.00 2.00	B ES DIO	PID=0.10	1.80	4.40		Greenish grey to brown clayey gravelly medium SAND. Gravel sub-angular to round (KEMPTON PARK GRAVEL MEMBER).	ed fine to coarse flint.
Dimensions					2.50	3.70			
				Alique	Shoring Used	Trench Support an	d Comment		

March Parameter Paramete									•				
	ot Name: Manor Road				Client: Avanton R	ichmond Develop	ment Ltd			Date: 27/04/2021			
	ion: 84 Manor Rd, Rich	hmond TW9			Contractor: SADS	8				Co-ords: E518955.11 N17549	4.54		
1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1, 1,	ct No. : LMB_Manor				Crew Name:								
1,000 1,00	Location Nu TP102	mber	Locati	on Type TP		Level 6.50m AoD		J607	ged By JN	Scale 1:25		Page Number Sheet 1 of 1	umber of 1
1.00 ES PID-0.20 0.00			sample and In Sit			Depth (m)	Level (m)			Stratum De	scription		
150 620 620 620 620 620 620 620 620 620 62						0.10	6.40		Brick paving over sand	i. o red brown sandy roadstone gravel	(sub-base).		
150 PID PD=0.20						0:30	6.20		Weak Concrete				
100 ES		0.40	ES PID	PID=0.20		0.50	6.00		MADE GROUND: brow	n sandy gravel and brick cobbbles.			
1.00 ES PID=0.30 1.40 5.10 1.40 5.10 1.50 4.50 1.50 1.50 4.50 1.50 1.50 1.50 1.50 1.50 1.50 1.50 1						0.70	5.80		MADE GOUND: relict	opsoil with brick and concrete fragmi	ents, wood, glass, clini	ker and large roots.	
1.50 ES PID=0.80 1.60 4.90 4.40 ES PID=0.20 2.10 4.40 ES PID=0.20 ES PID=0.20 2.10 4.40 ES PID=0.20 ES		1.00	ES PID	PID=0.30									
2.00 ES PID PID=0.20 2.10 4.40 4.40 4.40 4.40 4.40 4.40 4.40 4		1.50	S C	09 O=Cld		1.40	5.10		MADE GROUND: dark occasional flint cobbler	grey to greenish black silty sandy gr	avelly clay. Gravel suk	-angular fine flint and brick wil	ŧ
2.00 ES PID=0.20 2.10 4.40 2.30 B B 2.50 4.00 Dimensions Previous Previous Previous Used Trench Support and Comment Comment		}	1	1		1.60	4.90		Firm orange brown will rounded fine to coarse	h grey mottling sandy gravelly CLAY flint. (KEMPTON PARK GRAVEL ME	with partings of coarse	SAND. Gravel sub-angular to	op .
2.30 B		2.00 2.00 2.00	D PD	PID=0.20		2.10	- 4.40		Greenish to orange brown KARDTON DARK CA	wn slightly dlayey gravelly SAND. Gr wer mewaren	ravel sub-angular to ro	nunded fine to coarse flint.	
Dimensions		2.30	ш							74 FF IME (MIDELY)			
Dimensions Pit Width Pit Stability Shoring Used Trench Support and Comment Remarks Date Rate						2.50	4.00			End of Boreho	le at 2.500m		
Dimensions PR Width Pit Stability Shoring Used Trench Support and Comment Remarks Date Rate													
				Pit Stability	Shoring	Ned	Trench Support and	Comment	Remarks		Date	Pumping Data Rate	Remarks
		090											

1,000 E.B. Pro-0-20 1,22 6,006 Consultation to the significance of the consultation of the con
ES PID=0.20 2.50 3.76 End of Borehole at 2.500m

APPENDICES

TRIAL PIT PHOTOGRAPHS





Plate 2: TP01 excavation.

Photographic Record

Project: Manor Road

Plates 1 & 2

Total Interest of the August A





Plate 4: TPO2 arisings

Project: Manor Road

Plates 3 & 4



Photographic Record





Photographic Record

Project: Manor Road

Plates 5 & 6



APPENDICES

APPENDIX B CHEMICAL LABORATORY RESULTS





Philip Lewis LMB Geosolutions Ltd 28 Dresden Road London N19 3BD

i2 Analytical Ltd.
7 Woodshots Meadow,
Croxley Green
Business Park,
Watford,
Herts,
WD18 8YS

t: 01923 225404 **f:** 01923 237404

e: philip@lmbgeosolutions.com e: reception@i2analytical.com

Analytical Report Number: 21-71560

Replaces Analytical Report Number: 21-71560, issue no. 1 Additional analysis undertaken.

Project / Site name: Manor Road Samples received on: 26/04/2021

Your job number: LMB MANOR Samples instructed on/ 28/04/2021

Analysis started on:

Your order number: Analysis completed by: 12/05/2021

Report Issue Number: 2 **Report issued on:** 13/05/2021

Samples Analysed: 7 soil samples

Signed: M. Ottrachickal

Agnieszka Czerwińska Technical Reviewer (Reporting Team) For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41 -711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are : soils - 4

soils - 4 weeks from reporting leachates - 2 weeks from reporting waters - 2 weeks from reporting asbestos - 6 months from reporting

Excel copies of reports are only valid when accompanied by this PDF certificate.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies.

An estimate of measurement uncertainty can be provided on request.





Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	11	8.0	3.6	7.3	4.5
Total mass of sample received	kg	0.001	NONE	1.0	1.2	1.2	0.50	0.50
Asbestos in Soil Screen / Identification Name	Type	N/A	ISO 17025	-	-	-	-	-
Asbestos in Soil	Туре	N/A	ISO 17025	Not-detected	Not-detected	Not-detected	Not-detected	Not-detected
General Inorganics						Decoted	Decoted	
pH - Automated	pH Units	N/A	MCERTS	7.9	10.3	7.7	7.9	7.7
Total Cyanide	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Sulphate as SO4	mg/kg	50	MCERTS	3800	6100	630	240	4000
Water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	a#	0.00125	MCERTS	0.095	0.67	0.022	0.096	0.46
' '	g/l mg/kg	1	MCERTS		17			
Sulphide	mg/kg %	0.1	MCERTS	2.8	-	21	3.2	13 1.1
Organic Matter Total Organic Carbon (TOC)	%	0.1	MCERTS	5.0	0.5	0.1	0.2	0.6
Total Phenols Total Phenols (monohydric)	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Speciated PAHs	•							
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Fluorene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Phenanthrene	mg/kg	0.05	MCERTS	2.0	1.7	< 0.05	0.59	< 0.05
Anthracene	mg/kg	0.05	MCERTS	0.46	0.44	< 0.05	0.27	< 0.05
Fluoranthene	mg/kg	0.05	MCERTS	4.6	2.6	0.35	1.9	0.49
Pyrene	mg/kg	0.05	MCERTS	4.3	2.8	0.39	1.9	0.74
Benzo(a)anthracene	mg/kg	0.05	MCERTS	2.6	1.5	< 0.05	0.71	< 0.05
Chrysene	mg/kg	0.05	MCERTS	3.0	1.5	< 0.05	1.2	< 0.05
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	3.3	1.4	< 0.05	1.1	< 0.05
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	2.1	0.55	< 0.05	0.72	< 0.05
Benzo(a)pyrene	mg/kg	0.05	MCERTS MCERTS	3.2	1.0	< 0.05	1.1	< 0.05
Indeno(1,2,3-cd)pyrene	mg/kg mg/kg	0.05	MCERTS	1.6	0.67	< 0.05	0.60	< 0.05
Dibenz(a,h)anthracene	mg/kg mg/kg	0.05	MCERTS	0.54	0.24	< 0.05	< 0.05	< 0.05
Benzo(ghi)perylene	mg/kg	0.03	PICENTO	2.0	0.71	< 0.05	0.72	< 0.05





Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status	None Supplied	None Supplied	коле Заррпеа	None Supplied	нопе заррпеч
Heavy Metals / Metalloids								
Arsenic (aqua regia extractable)	mg/kg	1	MCERTS	65	14	26	20	9.9
Boron (water soluble)	mg/kg	0.2	MCERTS	1.3	2.6	0.3	< 0.2	0.6
Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	1.0	0.4	< 0.2	< 0.2	< 0.2
Chromium (hexavalent)	mg/kg	4	MCERTS	< 4.0	< 4.0	< 4.0	< 4.0	< 4.0
Chromium (aqua regia extractable)	mg/kg	1	MCERTS	20	25	11	27	22
Copper (aqua regia extractable)	mg/kg	1	MCERTS	110	37	11	15	28
Lead (aqua regia extractable)	mg/kg	1	MCERTS	440	120	58	20	40
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	1.3	0.3	< 0.3	< 0.3	< 0.3
Nickel (agua regia extractable)	mg/kg	1	MCERTS	33	19	9.9	22	15
Selenium (aqua regia extractable)	mg/kg	1	MCERTS	< 1.0	< 1.0	3.6	2.2	< 1.0
Zinc (agua regia extractable)	mg/kg	1	MCERTS	310	130	140	63	68
Monoaromatics & Oxygenates Benzene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Toluene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Ethylbenzene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
p & m-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
o-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
MTBE (Methyl Tertiary Butyl Ether)	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Petroleum Hydrocarbons								
TPH C10 - C40	mg/kg	10	MCERTS	74	220	< 10	100	470
		0.004	MOEDTO					
TPH-CWG - Aliphatic > EC5 - EC6	mg/kg mg/kg	0.001	MCERTS MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aliphatic > EC6 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aliphatic >EC8 - EC10			MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aliphatic > EC10 - EC12	mg/kg mg/kg	2	MCERTS	< 1.0	1.1	< 1.0	< 1.0	< 1.0
TPH-CWG - Aliphatic >EC12 - EC16		8	MCERTS	< 2.0	5.9	< 2.0	2.6	10
TPH-CWG - Aliphatic > EC16 - EC21	mg/kg mg/kg	8	MCERTS	< 8.0	16	< 8.0	< 8.0	35
TPH-CWG - Aliphatic (ECE EC35	mg/kg mg/kg	10	MCERTS	< 8.0	62	< 8.0	41	250
TPH-CWG - Aliphatic (EC5 - EC35)	mg/kg	10	FICERTS	< 10	85	< 10	49	300
TPH-CWG - Aromatic >EC5 - EC7	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aromatic >EC3 - EC7 TPH-CWG - Aromatic >EC7 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aromatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aromatic >EC10 - EC12	mg/kg	1	MCERTS	7.1	< 1.0	< 1.0	< 1.0	< 1.0
TPH-CWG - Aromatic >EC10 - EC12 TPH-CWG - Aromatic >EC12 - EC16	mg/kg	2	MCERTS	12	7.0	< 2.0	< 2.0	5.7
TPH-CWG - Aromatic >EC12 - EC16 TPH-CWG - Aromatic >EC16 - EC21	mg/kg	10	MCERTS	19	24	< 10	14	24
TPH-CWG - Aromatic >EC16 - EC21 TPH-CWG - Aromatic >EC21 - EC35	mg/kg	10	MCERTS	36	62	< 10	32	85
TPH-CWG - Aromatic (EC5 - EC35)	mg/kg	10	MCERTS	74	93	< 10	46	110
IFII-CWG - AIUIIIduc (ECS - ECSS)	5/119			/4	93	< 10	40	110





I - b CI - Nob - o				1052005	1052006	1052007	1053000	1052000
Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
VOCs			1					
Chloromethane	μg/kg	1	ISO 17025	T .	< 1.0	_	_	_
Chloroethane	μg/kg	1	NONE		< 1.0	_	_	_
Bromomethane	μg/kg	1	ISO 17025		< 1.0			
Vinyl Chloride	μg/kg	1	NONE	-	< 1.0		<u> </u>	
Trichlorofluoromethane	μg/kg	1	NONE	-	< 1.0	-	_	_
1,1-Dichloroethene	μg/kg	1	NONE		< 1.0			
1,1,2-Trichloro 1,2,2-Trifluoroethane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Cis-1,2-dichloroethene	μg/kg	1	MCERTS		< 1.0	-	-	-
MTBE (Methyl Tertiary Butyl Ether)	μg/kg	1	MCERTS	-	< 1.0			
1,1-Dichloroethane	μg/kg	1	MCERTS	-	< 1.0		<u> </u>	-
2,2-Dichloropropane	μg/kg	1	MCERTS		< 1.0	-	-	-
Z,2-Dichloropropane Trichloromethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,1,1-Trichloroethane	μg/kg	1	MCERTS	-	< 1.0		-	-
1,2-Dichloroethane	μg/kg	1	MCERTS	-	< 1.0	-	<u> </u>	-
•		1	MCERTS	-		-		-
1,1-Dichloropropene	μg/kg	1	NONE	-	< 1.0	-		-
Trans-1,2-dichloroethene	μg/kg			-	< 1.0	-	-	-
Benzene	μg/kg "	1	MCERTS	-	< 1.0	-	-	-
Tetrachloromethane	μg/kg 	1	MCERTS	-	< 1.0	-	-	-
1,2-Dichloropropane	μg/kg "	1	MCERTS	-	< 1.0	-	-	-
Trichloroethene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Dibromomethane	μg/kg 	1	MCERTS	-	< 1.0	-	-	-
Bromodichloromethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Cis-1,3-dichloropropene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Trans-1,3-dichloropropene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Toluene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,1,2-Trichloroethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,3-Dichloropropane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Dibromochloromethane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Tetrachloroethene	μg/kg	1	NONE	-	< 1.0	-	-	-
1,2-Dibromoethane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Chlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,1,1,2-Tetrachloroethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Ethylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
p & m-Xylene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Styrene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Tribromomethane	μg/kg	1	NONE	-	< 1.0	-	-	-
o-Xylene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,1,2,2-Tetrachloroethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Isopropylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Bromobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
n-Propylbenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
2-Chlorotoluene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
4-Chlorotoluene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,3,5-Trimethylbenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
tert-Butylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,2,4-Trimethylbenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
sec-Butylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,3-Dichlorobenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
p-Isopropyltoluene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
1,2-Dichlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,4-Dichlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Butylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-





Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
1,2-Dibromo-3-chloropropane	µg/kg	1	ISO 17025	-	< 1.0	-	-	-
1,2,4-Trichlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Hexachlorobutadiene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,2,3-Trichlorobenzene	µg/kg	1	ISO 17025	-	< 1.0	-	-	-





				1052005	1053006	1050007	1053000	105000
Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
SVOCs								
Aniline	mg/kg	0.1	NONE	-	< 0.1	-	-	-
Phenol	mg/kg	0.2	ISO 17025	_	< 0.2	_	-	-
2-Chlorophenol	mg/kg	0.1	MCERTS	_	< 0.1	_	_	_
Bis(2-chloroethyl)ether	mg/kg	0.2	MCERTS	_	< 0.2	_	_	_
1,3-Dichlorobenzene	mg/kg	0.2	MCERTS	-	< 0.2	-	_	_
1,2-Dichlorobenzene	mg/kg	0.1	MCERTS		< 0.1		-	
	mg/kg	0.2	MCERTS	-	< 0.2	-		
1,4-Dichlorobenzene	mg/kg	0.1	MCERTS		< 0.2	-		
Bis(2-chloroisopropyl)ether	mg/kg	0.1	MCERTS	-				
2-Methylphenol	mg/kg	0.05	MCERTS	-	< 0.3	-	-	-
Hexachloroethane	mg/kg mg/kg	0.05	MCERTS	-	< 0.05	-	-	
Nitrobenzene					< 0.3			-
4-Methylphenol	mg/kg	0.2	NONE	-	< 0.2	-	-	-
Isophorone	mg/kg		MCERTS	-	< 0.2	-	-	-
2-Nitrophenol	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
2,4-Dimethylphenol	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
Bis(2-chloroethoxy)methane	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
1,2,4-Trichlorobenzene	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
Naphthalene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
2,4-Dichlorophenol	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
4-Chloroaniline	mg/kg	0.1	NONE	-	< 0.1	-	-	-
Hexachlorobutadiene	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
4-Chloro-3-methylphenol	mg/kg	0.1	NONE	-	< 0.1	-	-	-
2,4,6-Trichlorophenol	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
2,4,5-Trichlorophenol	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
2-Methylnaphthalene	mg/kg	0.1	NONE	-	< 0.1	-	-	-
2-Chloronaphthalene	mg/kg	0.1	MCERTS		< 0.1	-	-	-
Dimethylphthalate	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
2,6-Dinitrotoluene	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
Acenaphthylene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
Acenaphthene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
2,4-Dinitrotoluene	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
Dibenzofuran	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
4-Chlorophenyl phenyl ether	mg/kg	0.3	ISO 17025	-	< 0.3	-	-	-
Diethyl phthalate	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
4-Nitroaniline	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
Fluorene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
Azobenzene	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
Bromophenyl phenyl ether	mg/kg	0.2	MCERTS	-	< 0.2	-	_	-
Hexachlorobenzene	mg/kg	0.3	MCERTS	_	< 0.3	_	_	_
Phenanthrene	mg/kg	0.05	MCERTS	-	1.7	-	_	_
Anthracene	mg/kg	0.05	MCERTS	-	0.44	-	_	-
Carbazole	mg/kg	0.3	MCERTS	_	< 0.3	_	_	-
Dibutyl phthalate	mg/kg	0.2	MCERTS	_	< 0.2	_	_	_
Anthraquinone	mg/kg	0.3	MCERTS	_	< 0.3	_	_	_
Fluoranthene	mg/kg	0.05	MCERTS	-	2.6	-	-	-
Pyrene	mg/kg	0.05	MCERTS	-	2.8		-	-
Butyl benzyl phthalate	mg/kg	0.03	ISO 17025	-	< 0.3	-		
Benzo(a)anthracene	mg/kg	0.05	MCERTS	-	1.5	-	-	-
Benzo(a)anthracene Chrysene	mg/kg	0.05	MCERTS	-	1.5	-	-	-
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS					-
	mg/kg	0.05	MCERTS	-	1.4	-	-	
Benzo(k)fluoranthene	_	0.05	MCERTS	-	0.55	-	-	-
Benzo(a)pyrene	mg/kg	0.05	PICERIS	-	1.0	-	-	-





Lab Sample Number		•		1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	-	0.67	-	-	-
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	-	0.24	-	-	-
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	-	0.71	-	-	-





Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number	Sample Number			None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken			None Supplied	None Supplied	None Supplied	None Supplied	None Supplied	
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
PCBs by GC-MS								
PCB Congener 28	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 52	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 101	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 118	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 138	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 153	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 180	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 153 PCB Congener 180 Total PCBs by GC-MS		0.001	MCERTS					
Total PCBs	mg/kg	0.007	MCERTS	-	-	-	-	-

U/S = Unsuitable Sample I/S = Insufficient Sample





Lab Sample Number				1852810	1852811
Sample Reference				TP101	TP102
Sample Number				None Supplied	None Supplied
Depth (m)				0.80	0.40
Date Sampled				26/04/2021	27/04/2021
Time Taken				None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status		
Stone Content	%	0.1	NONE	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	21	5.8
Total mass of sample received	kg	0.001	NONE	0.80	1.0
Asbestos in Soil Screen / Identification Name	Type	N/A	ISO 17025	-	Chrysotile- Loos Fibrous Debris
Asbestos in Soil	Type	N/A	ISO 17025	Not-detected	Detected
General Inorganics					
pH - Automated	pH Units	N/A	MCERTS	7.4	10.1
Total Cyanide	mg/kg	1	MCERTS	120	< 1.0
Total Sulphate as SO4	mg/kg	50	MCERTS	1000	1000
Water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.46	0.15
Sulphide	mg/kg	1	MCERTS	140	17
Organic Matter	%	0.1	MCERTS	1.8	-
Total Organic Carbon (TOC)	%	0.1	MCERTS	1.1	0.2

Total Phenois Total Phenols (monohydric)

Speciated PAHs					
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Fluorene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Phenanthrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Anthracene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Fluoranthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Pyrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(a)anthracene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Chrysene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(a)pyrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(ahi)pervlene	mg/kg	0.05	MCERTS	< 0.05	< 0.05

mg/kg 1 MCERTS

< 1.0

< 1.0

Total PAH

Speciated Total EPA-16 PAHs	mg/kg	0.8	MCERTS	< 0.80	< 0.80





Lab Sample Number				1852810	1852811		
•	Sample Reference						
Sample Number	TP101 None Supplied	TP102 None Supplied					
Depth (m)	0.80	0.40					
Date Sampled	26/04/2021	27/04/2021					
Time Taken				None Supplied	None Supplied		
Time raken	1	-	1	чоне заррнеа	None Supplied		
		Limit of detectior	Ac				
Analytical Parameter	Units	약	Accreditation Status				
(Soil Analysis)	ਫ਼ੋਂ	lete	itat				
		Ctio	ion				
Hanny Makala / Makallaida		5					
Heavy Metals / Metalloids	mg/kg	1	MCERTS	15	14		
Arsenic (aqua regia extractable)	mg/kg	0.2	MCERTS	1.8	0.5		
Boron (water soluble) Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	< 0.2	< 0.2		
Chromium (hexavalent)	mg/kg	4	MCERTS	< 4.0	< 4.0		
Chromium (nexavalent) Chromium (aqua regia extractable)	mg/kg	1	MCERTS	28	19		
Copper (aqua regia extractable)	mg/kg	1	MCERTS	62	35		
Lead (aqua regia extractable)	mg/kg	1	MCERTS	270	140		
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	< 0.3	< 0.3		
Nickel (agua regia extractable)	mg/kg	1	MCERTS	17	17		
Selenium (aqua regia extractable)	mg/kg	1	MCERTS	2.9	< 1.0		
Zinc (aqua regia extractable)	mg/kg	1	MCERTS	130	150		
=no (aqua 103ia ola acasa)	<u> </u>			130	150		
Monoaromatics & Oxygenates							
Benzene	μg/kg	1	MCERTS	< 1.0	< 1.0		
Toluene	μg/kg	1	MCERTS	< 1.0	< 1.0		
Ethylbenzene	μg/kg	1	MCERTS	< 1.0	< 1.0		
p & m-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0		
o-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0		
MTBE (Methyl Tertiary Butyl Ether)	µg/kg	1	MCERTS	< 1.0	< 1.0		
Petroleum Hydrocarbons							
TPH C10 - C40	mg/kg	10	MCERTS	49	< 10		
TPH-CWG - Aliphatic >EC5 - EC6	mg/kg	0.001	MCERTS	< 0.001	< 0.001		
TPH-CWG - Aliphatic >EC6 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001		
TPH-CWG - Aliphatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001	< 0.001		
TPH-CWG - Aliphatic >EC10 - EC12	mg/kg	1	MCERTS	< 1.0	< 1.0		
TPH-CWG - Aliphatic >EC12 - EC16	mg/kg	2	MCERTS	13	< 2.0		
TPH-CWG - Aliphatic >EC16 - EC21	mg/kg	8	MCERTS	18	< 8.0		
TPH-CWG - Aliphatic >EC21 - EC35	mg/kg	8	MCERTS	18	< 8.0		
TPH-CWG - Aliphatic (EC5 - EC35)	mg/kg	10	MCERTS	49	< 10		
			MOEDE:				
TPH-CWG - Aromatic >EC5 - EC7	mg/kg	0.001	MCERTS	< 0.001	< 0.001		
TPH-CWG - Aromatic >EC7 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001		
TPH-CWG - Aromatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001	< 0.001		
TPH-CWG - Aromatic >EC10 - EC12	mg/kg	1	MCERTS MCERTS	< 1.0	< 1.0		
TPH-CWG - Aromatic >EC12 - EC16	mg/kg mg/kg	2 10	MCERTS	< 2.0	< 2.0		
TPH-CWG - Aromatic > EC16 - EC21	mg/kg mg/kg	10	MCERTS	< 10	< 10		
TPH-CWG - Aromatic >EC21 - EC35	mg/kg mg/kg	10	MCERTS	< 10	< 10		
TPH-CWG - Aromatic (EC5 - EC35)	mg/kg	10	PICERTS	< 10	< 10		





Lab Sample Number				1852810	1852811
Sample Reference	TP101	TP102			
Sample Number	None Supplied	None Supplied			
Depth (m)				0.80	0.40
Date Sampled				26/04/2021	27/04/2021
Time Taken				None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status		
VOCs					
Chloromethane	μg/kg	1	ISO 17025	< 1.0	_
Chloroethane	μg/kg	1	NONE	< 1.0	_
Bromomethane	μg/kg	1	ISO 17025	< 1.0	-
Vinyl Chloride	μg/kg	1	NONE	< 1.0	_
Trichlorofluoromethane	μg/kg	1	NONE	< 1.0	-
1,1-Dichloroethene	μg/kg	1	NONE	< 1.0	-
1,1,2-Trichloro 1,2,2-Trifluoroethane	μg/kg	1	ISO 17025	< 1.0	-
Cis-1,2-dichloroethene	μg/kg	1	MCERTS	< 1.0	-
MTBE (Methyl Tertiary Butyl Ether)	μg/kg	1	MCERTS	< 1.0	-
1,1-Dichloroethane	μg/kg	1	MCERTS	< 1.0	-
2,2-Dichloropropane	μg/kg	1	MCERTS	< 1.0	-
Trichloromethane	μg/kg	1	MCERTS	< 1.0	-
1,1,1-Trichloroethane	μg/kg	1	MCERTS	< 1.0	-
1,2-Dichloroethane	μg/kg	1	MCERTS	< 1.0	-
1,1-Dichloropropene	μg/kg	1	MCERTS	< 1.0	-
Trans-1,2-dichloroethene	μg/kg	1	NONE	< 1.0	-
Benzene	μg/kg	1	MCERTS	< 1.0	-
Tetrachloromethane	μg/kg 	1	MCERTS	< 1.0	-
1,2-Dichloropropane	μg/kg	1	MCERTS	< 1.0	-
Trichloroethene	μg/kg	1	MCERTS MCERTS	< 1.0	-
Dibromomethane	μg/kg	1	MCERTS	< 1.0	-
Bromodichloromethane	μg/kg μg/kg	1	ISO 17025	< 1.0	
Cis-1,3-dichloropropene	μg/kg	1	ISO 17025	< 1.0 < 1.0	
Trans-1,3-dichloropropene Toluene	μg/kg	1	MCERTS	< 1.0	
1,1,2-Trichloroethane	μg/kg	1	MCERTS	< 1.0	
1,3-Dichloropropane	µg/kg	1	ISO 17025	< 1.0	_
Dibromochloromethane	µg/kg	1	ISO 17025	< 1.0	_
Tetrachloroethene	μg/kg	1	NONE	< 1.0	_
1,2-Dibromoethane	μg/kg	1	ISO 17025	< 1.0	_
Chlorobenzene	μg/kg	1	MCERTS	< 1.0	-
1,1,1,2-Tetrachloroethane	μg/kg	1	MCERTS	< 1.0	_
Ethylbenzene	μg/kg	1	MCERTS	< 1.0	-
p & m-Xylene	μg/kg	1	MCERTS	< 1.0	-
Styrene	μg/kg	1	MCERTS	< 1.0	-
Tribromomethane	μg/kg	1	NONE	< 1.0	-
o-Xylene	μg/kg	1	MCERTS	< 1.0	-
1,1,2,2-Tetrachloroethane	μg/kg	1	MCERTS	< 1.0	-
Isopropylbenzene	μg/kg	1	MCERTS	< 1.0	-
Bromobenzene	μg/kg	1	MCERTS	< 1.0	-
n-Propylbenzene	μg/kg	1	ISO 17025	< 1.0	-
2-Chlorotoluene	μg/kg	1	MCERTS	< 1.0	-
4-Chlorotoluene	μg/kg	1	MCERTS	< 1.0	-
1,3,5-Trimethylbenzene	μg/kg	1	ISO 17025	< 1.0	-
tert-Butylbenzene	μg/kg 	1	MCERTS	< 1.0	-
1,2,4-Trimethylbenzene	μg/kg 	1	ISO 17025	< 1.0	-
sec-Butylbenzene	μg/kg	1	MCERTS	< 1.0	-
1,3-Dichlorobenzene	μg/kg	1	ISO 17025	< 1.0	-
p-Isopropyltoluene	μg/kg	1	ISO 17025	< 1.0	-
1,2-Dichlorobenzene	μg/kg	1	MCERTS	< 1.0	-
1,4-Dichlorobenzene	μg/kg μg/kg	1	MCERTS MCERTS	< 1.0 < 1.0	-





Lab Sample Number	1852810	1852811			
Sample Reference	TP101	TP102			
Sample Number				None Supplied	None Supplied
Depth (m)				0.80	0.40
Date Sampled				26/04/2021	27/04/2021
Time Taken				None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status		
1,2-Dibromo-3-chloropropane	µg/kg	1	ISO 17025	< 1.0	-
1,2,4-Trichlorobenzene	μg/kg	1	MCERTS	< 1.0	-
Hexachlorobutadiene	μg/kg	1	MCERTS	< 1.0	-
1,2,3-Trichlorobenzene	µg/kg	1	ISO 17025	< 1.0	-





Lab Sample Number				1852810	1852811		
					TP102		
Sample Reference Sample Number	TP101 None Supplied	None Supplied					
•	Depth (m)						
				0.80	0.40		
Date Sampled				26/04/2021	27/04/2021		
Time Taken			1	None Supplied	None Supplied		
		Limit of detection	Þ				
Analytical Parameter	_	of of	Accreditation Status				
(Soil Analysis)	Units	de	creditat Status				
,		ect	s tio				
		ġ	-				
SVOCs							
Aniline	mg/kg	0.1	NONE	< 0.1	-		
Phenol	mg/kg	0.2	ISO 17025	< 0.2	-		
2-Chlorophenol	mg/kg	0.1	MCERTS	< 0.1	-		
Bis(2-chloroethyl)ether	mg/kg	0.2	MCERTS	< 0.2	-		
1,3-Dichlorobenzene	mg/kg	0.2	MCERTS	0.5	-		
1,2-Dichlorobenzene	mg/kg	0.1	MCERTS	< 0.1	-		
1,4-Dichlorobenzene	mg/kg	0.2	MCERTS	< 0.2	-		
Bis(2-chloroisopropyl)ether	mg/kg	0.1	MCERTS	< 0.1	-		
2-Methylphenol	mg/kg	0.3	MCERTS	< 0.3	-		
Hexachloroethane	mg/kg	0.05	MCERTS	< 0.05	-		
Nitrobenzene	mg/kg	0.3	MCERTS	< 0.3	-		
4-Methylphenol	mg/kg	0.2	NONE	< 0.2	-		
Isophorone	mg/kg	0.2	MCERTS	< 0.2	-		
2-Nitrophenol	mg/kg	0.3	MCERTS	< 0.3	-		
2,4-Dimethylphenol	mg/kg	0.3	MCERTS	< 0.3	-		
Bis(2-chloroethoxy)methane	mg/kg	0.3	MCERTS	< 0.3	-		
1,2,4-Trichlorobenzene	mg/kg	0.3	MCERTS	< 0.3	-		
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	-		
2.4-Dichlorophenol	mg/kg	0.3	MCERTS	< 0.3	-		
4-Chloroaniline	mg/kg	0.1	NONE	< 0.1	-		
Hexachlorobutadiene	mg/kg	0.1	MCERTS	< 0.1	-		
4-Chloro-3-methylphenol	mg/kg	0.1	NONE	< 0.1	_		
2,4,6-Trichlorophenol	mg/kg	0.1	MCERTS	< 0.1	-		
2,4,5-Trichlorophenol	mg/kg	0.2	MCERTS	< 0.2	-		
2-Methylnaphthalene	mg/kg	0.1	NONE	< 0.1	_		
2-Chloronaphthalene	mg/kg	0.1	MCERTS	< 0.1	-		
Dimethylphthalate	mg/kg	0.1	MCERTS	< 0.1	-		
2,6-Dinitrotoluene	mg/kg	0.1	MCERTS	< 0.1	-		
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	_		
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	-		
2,4-Dinitrotoluene	mg/kg	0.2	MCERTS	< 0.2	_		
Dibenzofuran	mg/kg	0.2	MCERTS	< 0.2	_		
4-Chlorophenyl phenyl ether	mg/kg	0.3	ISO 17025	< 0.3	-		
Diethyl phthalate	mg/kg	0.2	MCERTS	< 0.2	_		
4-Nitroaniline	mg/kg	0.2	MCERTS	< 0.2	-		
Fluorene	mg/kg	0.05	MCERTS	< 0.05			
Azobenzene	mg/kg	0.3	MCERTS	< 0.3	-		
Bromophenyl phenyl ether	mg/kg	0.2	MCERTS	< 0.3	-		
Hexachlorobenzene	mg/kg	0.2	MCERTS	< 0.3	-		
Phenanthrene	mg/kg	0.05	MCERTS	< 0.05	-		
Anthracene	mg/kg	0.05	MCERTS	< 0.05	-		
Carbazole	mg/kg	0.03	MCERTS	< 0.05	-		
Dibutyl phthalate	mg/kg	0.2	MCERTS	< 0.2	-		
Anthraguinone	mg/kg	0.2	MCERTS	< 0.2	-		
•	mg/kg	0.05	MCERTS	< 0.05	-		
Fluoranthene	mg/kg	0.05	MCERTS		-		
Pyrene Pyrene	mg/kg	0.05	ISO 17025	< 0.05	-		
Butyl benzyl phthalate	mg/kg	0.05	MCERTS	< 0.3			
Benzo(a)anthracene		0.05		< 0.05	-		
Chrysene	mg/kg		MCERTS	< 0.05	-		
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	=		
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	-		
Benzo(a)pyrene	mg/kg	0.05	MCERTS	< 0.05	-		





Lab Sample Number	1852810	1852811			
Sample Reference				TP101	TP102
Sample Number				None Supplied	None Supplied
Depth (m)				0.80	0.40
Date Sampled				26/04/2021	27/04/2021
Time Taken				None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status		
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	< 0.05	_
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05	-
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	< 0.05	-





Lab Sample Number	1852810	1852811			
Sample Reference	TP101	TP102			
Sample Number				None Supplied	None Supplied
Depth (m)				0.80	0.40
Date Sampled				26/04/2021	27/04/2021
Time Taken				None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status		
PCBs by GC-MS					
PCB Congener 28	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 52	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 101	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 118	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 138	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 153	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 180	mg/kg	0.001	MCERTS	< 0.001	-

Total PCBs by GC-MS

Total PCBs	mg/kg	0.007	MCERTS	< 0.007	-

U/S = Unsuitable Sample I/S = Insufficient Sample





* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
1852805	BH101	None Supplied	0.8	Brown sandy loam with gravel and rubble.
1852806	BH102	None Supplied	0.5	Brown sandy loam with gravel and rubble.
1852807	BH103	None Supplied	0.2	Brown sandy loam with gravel.
1852808	BH104	None Supplied	1.8	Brown loam and sand with gravel.
1852809	BH105	None Supplied	0.5	Brown loam and sand with gravel and brick.
1852810	TP101	None Supplied	0.8	Brown loam and clay with gravel and brick.
1852811	TP102	None Supplied	0.4	Light brown loam and sand with rubble.





Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status	
Sulphate, water soluble, in soil (16hr extraction)	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In house method.	L038-PL	D	MCERTS	
Metals in soil by ICP-OES	Determination of metals in soil by aqua-regia digestion followed by ICP-OES.	In-house method based on MEWAM 2006 Methods for the Determination of Metals in Soil.	L038-PL	D	MCERTS	
Asbestos identification in soil	Asbestos Identification with the use of polarised light microscopy in conjunction with disperion staining techniques.	In house method based on HSG 248	A001-PL	D	ISO 17025	
Boron, water soluble, in soil	Determination of water soluble boron in soil by hot water extract followed by ICP-OES.	In-house method based on Second Site Properties version 3	L038-PL	D	MCERTS	
Hexavalent chromium in soil	Determination of hexavalent chromium in soil by extraction in water then by acidification, addition of 1,5 diphenylcarbazide followed by colorimetry.	In-house method	L080-PL	W	MCERTS	
Moisture Content	Moisture content, determined gravimetrically. (30 oC)	In house method.	L019-UK/PL	W	NONE	
Monohydric phenols in soil	Determination of phenols in soil by extraction with sodium hydroxide followed by distillation followed by colorimetry.	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton (skalar)	L080-PL	w	MCERTS	
Organic matter (Automated) in soil	Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate.	In house method.	L009-PL	D	MCERTS	
Speciated EPA-16 PAHs in soil	Determination of PAH compounds in soil by extraction in dichloromethane and hexane followed by GC-MS with the use of surrogate and internal standards.	In-house method based on USEPA 8270	L064-PL	D	MCERTS	
PCB's By GC-MS in soil	Determination of PCB by extraction with acetone and hexane followed by GC-MS.	In-house method based on USEPA 8082	L027-PL	D	MCERTS	
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement.	In house method.	L099-PL	D	MCERTS	
Sulphide in soil	Determination of sulphide in soil by acidification and heating to liberate hydrogen sulphide, trapped in an alkaline solution then assayed by ion selective electrode.	In-house method	L010-PL	D	MCERTS	
Total sulphate (as SO4 in soil)	Determination of total sulphate in soil by extraction with 10% HCl followed by ICP-OES.	In house method.	L038-PL	D	MCERTS	
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight.	In-house method based on British Standard Methods and MCERTS requirements.	L019-UK/PL	D	NONE	
Semi-volatile organic compounds in soil	Determination of semi-volatile organic compounds in soil by extraction in dichloromethane and hexane followed by GC-MS.	In-house method based on USEPA 8270	L064-PL	D	MCERTS	
Total cyanide in soil	Determination of total cyanide by distillation followed by colorimetry.	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton (Skalar)	L080-PL	W	MCERTS	
Total organic carbon (Automated) in soil	Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate.	In house method.	L009-PL	D	MCERTS	





Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Volatile organic compounds in soil	Determination of volatile organic compounds in soil by headspace GC-MS.	In-house method based on USEPA8260	L073B-PL	W	MCERTS
BTEX and MTBE in soil (Monoaromatics)	Determination of BTEX in soil by headspace GC-MS.	In-house method based on USEPA8260	L073B-PL	W	MCERTS
TPHCWG (Soil)	Determination of hexane extractable hydrocarbons in soil by GC-MS/GC-FID.	In-house method with silica gel split/clean up.	L088/76-PL	W	MCERTS
TPH Banding in Soil by FID	Determination of hexane extractable hydrocarbons in soil by GC-FID.	In-house method, TPH with carbon banding and silica gel split/cleanup.	L076-PL	W	MCERTS

For method numbers ending in 'UK' analysis have been carried out in our laboratory in the United Kingdom.

For method numbers ending in 'PL' analysis have been carried out in our laboratory in Poland.

Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.

Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.

Sample Deviation Report



Sample ID	Other ID	Sample Type	Lab Sample Number			Test Ref	Test Deviation
BH101	None Supplied	S	1852805	С	Sulphide in soil	L010-PL	С
BH101	None Supplied	S	1852805	С	Total cyanide in soil	L080-PL	С
BH102	None Supplied	S	1852806	С	Sulphide in soil	L010-PL	С
BH102	None Supplied	S	1852806	С	Total cyanide in soil	L080-PL	С
BH103	None Supplied	S	1852807	С	Sulphide in soil	L010-PL	С
BH103	None Supplied	S	1852807	С	Total cvanide in soil	L080-PL	С





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Analytical Report Number: 21-71560

Replaces Analytical Report Number: 21-71560, issue no. 2 Additional analysis undertaken.

Project / Site name: Manor Road Samples received on: 26/04/2021

Your job number: LMB MANOR Samples instructed on/ 28/04/2021

Analysis started on:

Your order number: Analysis completed by: 19/05/2021

Report Issue Number: 3 **Report issued on:** 19/05/2021

Samples Analysed: 7 soil samples

ned:

Signed:

Joanna Wawrzeczko Technical Reviewer (Reporting Team) For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41 -711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are :

soils - 4 weeks from reporting leachates - 2 weeks from reporting waters - 2 weeks from reporting asbestos - 6 months from reporting

Excel copies of reports are only valid when accompanied by this PDF certificate.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies.

An estimate of measurement uncertainty can be provided on request.





Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	11	8.0	3.6	7.3	4.5
Total mass of sample received	kg	0.001	NONE	1.0	1.2	1.2	0.50	0.50
Asbestos in Soil Screen / Identification Name	Type	N/A	ISO 17025	-	-	-	-	-
Asbestos in Soil	Type	N/A	ISO 17025	Not-detected	Not-detected	Not-detected	Not-detected	Not-detected
Asbestos Quantification (Stage 2)	%	0.001	ISO 17025	-	-	-	-	-
Asbestos Quantification Total	%	0.001	ISO 17025	-	-	-	-	-
General Inorganics								
pH - Automated	pH Units	N/A	MCERTS	7.9	10.3	7.7	7.9	7.7
Total Cyanide	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Total Sulphate as SO4	mg/kg	50	MCERTS	3800	6100	630	240	4000
Water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.095	0.67	0.022	0.096	0.46
Sulphide	mg/kg	1	MCERTS	2.8	17	21	3.2	13
Organic Matter	%	0.1	MCERTS	-	-	-	-	1.1
Total Organic Carbon (TOC)	%	0.1	MCERTS	5.0	0.5	0.1	0.2	0.6
Total Phenols Total Phenols (monohydric)	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Speciated PAHs								
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Fluorene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Phenanthrene	mg/kg	0.05	MCERTS	2.0	1.7	< 0.05	0.59	< 0.05
Anthracene	mg/kg	0.05	MCERTS	0.46	0.44	< 0.05	0.27	< 0.05
Fluoranthene	mg/kg	0.05	MCERTS	4.6	2.6	0.35	1.9	0.49
Pyrene	mg/kg	0.05	MCERTS	4.3	2.8	0.39	1.9	0.74
Benzo(a)anthracene	mg/kg	0.05	MCERTS	2.6	1.5	< 0.05	0.71	< 0.05
Chrysene	mg/kg	0.05	MCERTS	3.0	1.5	< 0.05	1.2	< 0.05
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	3.3	1.4	< 0.05	1.1	< 0.05
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	2.1	0.55	< 0.05	0.72	< 0.05
Benzo(a)pyrene	mg/kg	0.05	MCERTS	3.2	1.0	< 0.05	1.1	< 0.05
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	1.6	0.67	< 0.05	0.60	< 0.05
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS MCERTS	0.54	0.24	< 0.05	< 0.05	< 0.05
Benzo(ghi)perylene	mg/kg	0.05	MICERIS	2.0	0.71	< 0.05	0.72	< 0.05
Total PAH								
		0.8	MCERTS					





Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status	None Supplied				
Heavy Metals / Metalloids								
Arsenic (aqua regia extractable)	mg/kg	1	MCERTS	65	14	26	20	9.9
Boron (water soluble)	mg/kg	0.2	MCERTS	1.3	2.6	0.3	< 0.2	0.6
Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	1.0	0.4	< 0.2	< 0.2	< 0.2
Chromium (hexavalent)	mg/kg	4	MCERTS	< 4.0	< 4.0	< 4.0	< 4.0	< 4.0
Chromium (aqua regia extractable)	mg/kg	1	MCERTS	20	25	11	27	22
Copper (aqua regia extractable)	mg/kg	1	MCERTS	110	37	11	15	28
Lead (aqua regia extractable)	mg/kg	1	MCERTS	440	120	58	20	40
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	1.3	0.3	< 0.3	< 0.3	< 0.3
Nickel (aqua regia extractable)	mg/kg	1	MCERTS	33	19	9.9	22	15
Selenium (agua regia extractable)	mg/kg	1	MCERTS	< 1.0	< 1.0	3.6	2.2	< 1.0
Zinc (aqua regia extractable)	mg/kg	1	MCERTS	310	130	140	63	68
Monoaromatics & Oxygenates Benzene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Toluene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Ethylbenzene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
p & m-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
o-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
MTBE (Methyl Tertiary Butyl Ether)	μg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Petroleum Hydrocarbons TPH C10 - C40	mg/kg	10	MCERTS	74	220	< 10	100	470
TPH-CWG - Aliphatic >EC5 - EC6	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aliphatic >EC6 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aliphatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aliphatic >EC10 - EC12	mg/kg	1	MCERTS	< 1.0	1.1	< 1.0	< 1.0	< 1.0
TPH-CWG - Aliphatic >EC12 - EC16	mg/kg	2	MCERTS	< 2.0	5.9	< 2.0	2.6	10
TPH-CWG - Aliphatic >EC16 - EC21	mg/kg	8	MCERTS	< 8.0	16	< 8.0	< 8.0	35
TPH-CWG - Aliphatic >EC21 - EC35	mg/kg	8	MCERTS	< 8.0	62	< 8.0	41	250
TPH-CWG - Aliphatic (EC5 - EC35)	mg/kg	10	MCERTS	< 10	85	< 10	49	300
				•				
TPH-CWG - Aromatic >EC5 - EC7	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aromatic >EC7 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aromatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001
TPH-CWG - Aromatic >EC10 - EC12	mg/kg	1	MCERTS	7.1	< 1.0	< 1.0	< 1.0	< 1.0
			MCERTS	12	7.0	< 2.0	< 2.0	5.7
TPH-CWG - Aromatic >EC12 - EC16	mg/kg	2	MCERTS	12				
	mg/kg mg/kg	10	MCERTS	19	24	< 10	14	24
TPH-CWG - Aromatic >EC12 - EC16								





I - b CI- Nb		1052005	1052006	1052007	1053000	1052000		
Lab Sample Number		1852805	1852806	1852807	1852808	1852809		
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
VOCs			1		1			
Chloromethane	μg/kg	1	ISO 17025		< 1.0	_	_	_
Chloroethane	μg/kg	1	NONE		< 1.0	-	_	_
Bromomethane	μg/kg	1	ISO 17025		< 1.0			
Vinyl Chloride	μg/kg	1	NONE	<u> </u>	< 1.0	-	<u> </u>	
Trichlorofluoromethane	μg/kg	1	NONE	-	< 1.0	-	_	_
1,1-Dichloroethene	µg/kg	1	NONE		< 1.0			
1,1,2-Trichloro 1,2,2-Trifluoroethane	μg/kg	1	ISO 17025		< 1.0	-	-	-
Cis-1,2-dichloroethene	μg/kg	1	MCERTS		< 1.0	-	-	-
MTBE (Methyl Tertiary Butyl Ether)	μg/kg	1	MCERTS	-	< 1.0			
1,1-Dichloroethane	μg/kg	1	MCERTS		< 1.0		<u> </u>	-
2,2-Dichloropropane	μg/kg	1	MCERTS		< 1.0	-	-	-
Z,2-Dichloropropane Trichloromethane	μg/kg	1	MCERTS		< 1.0	-	-	-
	μg/kg	1	MCERTS		< 1.0			
1,1,1-Trichloroethane 1,2-Dichloroethane	μg/kg	1	MCERTS		< 1.0	-		
		1	MCERTS	-		-	-	-
1,1-Dichloropropene	μg/kg	1	NONE	-	< 1.0	-		-
Trans-1,2-dichloroethene	μg/kg			-	< 1.0	-	-	-
Benzene	μg/kg "	1	MCERTS	-	< 1.0	-	-	-
Tetrachloromethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,2-Dichloropropane	μg/kg 	1	MCERTS	-	< 1.0	-	-	-
Trichloroethene	μg/kg 	1	MCERTS	-	< 1.0	-	-	-
Dibromomethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Bromodichloromethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Cis-1,3-dichloropropene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Trans-1,3-dichloropropene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Toluene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,1,2-Trichloroethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,3-Dichloropropane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Dibromochloromethane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Tetrachloroethene	μg/kg	1	NONE	-	< 1.0	-	-	-
1,2-Dibromoethane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
Chlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,1,1,2-Tetrachloroethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Ethylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
p & m-Xylene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Styrene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Tribromomethane	μg/kg	1	NONE	-	< 1.0	-	-	-
o-Xylene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,1,2,2-Tetrachloroethane	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Isopropylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Bromobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
n-Propylbenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
2-Chlorotoluene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
4-Chlorotoluene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,3,5-Trimethylbenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
tert-Butylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,2,4-Trimethylbenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
sec-Butylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,3-Dichlorobenzene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
p-Isopropyltoluene	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
1,2-Dichlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,4-Dichlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Butylbenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-





Lab Sample Number		1852805	1852806	1852807	1852808	1852809		
Sample Reference		BH101	BH102	BH103	BH104	BH105		
Sample Number	None Supplied	None Supplied	None Supplied	None Supplied	None Supplied			
Depth (m)	0.80	0.50	0.20	1.80	0.50			
Date Sampled	21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021			
Time Taken	None Supplied	None Supplied	None Supplied	None Supplied	None Supplied			
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
1,2-Dibromo-3-chloropropane	μg/kg	1	ISO 17025	-	< 1.0	-	-	-
1,2,4-Trichlorobenzene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
Hexachlorobutadiene	μg/kg	1	MCERTS	-	< 1.0	-	-	-
1,2,3-Trichlorobenzene	µg/kg	1	ISO 17025	-	< 1.0	-	-	-





				1052005	1053006	1050007	1053000	105000
Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled				21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
SVOCs								
Aniline	mg/kg	0.1	NONE	-	< 0.1	-	-	-
Phenol	mg/kg	0.2	ISO 17025	_	< 0.2	_	-	-
2-Chlorophenol	mg/kg	0.1	MCERTS	_	< 0.1	_	_	_
Bis(2-chloroethyl)ether	mg/kg	0.2	MCERTS	_	< 0.2	_	_	_
1,3-Dichlorobenzene	mg/kg	0.2	MCERTS	-	< 0.2	-	_	_
1,2-Dichlorobenzene	mg/kg	0.1	MCERTS		< 0.1		-	
	mg/kg	0.2	MCERTS	-	< 0.2	-		
1,4-Dichlorobenzene	mg/kg	0.1	MCERTS		< 0.2	-	-	
Bis(2-chloroisopropyl)ether	mg/kg	0.1	MCERTS	-				
2-Methylphenol	mg/kg	0.05	MCERTS	-	< 0.3	-	-	-
Hexachloroethane	mg/kg mg/kg	0.05	MCERTS	-	< 0.05	-	-	
Nitrobenzene					< 0.3			-
4-Methylphenol	mg/kg	0.2	NONE	-	< 0.2	-	-	-
Isophorone	mg/kg		MCERTS	-	< 0.2	-	-	-
2-Nitrophenol	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
2,4-Dimethylphenol	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
Bis(2-chloroethoxy)methane	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
1,2,4-Trichlorobenzene	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
Naphthalene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
2,4-Dichlorophenol	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
4-Chloroaniline	mg/kg	0.1	NONE	-	< 0.1	-	-	-
Hexachlorobutadiene	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
4-Chloro-3-methylphenol	mg/kg	0.1	NONE	-	< 0.1	-	-	-
2,4,6-Trichlorophenol	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
2,4,5-Trichlorophenol	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
2-Methylnaphthalene	mg/kg	0.1	NONE	-	< 0.1	-	-	-
2-Chloronaphthalene	mg/kg	0.1	MCERTS		< 0.1	-	-	-
Dimethylphthalate	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
2,6-Dinitrotoluene	mg/kg	0.1	MCERTS	-	< 0.1	-	-	-
Acenaphthylene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
Acenaphthene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
2,4-Dinitrotoluene	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
Dibenzofuran	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
4-Chlorophenyl phenyl ether	mg/kg	0.3	ISO 17025	-	< 0.3	-	-	-
Diethyl phthalate	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
4-Nitroaniline	mg/kg	0.2	MCERTS	-	< 0.2	-	-	-
Fluorene	mg/kg	0.05	MCERTS	-	< 0.05	-	-	-
Azobenzene	mg/kg	0.3	MCERTS	-	< 0.3	-	-	-
Bromophenyl phenyl ether	mg/kg	0.2	MCERTS	-	< 0.2	-	_	-
Hexachlorobenzene	mg/kg	0.3	MCERTS	_	< 0.3	_	_	_
Phenanthrene	mg/kg	0.05	MCERTS	-	1.7	-	_	_
Anthracene	mg/kg	0.05	MCERTS	-	0.44	-	_	-
Carbazole	mg/kg	0.3	MCERTS	_	< 0.3	_	_	-
Dibutyl phthalate	mg/kg	0.2	MCERTS	_	< 0.2	_	_	_
Anthraquinone	mg/kg	0.3	MCERTS	_	< 0.3	_	_	_
Fluoranthene	mg/kg	0.05	MCERTS	-	2.6	-	-	-
Pyrene	mg/kg	0.05	MCERTS	-	2.8		<u> </u>	-
Butyl benzyl phthalate	mg/kg	0.03	ISO 17025	-	< 0.3	-		
Benzo(a)anthracene	mg/kg	0.05	MCERTS	-	1.5	-	-	-
Benzo(a)anthracene Chrysene	mg/kg	0.05	MCERTS	-	1.5	-	-	-
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS					-
	mg/kg	0.05	MCERTS	-	1.4	-	-	
Benzo(k)fluoranthene	_	0.05	MCERTS	-	0.55	-	-	-
Benzo(a)pyrene	mg/kg	0.05	PICERIS	-	1.0	-	-	-





Lab Sample Number				1852805	1852806	1852807	1852808	1852809
Sample Reference				BH101	BH102	BH103	BH104	BH105
Sample Number				None Supplied				
Depth (m)				0.80	0.50	0.20	1.80	0.50
Date Sampled			21/04/2021	21/04/2021	21/04/2021	22/04/2021	23/04/2021	
Time Taken			None Supplied	None Supplied	None Supplied	None Supplied	None Supplied	
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	-	0.67	-	-	=
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	-	0.24	-	-	-
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	-	0.71	-	-	-
PCBs by GC-MS								
PCB Congener 28	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 52	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 101	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 118	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 138	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 153	mg/kg	0.001	MCERTS	-	-	-	-	-
PCB Congener 180	mg/kg	0.001	MCERTS	-	-	-	-	-
Total PCBs by GC-MS								
Total PCBs	mg/kg	0.007	MCERTS	-	-	-	-	-

 $\label{eq:US} \text{U/S} = \text{Unsuitable Sample} \qquad \text{I/S} = \ \text{Insufficient Sample}$





Lab Sample Number				1852810	1852811	
Sample Reference	Sample Reference					
Sample Number	None Supplied	None Supplied 0.40				
Depth (m)	0.80					
Date Sampled				26/04/2021	27/04/2021	
Time Taken	None Supplied	None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status			
Stone Content	%	0.1	NONE	< 0.1	< 0.1	
Moisture Content	%	0.01	NONE	21	5.8	
Total mass of sample received	kg	0.001	NONE	0.80	1.0	
Asbestos in Soil Screen / Identification Name	Туре	N/A	ISO 17025	-	Chrysotile	
Asbestos in Soil	Туре	N/A	ISO 17025	Not-detected	Detected	
Asbestos Quantification (Stage 2)	%	0.001	ISO 17025		0.005	
Asbestos Quantification Total	%	0.001	ISO 17025	-	0.005	
General Inorganics						
pH - Automated	pH Units	N/A	MCERTS	7.4	10.1	
Total Cyanide	mg/kg	1	MCERTS	120	< 1.0	
Total Sulphate as SO4	mg/kg	50	MCERTS	1000	1000	
water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.46	0.15	
Sulphide	mg/kg	1	MCERTS	140	17	
Organic Matter	%	0.1	MCERTS	1.8	-	
Total Organic Carbon (TOC)	%	0.1	MCERTS	1.1	0.2	
Total Phenols						
Total Phenols (monohydric)	mg/kg	1	MCERTS	< 1.0	< 1.0	
Speciated PAHs						
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	

Naphthalene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Fluorene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Phenanthrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Anthracene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Fluoranthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Pyrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(a)anthracene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Chrysene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(a)pyrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05	< 0.05
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	< 0.05	< 0.05

Total PAH

Speciated Total EPA-16 PAHs	mg/kg	0.8	MCERTS	< 0.80	< 0.80





Lab Sample Number				1852810	1852811
Sample Reference				TP101	TP102
Sample Number				None Supplied	None Supplied
Depth (m)				0.80	0.40
Date Sampled				26/04/2021	27/04/2021
Time Taken				None Supplied	None Supplied
Time raken	1	-	1	чоне заррнеа	None Supplied
		Limit of detectior	Ac		
Analytical Parameter	Units	약	Accreditation Status		
(Soil Analysis)	ਫ਼ੋਂ	lete	itat		
		Ctio	ion		
Hanny Makala / Makallaida		5			
Heavy Metals / Metalloids	mg/kg	1	MCERTS	15	14
Arsenic (aqua regia extractable)	mg/kg	0.2	MCERTS	1.8	0.5
Boron (water soluble) Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	< 0.2	< 0.2
Chromium (hexavalent)	mg/kg	4	MCERTS	< 4.0	< 4.0
Chromium (nexavalent) Chromium (aqua regia extractable)	mg/kg	1	MCERTS	28	19
Copper (aqua regia extractable)	mg/kg	1	MCERTS	62	35
Lead (aqua regia extractable)	mg/kg	1	MCERTS	270	140
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	< 0.3	< 0.3
Nickel (agua regia extractable)	mg/kg	1	MCERTS	17	17
Selenium (aqua regia extractable)	mg/kg	1	MCERTS	2.9	< 1.0
Zinc (aqua regia extractable)	mg/kg	1	MCERTS	130	150
=no (aqaa regia esta acaasie)	<u> </u>			150	150
Monoaromatics & Oxygenates					
Benzene	μg/kg	1	MCERTS	< 1.0	< 1.0
Toluene	μg/kg	1	MCERTS	< 1.0	< 1.0
Ethylbenzene	μg/kg	1	MCERTS	< 1.0	< 1.0
p & m-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0
o-xylene	μg/kg	1	MCERTS	< 1.0	< 1.0
MTBE (Methyl Tertiary Butyl Ether)	µg/kg	1	MCERTS	< 1.0	< 1.0
Petroleum Hydrocarbons					
TPH C10 - C40	mg/kg	10	MCERTS	49	< 10
TPH-CWG - Aliphatic >EC5 - EC6	mg/kg	0.001	MCERTS	< 0.001	< 0.001
TPH-CWG - Aliphatic >EC6 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001
TPH-CWG - Aliphatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001	< 0.001
TPH-CWG - Aliphatic >EC10 - EC12	mg/kg	1	MCERTS	< 1.0	< 1.0
TPH-CWG - Aliphatic >EC12 - EC16	mg/kg	2	MCERTS	13	< 2.0
TPH-CWG - Aliphatic >EC16 - EC21	mg/kg	8	MCERTS	18	< 8.0
TPH-CWG - Aliphatic >EC21 - EC35	mg/kg	8	MCERTS	18	< 8.0
TPH-CWG - Aliphatic (EC5 - EC35)	mg/kg	10	MCERTS	49	< 10
			MOEDE:		
TPH-CWG - Aromatic >EC5 - EC7	mg/kg	0.001	MCERTS	< 0.001	< 0.001
TPH-CWG - Aromatic >EC7 - EC8	mg/kg	0.001	MCERTS	< 0.001	< 0.001
TPH-CWG - Aromatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001	< 0.001
TPH-CWG - Aromatic >EC10 - EC12	mg/kg	1	MCERTS MCERTS	< 1.0	< 1.0
TPH-CWG - Aromatic >EC12 - EC16	mg/kg mg/kg	2 10	MCERTS	< 2.0	< 2.0
TPH-CWG - Aromatic > EC16 - EC21	mg/kg mg/kg	10	MCERTS	< 10	< 10
TPH-CWG - Aromatic >EC21 - EC35	mg/kg mg/kg	10	MCERTS	< 10	< 10
TPH-CWG - Aromatic (EC5 - EC35)	mg/kg	10	PICERTS	< 10	< 10





	ah Sample Number							
Lab Sample Number				1852810	1852811			
Sample Reference				TP101	TP102			
Sample Number				None Supplied	None Supplied			
Depth (m)				0.80	0.40			
Date Sampled				26/04/2021	27/04/2021			
Time Taken				None Supplied	None Supplied			
		Limit of detection	>					
Analytical Parameter	_	of of	St					
(Soil Analysis)	Units	det	creditat Status					
	-	ect	Accreditation Status					
		ion	•					
VOCs								
Chloromethane	µg/kg	1	ISO 17025	< 1.0	-			
Chloroethane	μg/kg	1	NONE	< 1.0	-			
Bromomethane	μg/kg	1	ISO 17025	< 1.0	-			
Vinyl Chloride	μg/kg	1	NONE	< 1.0	-			
Trichlorofluoromethane	μg/kg	1	NONE	< 1.0	-			
1,1-Dichloroethene	μg/kg	1	NONE	< 1.0	-			
1,1,2-Trichloro 1,2,2-Trifluoroethane	μg/kg	1	ISO 17025	< 1.0	-			
Cis-1,2-dichloroethene	μg/kg	1	MCERTS	< 1.0	-			
MTBE (Methyl Tertiary Butyl Ether)	μg/kg	1	MCERTS	< 1.0	-			
1,1-Dichloroethane	μg/kg	1	MCERTS	< 1.0	-			
2,2-Dichloropropane	μg/kg	1	MCERTS	< 1.0	-			
Trichloromethane	μg/kg	1	MCERTS	< 1.0	-			
1,1,1-Trichloroethane	μg/kg	1	MCERTS	< 1.0	-			
1,2-Dichloroethane	μg/kg	1	MCERTS	< 1.0	-			
1,1-Dichloropropene	μg/kg	1	MCERTS	< 1.0	-			
Trans-1,2-dichloroethene	μg/kg	1	NONE	< 1.0	-			
Benzene	μg/kg	1	MCERTS	< 1.0	1			
Tetrachloromethane	μg/kg	1	MCERTS	< 1.0	-			
1,2-Dichloropropane	μg/kg	1	MCERTS	< 1.0	-			
Trichloroethene	μg/kg	1	MCERTS	< 1.0	-			
Dibromomethane	μg/kg	1	MCERTS	< 1.0	1			
Bromodichloromethane	μg/kg	1	MCERTS	< 1.0	-			
Cis-1,3-dichloropropene	μg/kg	1	ISO 17025	< 1.0	-			
Trans-1,3-dichloropropene	µg/kg	1	ISO 17025	< 1.0	-			
Toluene	μg/kg	1	MCERTS	< 1.0	-			
1,1,2-Trichloroethane	μg/kg	1	MCERTS	< 1.0	-			
1,3-Dichloropropane	µg/kg	1	ISO 17025	< 1.0	-			
Dibromochloromethane	μg/kg	1	ISO 17025	< 1.0	-			
Tetrachloroethene	μg/kg	1	NONE	< 1.0	-			
1,2-Dibromoethane	μg/kg	1	ISO 17025	< 1.0	-			
Chlorobenzene	μg/kg	1	MCERTS	< 1.0	-			
1,1,1,2-Tetrachloroethane	μg/kg	1	MCERTS	< 1.0	-			
Ethylbenzene	μg/kg	1	MCERTS	< 1.0	-			
p & m-Xylene	μg/kg	1	MCERTS	< 1.0	-			
Styrene	μg/kg	1	MCERTS	< 1.0	-			
Tribromomethane	μg/kg	1	NONE	< 1.0	-			
o-Xylene	μg/kg	1	MCERTS	< 1.0	-			
1,1,2,2-Tetrachloroethane	μg/kg	1	MCERTS	< 1.0	-			
Isopropylbenzene	μg/kg	1	MCERTS	< 1.0	-			
Bromobenzene	μg/kg	1	MCERTS	< 1.0	-			
n-Propylbenzene	μg/kg	1	ISO 17025	< 1.0	-			
2-Chlorotoluene	μg/kg	1	MCERTS	< 1.0	-			
4-Chlorotoluene	μg/kg	1	MCERTS	< 1.0	-			
1,3,5-Trimethylbenzene	μg/kg	1	ISO 17025	< 1.0	-			
tert-Butylbenzene	µg/kg	1	MCERTS	< 1.0	-			
		1	ISO 17025	< 1.0	-			
1,2,4-Trimethylbenzene	μg/kg							
1,2,4-Trimethylbenzene sec-Butylbenzene	μg/kg	1	MCERTS	< 1.0	-			
	µg/kg µg/kg	1 1	ISO 17025	< 1.0 < 1.0	-			
sec-Butylbenzene	μg/kg	1 1 1	ISO 17025 ISO 17025					
sec-Butylbenzene 1,3-Dichlorobenzene	µg/kg µg/kg	1 1 1 1	ISO 17025 ISO 17025 MCERTS	< 1.0	-			
sec-Butylbenzene 1,3-Dichlorobenzene p-Isopropyltoluene	µg/kg µg/kg µg/kg	1 1 1	ISO 17025 ISO 17025	< 1.0 < 1.0	-			





Lab Sample Number	ab Sample Number					
Sample Reference	TP101	TP102				
Sample Number				None Supplied	None Supplied	
Depth (m)				0.80	0.40	
Date Sampled				26/04/2021	27/04/2021	
Time Taken	None Supplied	None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status			
1,2-Dibromo-3-chloropropane	μg/kg	1	ISO 17025	< 1.0	-	
1,2,4-Trichlorobenzene	μg/kg	1	MCERTS	< 1.0	-	
Hexachlorobutadiene	μg/kg	1	MCERTS	< 1.0	-	
1,2,3-Trichlorobenzene	μg/kg	1	ISO 17025	< 1.0	-	





Lab Sample Number				1852810	1852811	
					TP102	
Sample Reference Sample Number				TP101		
•				None Supplied	None Supplied	
Depth (m)				0.80	0.40	
Date Sampled				26/04/2021	27/04/2021	
Time Taken			1	None Supplied	None Supplied	
		Limit of detection	Þ			
Analytical Parameter	_	of of	Accreditation Status			
(Soil Analysis)	Units	de	creditat Status			
,		ect	stio			
		ġ	-			
SVOCs						
Aniline	mg/kg	0.1	NONE	< 0.1	-	
Phenol	mg/kg	0.2	ISO 17025	< 0.2	-	
2-Chlorophenol	mg/kg	0.1	MCERTS	< 0.1	-	
Bis(2-chloroethyl)ether	mg/kg	0.2	MCERTS	< 0.2	-	
1,3-Dichlorobenzene	mg/kg	0.2	MCERTS	0.5	-	
1,2-Dichlorobenzene	mg/kg	0.1	MCERTS	< 0.1	-	
1,4-Dichlorobenzene	mg/kg	0.2	MCERTS	< 0.2	-	
Bis(2-chloroisopropyl)ether	mg/kg	0.1	MCERTS	< 0.1	-	
2-Methylphenol	mg/kg	0.3	MCERTS	< 0.3	-	
Hexachloroethane	mg/kg	0.05	MCERTS	< 0.05	-	
Nitrobenzene	mg/kg	0.3	MCERTS	< 0.3	-	
4-Methylphenol	mg/kg	0.2	NONE	< 0.2	-	
Isophorone	mg/kg	0.2	MCERTS	< 0.2	-	
2-Nitrophenol	mg/kg	0.3	MCERTS	< 0.3	-	
2,4-Dimethylphenol	mg/kg	0.3	MCERTS	< 0.3	-	
Bis(2-chloroethoxy)methane	mg/kg	0.3	MCERTS	< 0.3	-	
1,2,4-Trichlorobenzene	mg/kg	0.3	MCERTS	< 0.3	-	
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	-	
2.4-Dichlorophenol	mg/kg	0.3	MCERTS	< 0.3	-	
4-Chloroaniline	mg/kg	0.1	NONE	< 0.1	-	
Hexachlorobutadiene	mg/kg	0.1	MCERTS	< 0.1	-	
4-Chloro-3-methylphenol	mg/kg	0.1	NONE	< 0.1	_	
2,4,6-Trichlorophenol	mg/kg	0.1	MCERTS	< 0.1	-	
2,4,5-Trichlorophenol	mg/kg	0.2	MCERTS	< 0.2	-	
2-Methylnaphthalene	mg/kg	0.1	NONE	< 0.1	_	
2-Chloronaphthalene	mg/kg	0.1	MCERTS	< 0.1	-	
Dimethylphthalate	mg/kg	0.1	MCERTS	< 0.1	-	
2,6-Dinitrotoluene	mg/kg	0.1	MCERTS	< 0.1	-	
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05	_	
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05	-	
2,4-Dinitrotoluene	mg/kg	0.2	MCERTS	< 0.2	_	
Dibenzofuran	mg/kg	0.2	MCERTS	< 0.2	_	
4-Chlorophenyl phenyl ether	mg/kg	0.3	ISO 17025	< 0.3	-	
Diethyl phthalate	mg/kg	0.2	MCERTS	< 0.2	_	
4-Nitroaniline	mg/kg	0.2	MCERTS	< 0.2	-	
Fluorene	mg/kg	0.05	MCERTS	< 0.05		
Azobenzene	mg/kg	0.3	MCERTS	< 0.3	-	
Bromophenyl phenyl ether	mg/kg	0.3	MCERTS	< 0.3	-	
Hexachlorobenzene	mg/kg	0.2	MCERTS	< 0.3	-	
Phenanthrene	mg/kg	0.05	MCERTS	< 0.05	-	
Anthracene	mg/kg	0.05	MCERTS	< 0.05	-	
Carbazole	mg/kg	0.03	MCERTS	< 0.05	-	
Dibutyl phthalate	mg/kg	0.2	MCERTS	< 0.2	-	
Anthraguinone	mg/kg	0.2	MCERTS	< 0.2	-	
•	mg/kg	0.05	MCERTS	< 0.05	-	
Fluoranthene	mg/kg	0.05	MCERTS		-	
Pyrene Pyrene	mg/kg	0.05	ISO 17025	< 0.05	-	
Butyl benzyl phthalate	mg/kg	0.05	MCERTS	< 0.3		
Benzo(a)anthracene		0.05		< 0.05	-	
Chrysene	mg/kg		MCERTS	< 0.05	-	
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	=	
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	< 0.05	-	
Benzo(a)pyrene	mg/kg	0.05	MCERTS	< 0.05	-	





Lab Sample Number	1852810	1852811			
Sample Reference				TP101	TP102
Sample Number				None Supplied	None Supplied
Depth (m)				0.80	0.40
Date Sampled				26/04/2021	27/04/2021
Time Taken	None Supplied	None Supplied			
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status		
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	< 0.05	-
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05	-
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	< 0.05	-

PCBs by GC-MS

PCB Congener 28	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 52	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 101	mg/kg	0.001	MCERTS	< 0.001	=
PCB Congener 118	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 138	mg/kg	0.001	MCERTS	< 0.001	-
PCB Congener 153	mg/kg	0.001	MCERTS	< 0.001	
PCB Congener 180	mg/kg	0.001	MCERTS	< 0.001	-

Total PCBs by GC-MS

1044111 022 27 00 110					
Total PCBs	mg/kg	0.007	MCERTS	< 0.007	-

 $\label{eq:U/S} \text{U/S} = \text{Unsuitable Sample} \qquad \text{I/S} = \ \text{Insufficient Sample}$





Your Order No:

Certificate of Analysis - Asbestos Quantification

Methods:

Qualitative Analysis

The samples were analysed qualitatively for asbestos by polarising light and dispersion staining as described by the Health and Safety Executive in HSG 248.

Quantitative Analysis

The analysis was carried out using our documented in-house method A006-PL based on HSE Contract Research Report No: 83/1996: Development and Validation of an analytical method to determine the amount of asbestos in soils and loose aggregates (Davies et al, 1996) and HSG 248. Our method includes initial examination of the entire representative sample, then fractionation and detailed analysis of each fraction, with quantification by hand picking and weighing.

The limit of detection (reporting limit) of this method is 0.001 %.

The method has been validated using samples of at least 100 g, results for samples smaller than this should be interpreted with caution.

Both Qualitative and Quantitative Analyses are UKAS accredited.

Sample Number	Sample ID	Sample Depth (m)	Sample Weight (g)	Asbestos Containing Material Types Detected (ACM)	PLM Results	Asbestos by hand picking/weighing (%)	Total % Asbestos in Sample
1852811	TP102	0.40	188	Loose Fibrous Debris	Chrysotile	0.005	0.005

Opinions and interpretations expressed herein are outside the scope of UKAS accreditation.





* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
1852805	BH101	None Supplied	0.8	Brown sandy loam with gravel and rubble.
1852806	BH102	None Supplied	0.5	Brown sandy loam with gravel and rubble.
1852807	BH103	None Supplied	0.2	Brown sandy loam with gravel.
1852808	BH104	None Supplied	1.8	Brown loam and sand with gravel.
1852809	BH105	None Supplied	0.5	Brown loam and sand with gravel and brick.
1852810	TP101	None Supplied	0.8	Brown loam and clay with gravel and brick.
1852811	TP102	None Supplied	0.4	Light brown loam and sand with rubble.





Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Sulphate, water soluble, in soil (16hr extraction)	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In house method.	L038-PL	D	MCERTS
Metals in soil by ICP-OES	s in soil by ICP-OES Determination of metals in soil by aqua-regia digestion followed by ICP-OES. In-house method based on MEWAM 2006 Methods for the Determination of Metals in Soil.		L038-PL	D	MCERTS
Asbestos identification in soil	stos identification in soil Asbestos Identification with the use of polarised light microscopy in conjunction with disperion staining techniques. In house method based on HSG 248		A001-PL	D	ISO 17025
Boron, water soluble, in soil	Determination of water soluble boron in soil by hot water extract followed by ICP-OES.	In-house method based on Second Site Properties version 3	L038-PL	D	MCERTS
Hexavalent chromium in soil	Determination of hexavalent chromium in soil by extraction in water then by acidification, addition of 1,5 diphenylcarbazide followed by colorimetry.	In-house method	L080-PL	W	MCERTS
Moisture Content	Moisture content, determined gravimetrically. (30 oC)	In house method.	L019-UK/PL	W	NONE
Monohydric phenols in soil	ohydric phenols in soil Determination of phenols in soil by extraction with sodium hydroxide followed by distillation followed by colorimetry. Determination of phenols in soil by extraction with sodium hydroxide followed by distillation followed by and Wastewater 20th Edition: Clesceri, Greenberg & Eaton (skalar)		L080-PL	w	MCERTS
Organic matter (Automated) in soil	anic matter (Automated) in soil Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate.		L009-PL	D	MCERTS
Speciated EPA-16 PAHs in soil	ciated EPA-16 PAHs in soil Determination of PAH compounds in soil by extraction in dichloromethane and hexane followed by GC-MS with the use of surrogate and internal standards.		L064-PL	D	MCERTS
PCB's By GC-MS in soil	PCB's By GC-MS in soil Determination of PCB by extraction with acetone and hexane followed by GC-MS. In-hous		L027-PL	D	MCERTS
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement.	In house method.	L099-PL	D	MCERTS
Sulphide in soil	Determination of sulphide in soil by acidification and heating to liberate hydrogen sulphide, trapped in an alkaline solution then assayed by ion selective electrode.	In-house method	L010-PL	D	MCERTS
Total sulphate (as SO4 in soil)	Determination of total sulphate in soil by extraction with 10% HCl followed by ICP-OES.	In house method.	L038-PL	D	MCERTS
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight.	In-house method based on British Standard Methods and MCERTS requirements.	L019-UK/PL	D	NONE
Semi-volatile organic compounds in soil Determination of semi-volatile organic compounds in soil by extraction in dichloromethane and hexane followed by GC-MS.		In-house method based on USEPA 8270	L064-PL	D	MCERTS
Total cyanide in soil	Determination of total cyanide by distillation followed by colorimetry.	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton (Skalar)	L080-PL	W	MCERTS
Total organic carbon (Automated) in soil	Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate.	In house method.	L009-PL	D	MCERTS





Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Volatile organic compounds in soil	Determination of volatile organic compounds in soil by headspace GC-MS.	In-house method based on USEPA8260	L073B-PL	W	MCERTS
BTEX and MTBE in soil (Monoaromatics)	Determination of BTEX in soil by headspace GC-MS.	In-house method based on USEPA8260	L073B-PL	W	MCERTS
TPHCWG (Soil)	Determination of hexane extractable hydrocarbons in soil by GC-MS/GC-FID.	In-house method with silica gel split/clean up.	L088/76-PL	W	MCERTS
TPH Banding in Soil by FID	Determination of hexane extractable hydrocarbons in soil by GC-FID.	In-house method, TPH with carbon banding and silica gel split/cleanup.	L076-PL	W	MCERTS
Asbestos Quantification - Gravimetric	Asbestos quantification by gravimetric method - in house method based on references.	HSE Report No: 83/1996, HSG 248, HSG 264 & SCA Blue Book (draft).	A006-PL	D	ISO 17025

For method numbers ending in 'UK' analysis have been carried out in our laboratory in the United Kingdom.

For method numbers ending in 'PL' analysis have been carried out in our laboratory in Poland.

Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.

Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.

Sample Deviation Report



Sample ID	Other ID	Sample Type	Lab Sample Number	Sample Deviation	Test Name	Test Ref	Test Deviation
BH101	None Supplied	S	1852805	С	Sulphide in soil	L010-PL	С
BH101	None Supplied	S	1852805	С	Total cyanide in soil	L080-PL	С
BH102	None Supplied	S	1852806	С	Sulphide in soil	L010-PL	С
BH102	None Supplied	S	1852806	С	Total cyanide in soil	L080-PL	С
BH103	None Supplied	S	1852807	С	Sulphide in soil	L010-PL	С
BH103	None Supplied	S	1852807	С	Total cyanide in soil	L080-PL	С





Philip Lewis LMB Geosolutions Ltd 28 Dresden Road London N19 3BD

i2 Analytical Ltd. 7 Woodshots Meadow, Croxley Green Business Park, Watford, Herts, WD18 8YS

t: 01923 225404 f: 01923 237404

e: philip@Imbgeosolutions.com e: reception@i2analytical.com

Analytical Report Number: 21-74598

Replaces Analytical Report Number: 21-74598, issue no. 1 Additional analysis undertaken.

Project / Site name: Manor Road Samples received on: 28/04/2021

Your job number: Samples instructed on/ 14/05/2021

Analysis started on:

Your order number: Analysis completed by: 24/05/2021

Report Issue Number: Report issued on: 24/05/2021

Samples Analysed: 1 soil sample

Signed:

Karolina Marek PL Head of Reporting Team For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41 -711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are :

- 4 weeks from reporting leachates - 2 weeks from reporting waters - 2 weeks from reporting asbestos - 6 months from reporting

Excel copies of reports are only valid when accompanied by this PDF certificate.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies. An estimate of measurement uncertainty can be provided on request.





Lab Sample Number				1869216				
Sample Reference				TP103				
Sample Number				None Supplied				
Depth (m)								
Date Sampled								
Time Taken				None Supplied				
		Ė	_					
		Limit of detection	Accreditation Status					
Analytical Parameter	Units	of d	reditat Status					
(Soil Analysis)	Ŗ	ete	tati					
		ctio	9					
Stone Content	%	0.1	NONE	< 0.1				
Moisture Content	%	0.01	NONE	3.6				
Total mass of sample received	kg	0.001	NONE	1.0				
Asbestos in Soil Screen / Identification Name	Type	N/A	ISO 17025	Chrysotile				
Asbestos in Soil	Type 0/-	N/A	ISO 17025	Detected				
Asbestos Quantification (Stage 2)	%	0.001	ISO 17025	0.008				
Asbestos Quantification Total	%	0.001	ISO 17025	0.008				
General Inorganics								
pH - Automated	pH Units	N/A	MCERTS	9.5				
Total Cyanide	mg/kg	1	MCERTS	< 1.0				
Total Sulphate as SO4	mg/kg	50	MCERTS	1500				
Water Soluble SO4 (2:1 Leach. Equiv.) 1hr extraction	g/l	0.00125	MCERTS	0.17				
Sulphide	mg/kg	1	MCERTS	33				
Total Organic Carbon (TOC)	%	0.1	MCERTS	0.2				
Total Phenols (monohydric)	mg/kg		MCERTS	< 1.0				
Speciated PAHs								
Naphthalene	mg/kg	0.05	MCERTS	< 0.05				
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05				
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05				
Fluorene	mg/kg	0.05	MCERTS	< 0.05				
Phenanthrene	mg/kg	0.05	MCERTS	< 0.05				
Anthracene	mg/kg	0.05	MCERTS	< 0.05				
Fluoranthene	mg/kg	0.05	MCERTS	0.39				
Pyrene	mg/kg	0.05	MCERTS	0.42				
Benzo(a)anthracene	mg/kg	0.05	MCERTS	0.38				
Chrysene	mg/kg	0.05	MCERTS	0.33				
Benzo(b)fluoranthene	mg/kg	0.05	MCERTS	0.47				
Benzo(k)fluoranthene	mg/kg	0.05	MCERTS	0.27				
Benzo(a)pyrene	mg/kg	0.05	MCERTS	0.46				
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	0.35				
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05				
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	0.40				
Total PAH	mg/kg	0.8	MCERTS	2.47				
Speciated Total EPA-16 PAHs	mg/kg	0.0	PICENTS	3.47				
Heavy Metals / Metalloids								
Arsenic (aqua regia extractable)	mg/kg	1	MCERTS	11				
Boron (water soluble)	mg/kg	0.2	MCERTS	0.2				
Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	< 0.2				
Chromium (hexavalent)	mg/kg	4	MCERTS	< 4.0				
			MCEDIC					

mg/kg

mg/kg

mg/kg

mg/kg

mg/kg

mg/kg mg/kg 1

1

0.3

1

1

Chromium (aqua regia extractable)

Copper (aqua regia extractable)

Mercury (aqua regia extractable)

Selenium (aqua regia extractable)

Lead (aqua regia extractable)

Nickel (aqua regia extractable)

Zinc (aqua regia extractable)

MCERTS

MCERTS

MCERTS

MCERTS

MCERTS

MCERTS

MCERTS

14

9.4

52

< 0.3

12

< 1.0

96





Lab Sample Number				1869216
Sample Reference				TP103
Sample Number				None Supplied
Depth (m)				0.50
Date Sampled	26/04/2021			
Time Taken				None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status	

Monoaromatics & Oxygenates

Benzene	μg/kg	1	MCERTS	< 1.0
Toluene	μg/kg	1	MCERTS	< 1.0
Ethylbenzene	μg/kg	1	MCERTS	< 1.0
p & m-xylene	μg/kg	1	MCERTS	< 1.0
o-xylene	μg/kg	1	MCERTS	< 1.0
MTBE (Methyl Tertiary Butyl Ether)	µg/kg	1	MCERTS	< 1.0

Petroleum Hydrocarbons

TPH C10 - C40	mg/kg	10	MCERTS	190
TPH-CWG - Aliphatic >EC5 - EC6	mg/kg	0.001	MCERTS	< 0.001
TPH-CWG - Aliphatic >EC6 - EC8	mg/kg	0.001	MCERTS	< 0.001
TPH-CWG - Aliphatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001
TPH-CWG - Aliphatic >EC10 - EC12	mg/kg	1	MCERTS	< 1.0
TPH-CWG - Aliphatic >EC12 - EC16	mg/kg	2	MCERTS	4.0
TPH-CWG - Aliphatic >EC16 - EC21	mg/kg	8	MCERTS	18
TPH-CWG - Aliphatic >EC21 - EC35	mg/kg	8	MCERTS	91
TPH-CWG - Aliphatic (EC5 - EC35)	mg/kg	10	MCERTS	110

TPH-CWG - Aromatic >EC5 - EC7	mg/kg	0.001	MCERTS	< 0.001
TPH-CWG - Albinduc >EC5 - EC7	mg/kg	0.001		< 0.001
TPH-CWG - Aromatic >EC7 - EC8	mg/kg	0.001	MCERTS	< 0.001
TPH-CWG - Aromatic >EC8 - EC10	mg/kg	0.001	MCERTS	< 0.001
TPH-CWG - Aromatic >EC10 - EC12	mg/kg	1	MCERTS	< 1.0
TPH-CWG - Aromatic >EC12 - EC16	mg/kg	2	MCERTS	< 2.0
TPH-CWG - Aromatic >EC16 - EC21	mg/kg	10	MCERTS	< 10
TPH-CWG - Aromatic >EC21 - EC35	mg/kg	10	MCERTS	34
TPH-CWG - Aromatic (EC5 - EC35)	mg/kg	10	MCERTS	37

 $\mbox{U/S} = \mbox{Unsuitable Sample} \hspace{0.5cm} \mbox{I/S} = \mbox{Insufficient Sample}$





Your Order No:

Certificate of Analysis - Asbestos Quantification

Methods:

Qualitative Analysis

The samples were analysed qualitatively for asbestos by polarising light and dispersion staining as described by the Health and Safety Executive in HSG 248.

Quantitative Analysis

The analysis was carried out using our documented in-house method A006-PL based on HSE Contract Research Report No: 83/1996: Development and Validation of an analytical method to determine the amount of asbestos in soils and loose aggregates (Davies et al, 1996) and HSG 248. Our method includes initial examination of the entire representative sample, then fractionation and detailed analysis of each fraction, with quantification by hand picking and weighing.

The limit of detection (reporting limit) of this method is 0.001 %.

The method has been validated using samples of at least 100 g, results for samples smaller than this should be interpreted with caution.

Both Qualitative and Quantitative Analyses are UKAS accredited.

	Sample Number	Sample ID	Sample Depth (m)	Sample Weight (g)	Asbestos Containing Material Types Detected (ACM)	PLM Results	Asbestos by hand picking/weighing (%)	Total % Asbestos in Sample
Γ	1869216	TP103	0.50	194	Loose Fibrous Debris	Chrysotile	0.008	0.008

Opinions and interpretations expressed herein are outside the scope of UKAS accreditation.





* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
1869216	TP103	None Supplied	0.5	Brown sand with gravel.





Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method	Wet / Dry	Accreditation
,	,	,	number	Analysis	Status
Sulphate, water soluble, in soil (16hr extraction)	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In house method.	L038-PL	D	MCERTS
Metals in soil by ICP-OES	Determination of metals in soil by aqua-regia digestion followed by ICP-OES.	In-house method based on MEWAM 2006 Methods for the Determination of Metals in Soil.	L038-PL	D	MCERTS
Asbestos identification in soil	Asbestos Identification with the use of polarised light microscopy in conjunction with disperion staining techniques.	In house method based on HSG 248	A001-PL	D	ISO 17025
Boron, water soluble, in soil	Determination of water soluble boron in soil by hot water extract followed by ICP-OES.	In-house method based on Second Site Properties version 3	L038-PL	D	MCERTS
Hexavalent chromium in soil	Determination of hexavalent chromium in soil by extraction in water then by acidification, addition of 1,5 diphenylcarbazide followed by colorimetry.	In-house method	L080-PL	W	MCERTS
Moisture Content	Moisture content, determined gravimetrically. (30 oC)	In house method.	L019-UK/PL	W	NONE
Monohydric phenols in soil	Determination of phenols in soil by extraction with sodium hydroxide followed by distillation followed by colorimetry.	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton (skalar)	L080-PL	W	MCERTS
Speciated EPA-16 PAHs in soil	Determination of PAH compounds in soil by extraction in dichloromethane and hexane followed by GC-MS with the use of surrogate and internal standards.	In-house method based on USEPA 8270	L064-PL	D	MCERTS
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement.	In house method.	L099-PL	D	MCERTS
Sulphide in soil	Determination of sulphide in soil by acidification and heating to liberate hydrogen sulphide, trapped in an alkaline solution then assayed by ion selective electrode.	In-house method	L010-PL	D	MCERTS
Total sulphate (as SO4 in soil)	Determination of total sulphate in soil by extraction with 10% HCl followed by ICP-OES.	In house method.	L038-PL	D	MCERTS
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight.	In-house method based on British Standard Methods and MCERTS requirements.	L019-UK/PL	D	NONE
Total cyanide in soil	Determination of total cyanide by distillation followed by colorimetry.	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton (Skalar)	L080-PL	W	MCERTS
Total organic carbon (Automated) in soil	Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate.	In house method.	L009-PL	D	MCERTS
BTEX and MTBE in soil (Monoaromatics)	Determination of BTEX in soil by headspace GC-MS.	In-house method based on USEPA8260	L073B-PL	W	MCERTS
TPHCWG (Soil)	Determination of hexane extractable hydrocarbons in soil by GC-MS/GC-FID.	In-house method with silica gel split/clean up.	L088/76-PL	W	MCERTS
TPH Banding in Soil by FID	Determination of hexane extractable hydrocarbons in soil by GC-FID.	In-house method, TPH with carbon banding and silica gel split/cleanup.	L076-PL	W	MCERTS





Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Asbestos Quantification - Gravimetric	Asbestos quantification by gravimetric method - in house method based on references.	HSE Report No: 83/1996, HSG 248, HSG 264 & SCA Blue Book (draft).	A006-PL	D	ISO 17025
Sulphate, water soluble, in soil (1hr extraction)	Sulphate, water soluble, in soil (1hr extraction)	In-house method	L038-PL	D	MCERTS

For method numbers ending in 'UK' analysis have been carried out in our laboratory in the United Kingdom.

For method numbers ending in 'PL' analysis have been carried out in our laboratory in Poland.

Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.

Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.

Sample Deviation Report



Sample ID	Other ID			Sample Deviation	Test Name	Test Ref	Test Deviation
TP103	None Supplied	S	1869216	С	BTEX and MTBE in soil (Monoaromatics)	L073B-PL	С
TP103	None Supplied	S	1869216	С	Sulphide in soil	L010-PL	С
TP103	None Supplied	S	1869216	С	Total cyanide in soil	L080-PL	С





Philip Lewis LMB Geosolutions Ltd 28 Dresden Road London N19 3BD

i2 Analytical Ltd.
7 Woodshots Meadow,
Croxley Green
Business Park,
Watford,
Herts,
WD18 8YS

t: 01923 225404 **f:** 01923 237404

e: reception@i2analytical.com

e: philip@Imbgeosolutions.com

Analytical Report Number: 21-75177

Project / Site name: Manor Road Samples received on: 23/04/2021

Your job number: LMB- MANOR ROAD Samples instructed on/ 23/04/2021

Analysis started on:

Your order number: Analysis completed by: 25/05/2021

Report Issue Number: 1 **Report issued on:** 25/05/2021

Samples Analysed: 19 soil samples

Signed: Patricipal Control

Agnieszka Czerwińska Technical Reviewer (Reporting Team) For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41 -711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are : soils - 4 weeks from

soils - 4 weeks from reporting leachates - 2 weeks from reporting waters - 2 weeks from reporting asbestos - 6 months from reporting

Excel copies of reports are only valid when accompanied by this PDF certificate.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement. Application of uncertainty of measurement would provide a range within which the true result lies.

An estimate of measurement uncertainty can be provided on request.





			1872362	1872363	1872364	1872365	1872366	
Sample Reference				BH101	BH101	BH102	BH102	BH103
Sample Number				None Supplied	None Supplied	None Supplied	None Supplied	None Supplied
Depth (m)				2.00	2.75	1.20	7.00	0.80
Date Sampled				Deviating	Deviating	21/04/2021	27/04/2021	21/04/2021
Time Taken				None Supplied	None Supplied	None Supplied	None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	10	7.1	8.8	17	6.2
Total mass of sample received	kg	0.001	NONE	0.40	1.0	0.90	0.90	0.50
General Inorganics	pH Units	N/A	MCERTS	7.8	7.8	9.3	8.3	8.5
pH - Automated	pH Units	N/A 0.005	MCERTS MCERTS	7.8 0.380	7.8 0.121	9.3 0.129	8.3 0.103	8.5 0.047
pH - Automated Total Sulphate as SO4 water Soluble 50/4 Tohr extraction (2:1 Leachate Equivalent)				7.8 0.380 1.8	7.8 0.121 0.46	9.3 0.129 0.22	8.3 0.103 0.43	8.5 0.047 0.031
ph - Automated Total Sulphate as SO4 water Soludie SO4 15hr extraction (2:1 Leachate	%	0.005 0.00125 1.25	MCERTS MCERTS	0.380	0.121	0.129	0.103	0.047
pH - Automated Total Sulphate as SO4 water Soluble SU4 Tohr extraction (2:1 Leachate Equivalent) water Soluble SU4 Tohr extraction (2:1 Leachate	% g/l	0.005	MCERTS MCERTS	0.380	0.121 0.46	0.129	0.103 0.43	0.047 0.031
pH - Automated Total Sulphate as SO4 water Soluble SO4 Tohir extraction (2:1 Leachate Equivalent) water Soluble SO4 Tohir extraction (2:1 Leachate Equivalent)	g/l mg/l	0.005 0.00125 1.25	MCERTS MCERTS MCERTS MCERTS MCERTS	0.380 1.8 1840	0.121 0.46 463	0.129 0.22 223	0.103 0.43 426	0.047 0.031 30.5
pH - Automated Total Sulphate as SO4 water Soluble SO4 Ibnr extraction (2:1 Leacnate Equivalent) water Soluble SO4 Ibnr extraction (2:1 Leacnate Equivalent) Water Soluble Chloride (2:1) (leachate equivalent)	g/l mg/l mg/l	0.005 0.00125 1.25 0.5	MCERTS MCERTS MCERTS MCERTS	0.380 1.8 1840 5.9	0.121 0.46 463 5.6	0.129 0.22 223 5.2	0.103 0.43 426 13	0.047 0.031 30.5 6.3
pH - Automated Total Sulphate as SO4 water Soluble SO4 Ibnr extraction (2:1 Leacnate Equivalent) water Soluble SO4 Ibnr extraction (2:1 Leacnate Equivalent) Water Soluble Chloride (2:1) (leachate equivalent) Total Sulphur	g/l mg/l mg/l	0.005 0.00125 1.25 0.5 0.005	MCERTS MCERTS MCERTS MCERTS MCERTS	0.380 1.8 1840 5.9 0.140	0.121 0.46 463 5.6 0.041	0.129 0.22 223 5.2 0.057	0.103 0.43 426 13 0.453	0.047 0.031 30.5 6.3 0.021
pH - Automated Total Sulphate as SO4 water Solicies SU4 Tohr extraction (2:1 Leachate Equivalent) water Solicies SU4 Tohr extraction (2:1 Leachate Equivalent) Water Solicies Coloride (2:1) (leachate equivalent) Total Sulphur Water Solicies Nitrate (2:1) as N (leachate equivalent)	g/l mg/l mg/l	0.005 0.00125 1.25 0.5 0.005	MCERTS MCERTS MCERTS MCERTS MCERTS	0.380 1.8 1840 5.9 0.140	0.121 0.46 463 5.6 0.041	0.129 0.22 223 5.2 0.057	0.103 0.43 426 13 0.453	0.047 0.031 30.5 6.3 0.021

U/S = Unsuitable Sample I/S = Insufficient Sample





Lab Sample Number		•		1872367	1872368	1872369	1872370	1872371
Sample Reference				BH103	BH103	BH104	BH104	BH104
Sample Number				None Supplied				
Depth (m)				2.75	6.00	10.50	13.50	18.00
Date Sampled				21/04/2021	21/04/2021	22/04/2021	22/04/2021	22/04/2021
ime Taken				None Supplied				
Accreditation Status Analytical Parameter (Soil Analysis)								
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	6.2	15	16	14	15
Total mass of sample received	kg	0.001	NONE	1.0	1.0	1.0	0.90	0.70
General Inorganics						_		
pH - Automated	pH Units	N/A	MCERTS	8.3	8.8	8.9	8.6	8.3
Total Sulphate as SO4	%	0.005	MCERTS	0.032	0.097	0.077	0.108	0.146
Water Soluble SO4 16hr extraction (2:1 Leachate	o/l	0.00125	MCERTS	0.051	0.22	0.37	0.49	0.25

pH - Automated	pH Units	N/A	MCERTS	8.3	8.8	8.9	8.6	8.3
Total Sulphate as SO4	%	0.005	MCERTS	0.032	0.097	0.077	0.108	0.146
water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.051	0.22	0.37	0.49	0.25
water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	mg/l	1.25	MCERTS	51.2	222	366	491	255
Water Soluble Chloride (2:1) (leachate equivalent)	mg/l	0.5	MCERTS	10	21	12	32	37
Total Sulphur	%	0.005	MCERTS	0.018	0.366	0.377	0.699	0.742
Water Soluble Nitrate (2:1) as N (leachate equivalent)	mg/l	2	NONE	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0

Heavy Metals / Metalloids

Magnesium (water soluble)	mg/kg	5	NONE	8.0	27	45	60	30
Magnesium (leachate equivalent)	mg/l	2.5	NONE	4.0	13	22	30	15

U/S = Unsuitable Sample I/S = Insufficient Sample





Lab Sample Number				1872372	1872373	1872374	1872375	1872376
Sample Reference				BH104	BH105	BH105	BH105	BH105
Sample Number				None Supplied				
Depth (m)				19.50	4.75	7.50	12.00	14.00
Date Sampled				22/04/2021	23/04/2021	26/04/2021	26/04/2021	26/04/2021
Time Taken				None Supplied				
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status					
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	9.1	17	18	19	11
Total mass of sample received	kg	0.001	NONE	0.40	0.70	0.90	1.0	0.30
General Inorganics pH - Automated	pH Units	N/A	MCERTS	8.6	8.0	8.4	8.4	8.5
Total Sulphate as SO4	%	0.005	MCERTS	0.106	0.047	0.090	0.089	0.109
water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.63	0.019	0.15	0.39	0.55
water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	mg/l	1.25	MCERTS	628	19.0	148	395	554
Water Soluble Chloride (2:1) (leachate equivalent)	mg/l	0.5	MCERTS	23	5.4	4.4	4.2	46
Total Sulphur	%	0.005	MCERTS	0.701	0.012	0.362	0.896	0.453
Water Soluble Nitrate (2:1) as N (leachate equivalent)	mg/l	2	NONE	< 2.0	< 2.0	< 2.0	< 2.0	< 2.0

NONE

NONE

32

13

20

53

32

U/S = Unsuitable Sample I/S = Insufficient Sample

mg/kg mg/l

Heavy Metals / Metalloids

Magnesium (leachate equivalent)

Magnesium (water soluble)





Lab Sample Number				1872377	1872378	1872379	1875497
Sample Reference				BH105	BH105	TP102	BH104
Sample Number	None Supplied	None Supplied	None Supplied	None Supplied			
Depth (m)	17.00	23.00	2.00	22.50			
Date Sampled	26/04/2021	26/04/2021	27/04/2021	22/04/2021			
Time Taken				None Supplied	None Supplied	None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Limit of detection	Accreditation Status				
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1	< 0.1
Moisture Content	NONE	12	12	4.7	16		
Total mass of sample received	kg	0.001	NONE	0.30	0.50	0.40	1.0

General Inorganics

Contract Interigrants							
pH - Automated	pH Units	N/A	MCERTS	8.5	8.5	8.8	8.8
Total Sulphate as SO4	%	0.005	MCERTS	0.142	0.136	0.032	0.125
water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	g/l	0.00125	MCERTS	0.67	0.78	0.12	0.60
water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	mg/l	1.25	MCERTS	671	777	117	596
Water Soluble Chloride (2:1) (leachate equivalent)	mg/l	0.5	MCERTS	51	70	4.6	50
Total Sulphur	%	0.005	MCERTS	0.405	0.402	0.018	0.330
Water Soluble Nitrate (2:1) as N (leachate equivalent)	mg/l	2	NONE	< 2.0	< 2.0	< 2.0	< 2.0

Heavy Metals / Metalloids

Magnesium (water soluble)	mg/kg	5	NONE	73	78	12	51
Magnesium (leachate equivalent)	mg/l	2.5	NONE	37	39	5.9	25

U/S = Unsuitable Sample I/S = Insufficient Sample





* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
1872362	BH101	None Supplied	2	Brown loam and clay with gravel.
1872363	BH101	None Supplied	2.75	Brown loam and clay with gravel.
1872364	BH102	None Supplied	1.2	Brown loam and clay with gravel.
1872365	BH102	None Supplied	7	Brown clay.
1872366	BH103	None Supplied	0.8	Brown loam and sand with gravel.
1872367	BH103	None Supplied	2.75	Brown loam and sand with gravel.
1872368	BH103	None Supplied	6	Brown clay.
1872369	BH104	None Supplied	10.5	Brown clay.
1872370	BH104	None Supplied	13.5	Brown clay.
1872371	BH104	None Supplied	18	Brown clay.
1872372	BH104	None Supplied	19.5	Brown clay.
1872373	BH105	None Supplied	4.75	Brown clay and sand with gravel.
1872374	BH105	None Supplied	7.5	Brown clay.
1872375	BH105	None Supplied	12	Brown clay.
1872376	BH105	None Supplied	14	Brown clay.
1872377	BH105	None Supplied	17	Brown clay.
1872378	BH105	None Supplied	23	Brown clay.
1872379	TP102	None Supplied	2	Brown loam and sand with gravel.
1875497	BH104	None Supplied	22.5	Brown clay.





Water matrix abbreviations: Surface Water (SW) Potable Water (PW) Ground Water (GW)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Magnesium, water soluble, in soil	Determination of water soluble magnesium by extraction with water followed by ICP-OES.	In-house method based on TRL 447	L038-PL	D	NONE
Moisture Content	Moisture content, determined gravimetrically. (30 oC)	In house method.	L019-UK/PL	w	NONE
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement.	In house method.	L099-PL	D	MCERTS
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight.	In-house method based on British Standard Methods and MCERTS requirements.	L019-UK/PL	D	NONE
Total Sulphate in soil as %	Determination of total sulphate in soil by extraction with 10% HCl followed by ICP-OES.	In house method.	L038-PL	D	MCERTS
Total Sulphur in soil as %	Determination of total sulphur in soil by extraction with aqua-regia, potassium bromide/bromate followed by ICP-OES.	In house method.	L038-PL	D	MCERTS
Sulphate, water soluble, in soil (16hr extraction)	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In house method.	L038-PL	D	MCERTS
Water Soluble Nitrate (2:1) as N in soil	Determination of nitrate by reaction with sodium salicylate and colorimetry.	In-house method based on Examination of Water and Wastewatern & Polish Standard Method PN- 82/C-04579.08, 2:1 extraction.	L078-PL	W	NONE
Chloride, water soluble, in soil	Determination of Chloride colorimetrically by discrete analyser.	In house method.	L082-PL	D	MCERTS
Sulphate, water soluble, in soil	Determination of water soluble sulphate by ICP-OES. Results reported directly (leachate equivalent) and corrected for extraction ratio (soil equivalent).	In house method.	L038-PL	D	MCERTS

For method numbers ending in 'UK' analysis have been carried out in our laboratory in the United Kingdom.

For method numbers ending in 'PL' analysis have been carried out in our laboratory in Poland.

Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.

Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.

Sample Deviation Report



Sample ID	()ther II)		Lab Sample Number	Sample Deviation	Test Name	Test Ref	Test Deviation
BH101	None Supplied	S	1872362	a	None Supplied	None Supplied	None Supplied
BH101	None Supplied	S	1872363	a	None Supplied	None Supplied	None Supplied

APPENDICES

APPENDIX C GEOTECHNICAL LABORATORY RESULTS



Liquid and Plastic Limits

12 Analytical Ltd Unit 5 Harrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Date Sampled: Not Given Date Received: 26/04/2021 Date Tested: 25/05/2021

Sampled By: Client - PIL, DN & HB

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Test Results:

Laboratory Reference: 1872319
Hole No.: BH101
Sample Reference: Not Given

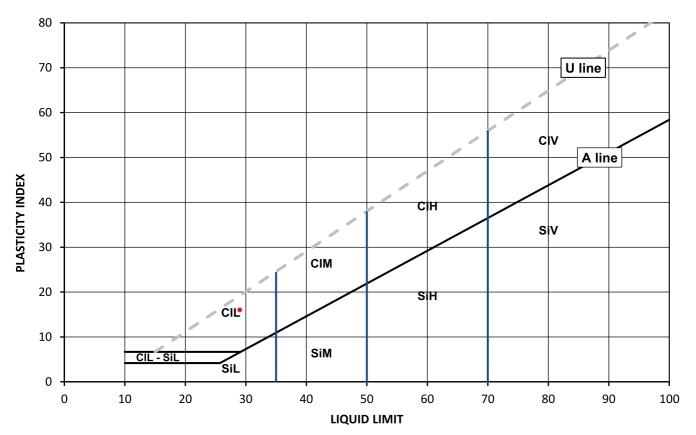
Soil Description: Brown gravelly very sandy CLAY

Sample Preparation: Tested after washing to remove >425um

Depth Top [m]: 2.00
Depth Base [m]: Not Given

Sample Type: D

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425µm
Content [W] %	[WL] %	[Wp]%	[lp]%	BS Test Sieve
9.4	29	13	16	51



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

O Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Opinions and interpretations expressed herein are outside of the scope of the UKAS Accreditation. This

report may not be reproduced other than in full without the prior written approval of the issuing

laboratory. The results included within the report relate only to the sample(s) submitted for testing.

Remarks:

Signed:

Monika Janoszek PL Deputy Head of Geotechnical Section

for and on behalf of i2 Analytical Ltd

Date Reported: 02/06/2021

Page 1 of 1

GF 232.10



Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Job Number: 21-75176 Date Sampled: Not Given Date Received: 26/04/2021 Date Tested: 25/05/2021

Client Reference: LMB-MANOR ROAD

Sampled By: Client - PIL, DN & HB

Test Results:

Sample Preparation:

Laboratory Reference: 1872321 BH101 Hole No.: Not Given

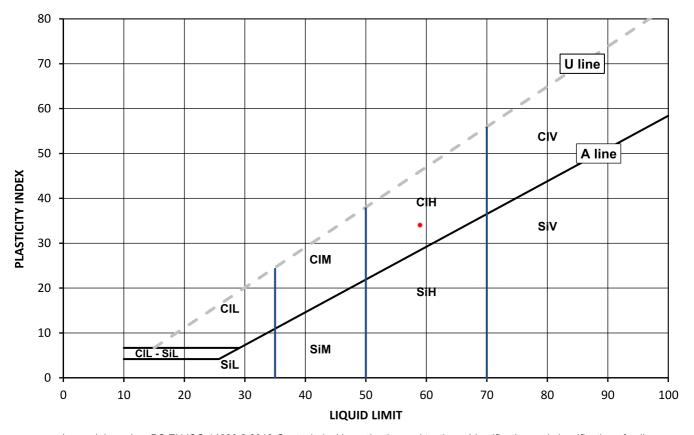
Sample Reference: Soil Description: Brown slightly gravelly slightly sandy CLAY

Tested after >425um removed by hand

Depth Top [m]: 7.50 Depth Base [m]: Not Given

Sample Type: D

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425μm
Content [W] %	[WL] %	[Wp]%	[lp]%	BS Test Sieve
31	59	25	34	95



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed: Months

Intille

Monika Janoszek PL Deputy Head of Geotechnical Section

for and on behalf of i2 Analytical Ltd



Liquid and Plastic Limits

12 Analytical Ltd Unit 5 Herrowden Road Brackmille Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: LMB Geosolutions Ltd

Client Address: 28 Dre

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis
Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176
Date Sampled: 27/04/2021
Date Received: 26/04/2021
Date Tested: 25/05/2021

Depth Top [m]: 6.00

Sample Type: D

Depth Base [m]: Not Given

Sampled By: Client - PIL, DN & HB

Test Results:

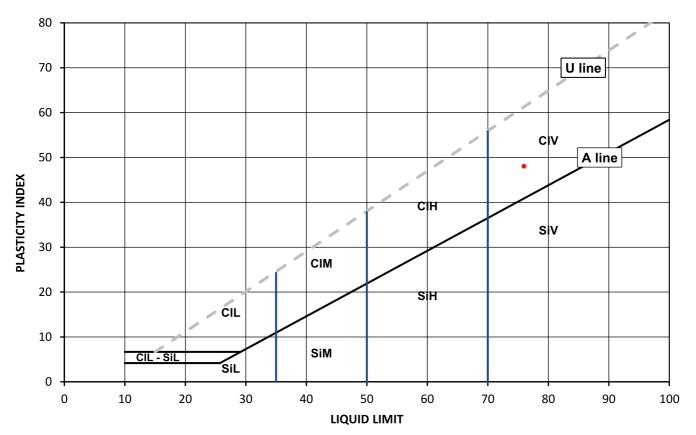
Laboratory Reference: 1872323
Hole No.: BH102
Sample Reference: Not Given

Soil Description: Dark brown CLAY

Sample Preparation: Tested in natural condition

campic Type: -				

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425µm
Content [W] %	[WL] %	[Wp]%	[lp] %	BS Test Sieve
29	76	28	48	100



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

O Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed:

Intille

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd



Liquid and Plastic Limits

IZ Analytical Ltd Unit 5 Harrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Manor Road

2 Analytical Limited, yl. Pionierow 30, 41,711 Puda Slaska, Poland

Test Results:

Laboratory Reference: 1872326 Hole No.: BH103 Sample Reference: Not Given

Soil Description: Dark brown gravelly CLAY

Sample Preparation: Tested after washing to remove >425um

Sampled By: Client - PIL, DN & HB

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Date Sampled: 21/04/2021

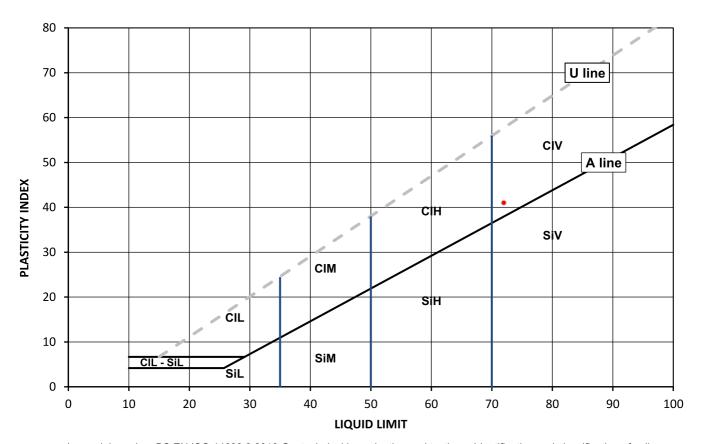
Date Received: 26/04/2021

Date Tested: 25/05/2021

Depth Base [m]: Not Given Sample Type: D

Depth Top [m]: 5.00

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425µm
Content [W] %	[WL] %	[Wp]%	[lp]%	BS Test Sieve
26	72	31	41	53



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing – Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

O Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section

Page 1 of 1

Intille

for and on behalf of i2 Analytical Ltd

GF 232.10

Date Reported: 02/06/2021

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Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Date Sampled: 21/04/2021 Date Received: 26/04/2021 Date Tested: 25/05/2021

Sampled By: Client - PIL, DN & HB

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Test Results:

Laboratory Reference: 1872327 BH103 Hole No.: Sample Reference: Not Given

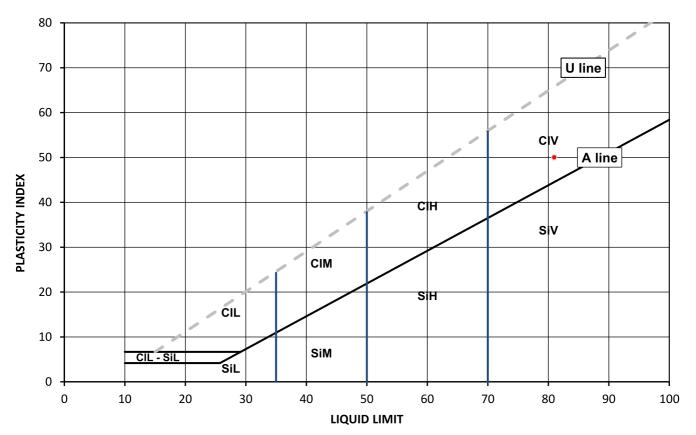
Soil Description: Dark brown CLAY

Sample Preparation: Tested in natural condition

Depth Top [m]: 9.00 Depth Base [m]: Not Given

Sample Type: D

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425µm
Content [W] %	[WL] %	[Wp]%	[lp] %	BS Test Sieve
29	81	31	50	100



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed: Months

Intille

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd



Liquid and Plastic Limits

12 Analytical Ltd Unit 5 Harrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: LMB Geosolutions Ltd

Client Address: 28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Job Number: 21-75176
Date Sampled: 21/04/2021
Date Received: 26/04/2021
Date Tested: 25/05/2021

Depth Top [m]: 11.00

Sample Type: D

Depth Base [m]: Not Given

Client Reference: LMB-MANOR ROAD

Sampled By: Client - PIL, DN & HB

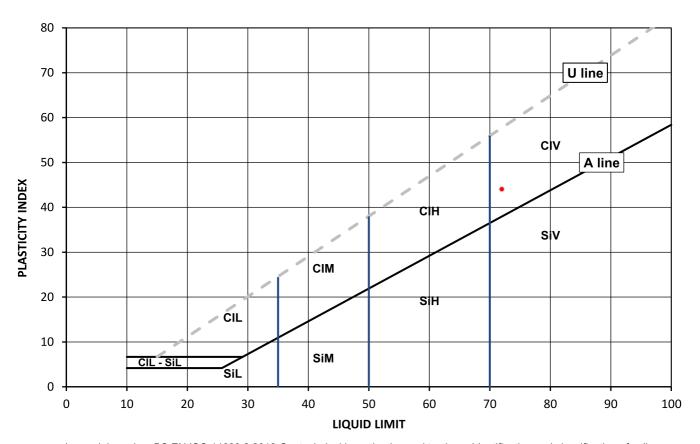
Test Results:

Laboratory Reference: 1872328
Hole No.: BH103
Sample Reference: Not Given

Soil Description: Dark brown slightly gravelly CLAY

Sample Preparation: Tested after >425um removed by hand

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425μm
Content [W] %	[WL] %	[Wp]%	[lp]%	BS Test Sieve
25	72	28	44	94



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing – Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

O Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

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report may not be reproduced other than in full without the prior written approval of the issuing

Remarks:

Signed:

Monika Janoszek PL Deputy Head of Geotechnical Section

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

GF 232.10



Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 22/04/2021 Date Received: 26/04/2021 Date Tested: 25/05/2021

Depth Top [m]: 4.35

Sample Type: D

Depth Base [m]: Not Given

Sampled By: Client - PIL, DN & HB

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

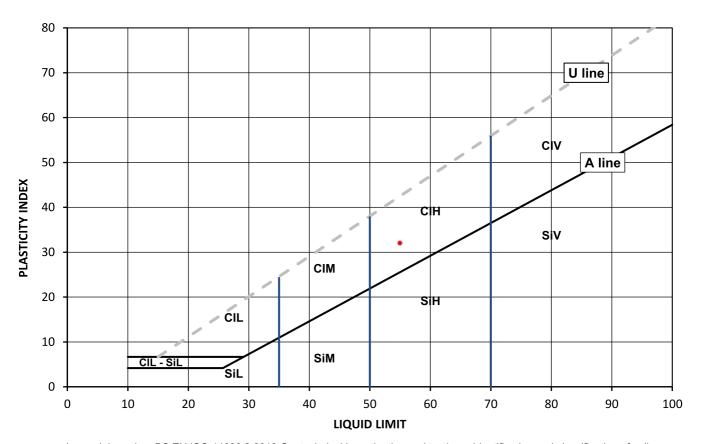
Test Results:

Laboratory Reference: 1872329 BH104 Hole No.: Sample Reference: Not Given

Soil Description: Yellowish brown very gravelly slightly sandy CLAY

Sample Preparation: Tested after washing to remove >425um

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425μm		
Content [W] %	[WL] %	[Wp]%	[lp]%	BS Test Sieve		
18	55	23	32	30		



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

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Remarks:

Signed: Months

Monika Janoszek PL Deputy Head of Geotechnical Section

Date Reported: 02/06/2021

for and on behalf of i2 Analytical Ltd

Page 1 of 1

Intille

GF 232.10



Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test Results:

Laboratory Reference: 1872332 BH104 Hole No.: Sample Reference: Not Given Soil Description: **Brown CLAY**

Sample Preparation: Tested in natural condition Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 22/04/2021 Date Received: 26/04/2021 Date Tested: 25/05/2021

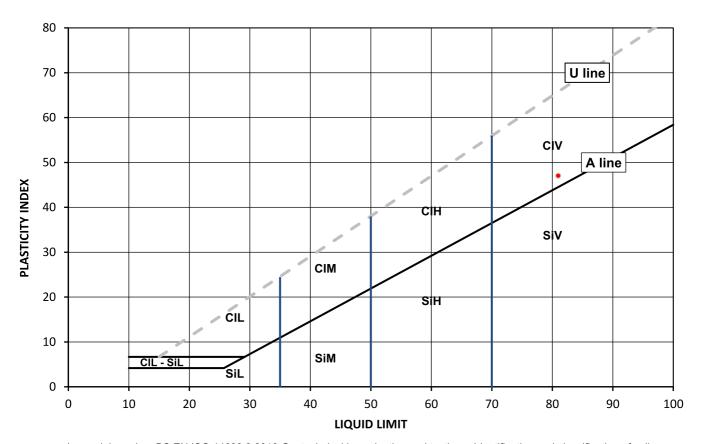
Depth Top [m]: 9.50

Sample Type: D

Depth Base [m]: Not Given

Sampled By: Client - PIL, DN & HB

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425µm
Content [W] %	[WL] %	[Wp]%	[lp]%	BS Test Sieve
28	81	34	47	



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

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Remarks:

Signed: Months

Intille

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd



Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 22/04/2021 Date Received: 26/04/2021 Date Tested: 25/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872333 BH104 Hole No.: Sample Reference: Not Given Soil Description: **Brown CLAY**

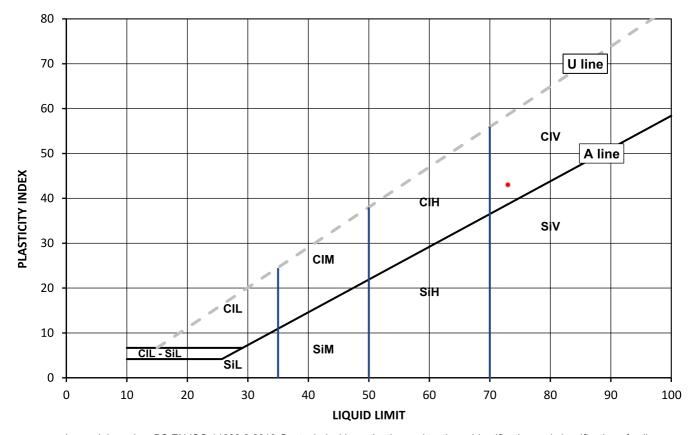
Sample Preparation:

Tested in natural condition

Depth Top [m]: 12.00 Depth Base [m]: Not Given

Sample Type: D

As Received Moisture			Plasticity Index	% Passing 425μm		
Content [W] %			[lp]%	BS Test Sieve		
29	73	30	43	100		



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed: Months

Intille

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

Page 1 of 1



Liquid and Plastic Limits

IZ Analytical Ltd Unit 5 Flarrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: LMB Geosolutions Ltd

Client Address: 28 Dresden

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176
Date Sampled: 22/04/2021
Date Received: 26/04/2021
Date Tested: 25/05/2021

Sampled By: Client - PIL, DN & HB

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test Results:

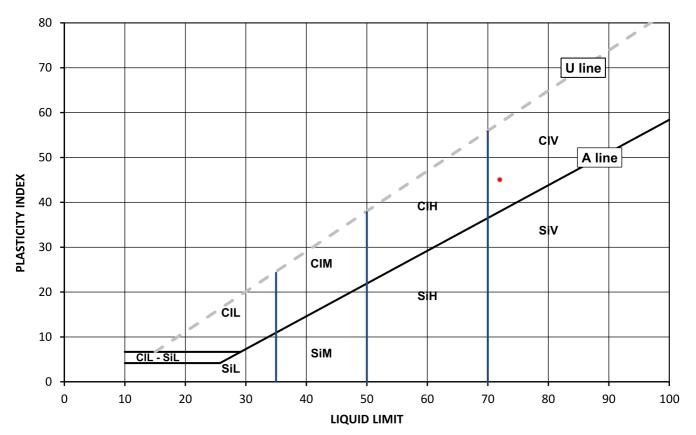
Laboratory Reference: 1872334
Hole No.: BH104
Sample Reference: Not Given
Soil Description: Brown CLAY

Sample Preparation: Tested in natural condition

Depth Top [m]: 15.50 Depth Base [m]: Not Given

Sample Type: D

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425μm		
Content [W] %	[WL]%	[Wp]%	[lp] %	BS Test Sieve		
24	72	27	45	100		



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

O Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

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Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section

for and on behalf of i2 Analytical Ltd



Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Job Number: 21-75176 Date Sampled: 22/04/2021

Client Reference: LMB-MANOR ROAD

Date Received: 26/04/2021 Date Tested: 25/05/2021

Depth Top [m]: 21.00

Sample Type: D

Depth Base [m]: Not Given

Sampled By: Client - PIL, DN & HB

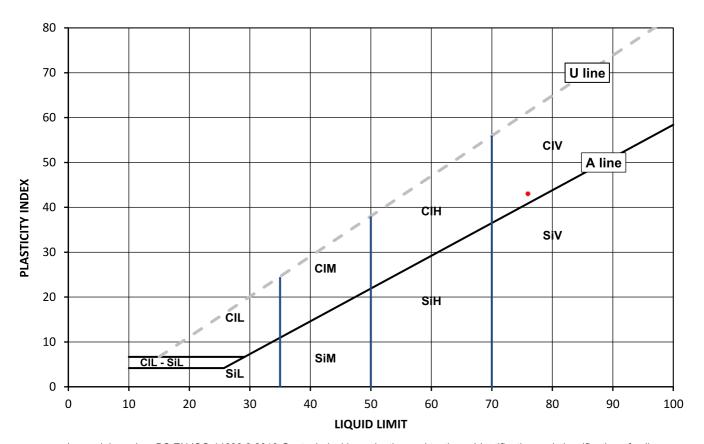
Test Results:

Laboratory Reference: 1872336 BH104 Hole No.: Sample Reference: Not Given

Soil Description: Dark brown CLAY

Sample Preparation: Tested in natural condition

As Received Moisture	Liquid Limit	Plastic Limit	Plasticity Index	% Passing 425µm
Content [W] %	[WL]%	[Wp]%	[lp]%	BS Test Sieve
26	76	33	13	100



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed: Months

Monika Janoszek PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

Intille **Date Reported: 02/06/2021**

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GF 232.10



Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Plastic Limit

[Wp]%

29

LMB Geosolutions Ltd Client:

Client Address: 28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Liquid Limit

[WL]%

71

Test Results:

Laboratory Reference: 1872343 BH105 Hole No.: Sample Reference: Not Given

As Received Moisture

Content [W]%

32

Soil Description: Dark brown CLAY

Sample Preparation: Tested in natural condition

Sampled By: Client - PIL, DN & HB

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Date Sampled: 26/04/2021

Date Received: 26/04/2021

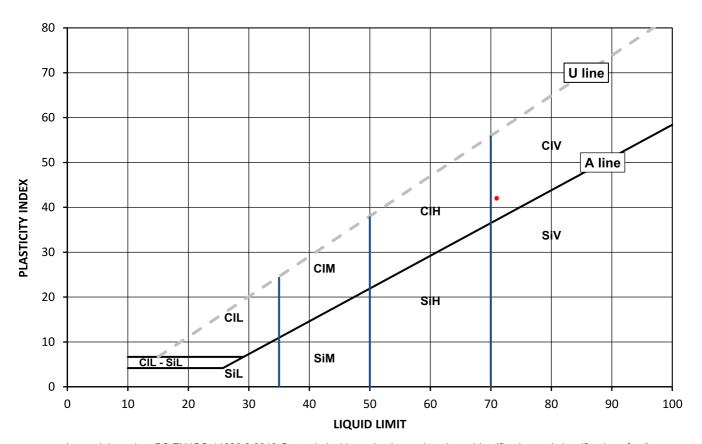
Date Tested: 25/05/2021

Depth Top [m]: 18.00 Depth Base [m]: Not Given Sample Type: D

Plastic

sticity Index	% Passing 425μm
[lp]%	BS Test Sieve

100



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

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Monika Janoszek PL Deputy Head of Geotechnical Section

for and on behalf of i2 Analytical Ltd

Page 1 of 1

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Date Reported: 02/06/2021

GF 232.10



Liquid and Plastic Limits

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Date Received: 26/04/2021

Test Results:

Laboratory Reference: 1872345 TP101 Hole No.: Sample Reference: Not Given

Soil Description: Brown gravelly sandy CLAY

Sample Preparation: Tested after washing to remove >425um

Depth Top [m]:	1.50
Depth Base [m]:	Not Given

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

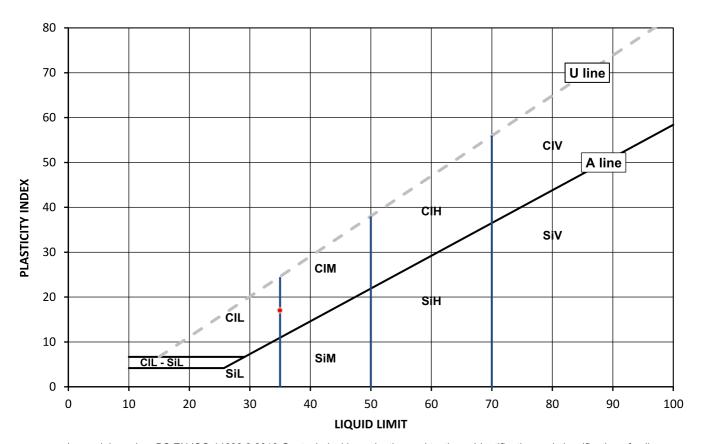
Date Sampled: 26/04/2021

Date Tested: 25/05/2021

Sampled By: Client - PIL, DN & HB

Sample Type: D

As Received Moisture	1.1		Plasticity Index	% Passing 425μm		
Content [W] %			[lp]%	BS Test Sieve		
17	35	18	17	54		



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

> 0 Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

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Remarks:

Signed: Months

Monika Janoszek PL Deputy Head of Geotechnical Section

for and on behalf of i2 Analytical Ltd



Liquid and Plastic Limits

12 Analytical Ltd Unit 5 Harrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-2: 1990: Clause 4.4 and 5

Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: Not Given Date Received: 26/04/2021 Date Tested: 26/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1875456
Hole No.: BH104
Sample Reference: Not Given

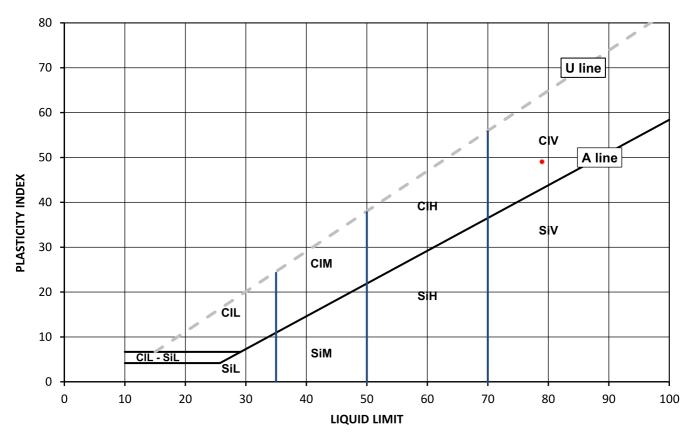
Soil Description: Greyish brown CLAY

Sample Preparation: Tested in natural condition

Depth Top [m]: 24.55
Depth Base [m]: Not Given

Sample Type: D

As Received Moisture			Plasticity Index	% Passing 425μm		
Content [W] %			[lp] %	BS Test Sieve		
26	79	30	49	100		



Legend, based on BS EN ISO 14688 2:2018 Geotechnical investigation and testing - Identification and classification of soil

Plasticity Liquid Limit CI Clay L Low below 35 Si Silt М Medium 35 to 50 Н High 50 to 70 ٧ Very high exceeding 70

O Organic append to classification for organic material (eg CIHO)

Note: Moisture Content by BS 1377-2: 1990: Clause 3.2

Remarks:

Signed:

Intille

Monika Janoszek
PL Deputy Head of Geotechnical Section

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

Page

Summary of Classification Test Results

Tested in Accordance with:

Moisture Content by BS 1377-2: 1990: Clause 3.2; Water Content by BS EN 17892-1: 2014; Atterberg by BS 1377-2: 1990: Clause 4.3 (4 Point Test), Clause 4.4 (1 Point Test) and 5; PD by BS 1377-2: 1990: Clause 8.2

28 Dresden Road, London,

Client Address:

4041

Client:

N19 3BD

Philip Lewis Manor Road

Site Address:

Contact:

LMB Geosolutions Ltd

Brackmills Industrial Estate IZ Analytical Ltd Unit 8 Harrowden Road Northampton NN4 7EB

Date Sampled: 21/04 - 27/04/2021 Job Number: 21-75176

Client Reference: LMB-MANOR ROAD

Date Received: 26/04/2021

Date Tested: 25/05 - 26/05/2021

Sampled By: Client - PIL, DN & HB

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test results

#/	lstoT fisoro¶	%										
	PD	Mg/m3										
Density	dry	Mg/m3										
	bulk	Mg/m3										
	Ġ	%				16		34		48		
berg	Wp	%				13		25		28		
Atterberg	WL	%				29		59		92		
	% Passing 425um	%				51		92		100		
tent	Water Con	%										
ntent	O Sarure Co [W]	%	7.2	19	17	9.4	8.6	31	12	29	8.3	6.2
	Remarks					Atterberg 1 Point		Atterberg 1 Point		Atterberg 1 Point		
	Description		Dark brown sandy GRAVEL	Dark brown slightly gravelly sandy CLAY	Dark brown slightly gravelly sandy CLAY	Brown gravelly very sandy CLAY	Brown very gravelly SAND	Brown slightly gravelly slightly sandy CLAY	Dark brown slightly clayey very sandy GRAVEL	Dark brown CLAY	Yellowish brown sandy GRAVEL	Yellowish brown sandy GRAVEL
	Туре		Q	Q	Q	Q	Q	Q	a	Q	Q	Q
	Depth Base	Е	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given
Sample	Depth Top	E	0.75	08.0	1.20	2.00	3.75	7.50	06:0	00.9	1.85	3.00
	Reference		Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given
	Hole No.		BH101	BH101	BH101	BH101	BH101	BH101	BH102	BH102	BH103	BH103
	Laboratory Reference		1872316	1872317	1872318	1872319	1872320	1872321	1872322	1872323	1872324	1872325

Note: # Non accredited; NP - Non plastic

Comments:

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PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd Monika Janoszek

Date Reported: 02/06/2021

Summary of Classification Test Results

Tested in Accordance with:

Moisture Content by BS 1377-2: 1990: Clause 3.2; Water Content by BS EN 17892-1: 2014; Atterberg by BS 1377-2: 1990: Clause 4.3 (4 Point Test), Clause 4.4 (1 Point Test) and 5; PD by BS 1377-2: 1990: Clause 8.2

Brackmills Industrial Estate IZ Analytical Ltd Unit 8 Harrowden Road Northampton NN4 7EB

Client Reference: LMB-MANOR ROAD

Date Sampled: 21/04 - 22/04/2021 Job Number: 21-75176

Date Tested: 25/05 - 26/05/2021 Date Received: 26/04/2021

Sampled By: Client - PIL, DN & HB

Manor Road Philip Lewis Site Address: Contact:

28 Dresden Road, London,

Client Address:

4041

Client:

N19 3BD

LMB Geosolutions Ltd

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test results

#/	Total Yorostty	%										
	PD	Mg/m3										
Density	dry	Mg/m3										
	bulk	Mg/m3										
	ф	%	41	50	44	32			47	43	45	
Atterberg	Wp	%	31	31	28	23			34	30	27	
Atter	WL	%	72	81	72	22			81	73	72	
	% Passing 425um	%	53	100	94	30			100	100	100	
tent	Water Con	%										
ntent	OD erutsioM [W]	%	26	29	25	18	29	31	28	29	24	22
	Remarks		Atterberg 1 Point	Atterberg 1 Point	Atterberg 1 Point	Atterberg 1 Point			Atterberg 1 Point	Atterberg 1 Point	Atterberg 1 Point	
	Description		Dark brown gravelly CLAY	Dark brown CLAY	Dark brown slightly gravelly CLAY	Yellowish brown very gravelly slightly sandy CLAY	Brownish grey slightly gravelly CLAY	Brown CLAY	Brown CLAY	Brown CLAY	Brown CLAY	Brown sandy CLAY
	туре		Q	Q	Q	Q	Q	Q	Q	Q	Q	Q
	Depth Base	Ε	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given
Sample	Depth Top	Ε	2.00	9.00	11.00	4.35	2.00	7.50	9.50	12.00	15.50	18.50
	Reference		Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given
	Hole No.		BH103	BH103	BH103	BH104	BH104	BH104	BH104	BH104	BH104	BH104
	Laboratory Reference		1872326	1872327	1872328	1872329	1872330	1872331	1872332	1872333	1872334	1872335

Note: # Non accredited; NP - Non plastic

Comments:

Opinions and interpretations expressed herein are outside of the scope of the UKAS Accreditation. This report may not be reproduced other than in full without the prior written approval of the issuing laboratory. The results included within the report relate only to the sample(s) submitted for testing.



Signed:

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd Monika Janoszek

GF 234.12

Date Reported: 02/06/2021

Summary of Classification Test Results

IZ Analytical Ltd Unit 8 Harrowden Road Northampton NN4 7EB

Brackmills Industrial Estate

Tested in Accordance with:

Moisture Content by BS 1377-2: 1990: Clause 3.2; Water Content by BS EN 17892-1: 2014; Atterberg by BS 1377-2: 1990: Clause 4.3 (4 Point Test), Clause 4.4 (1 Point Test) and 5; PD by BS 1377-2: 1990: Clause 8.2

28 Dresden Road, London,

Client Address:

4041

Client:

N19 3BD

Philip Lewis Manor Road

Site Address:

Contact:

LMB Geosolutions Ltd

Date Sampled: 22/04 - 26/04/2021 Date Received: 26/04/2021

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Date Tested: 25/05 - 26/05/2021

Sampled By: Client - PIL, DN & HB

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test results

#4	lstoT Yjisoro¶	%										
	PD	Mg/m3										
Density	dry	Mg/m3										
	bulk	Mg/m3										
	ď	%	43		49							42
berg	Wp	%	33		30							29
Atterberg	WL	%	9/		62							71
	% Passing 425um	%	100		100							100
tent	Water Con	%										
ntent	oO erutsioM [W]	%	26	27	26	7.0	5.4	9.9	6.1	31	29	32
	Remarks		Atterberg 1 Point		Atterberg 1 Point							Atterberg 1 Point
	Description		Dark brown CLAY	Greyish brown CLAY	Greyish brown CLAY	Brown sandy GRAVEL	Yellowish brown slightly clayey very sandy GRAVEL	Brown sandy GRAVEL	Brown sandy GRAVEL	Dark brown CLAY	Dark brown CLAY	Dark brown CLAY
	Туре		D	D	D	Q	D	D	D	D	D	O
	Depth Base	٤	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given
Sample	Depth Top	٤	21.00	24.00	24.55	0.75	1.20	2.00	3.75	00.9	9.00	18.00
	Reference		Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given	Not Given
	Hole No.		BH104	BH104	BH104	BH105	BH105	BH105	BH105	BH105	BH105	BH105
	Laboratory Reference		1872336	1875455	1875456	1872337	1872338	1872339	1872340	1872341	1872342	1872343

Note: # Non accredited; NP - Non plastic

Comments:

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Signed:

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd Monika Janoszek

Date Reported: 02/06/2021

GF 234.12

Summary of Classification Test Results

Tested in Accordance with:

Moisture Content by BS 1377-2: 1990: Clause 3.2; Water Content by BS EN 17892-1: 2014; Atterberg by BS 1377-2: 1990: Clause 4.3 (4 Point Test), Clause 4.4 (1 Point Test) and 5; PD by BS 1377-2: 1990: Clause 8.2

28 Dresden Road, London,

Client Address:

4041

Client:

N19 3BD

LMB Geosolutions Ltd

Brackmills Industrial Estate IZ Analytical Ltd Unit 8 Harrowden Road Northampton NN4 7EB Client Reference: LMB-MANOR ROAD

Sampled By: Client - PIL, DN & HB

Date Sampled: 26/04 - 27/04/2021 Date Tested: 25/05 - 26/05/2021 Date Received: 26/04/2021 Job Number: 21-75176

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Manor Road Philip Lewis

Site Address:

Contact:

Test results

#	letoT (tisoroq	%							
	PD	Mg/m3							
Density	dry	Mg/m3							
	bulk	Mg/m3							
	ď	%		17					
berg	Wp	%		18					
Atterberg	WL	%		35					
	% Passing 425um	%		54					
tent	Water Con [W]	%							
ntent	oO sture Co [W]	%	25	17	18				
	Remarks			Atterberg 1 Point					
	Description		Dark brown CLAY	Brown gravelly sandy CLAY	Brown sandy CLAY				
	Туре		D	D	D				
	Depth Base	Ε	Not Given	Not Given	Not Given				
Sample	Depth Top	Е	20.00	1.50	2.00				
	Reference		Not Given	Not Given	Not Given				
	Hole No.		BH105	TP101	TP102				
	Laboratory Reference		1872344	1872345	1872346				

Note: # Non accredited; NP - Non plastic

Comments:

Opinions and interpretations expressed herein are outside of the scope of the UKAS Accreditation. This report may not be reproduced other than in full without the prior written approval of the issuing laboratory. The results included within the report relate only to the sample(s) submitted for testing.

Sports Prints

Page 1 of 1

Signed:

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd Monika Janoszek

Date Reported: 02/06/2021

GF 234.12



Particle Size Distribution

Tested in Accordance with: BS 1377-2: 1990

12 Analytical Ltd Unit 5 Harrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Manor Road

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: Not Given Date Received: 26/04/2021 Date Tested: 26/05/2021

Sampled By: Client - PIL, DN & HB

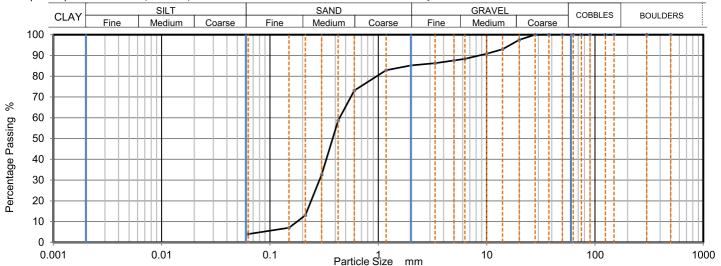
Test Results:

Laboratory Reference:1872307Depth Top [m]: 6.00Hole No.:BH101Depth Base [m]: Not GivenSample Reference:Not GivenSample Type: B

Sample Description: Brown slightly clayey slightly silty gravelly SAND

Sample Preparation: Sample was quartered, oven dried at 106 °C and broken down by hand.

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland



Siev	ring	Sedime	ntation
Particle Size mm	% Passing	Particle Size mm	% Passing
500	100		
300	100		
150	100		
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	98		
14	93		
10	91		
6.3	88		
5	88		
3.35	86		
2	85		
1.18	83		
0.6	73		
0.425	59		
0.3	33		
0.212	13		
0.15	7		
0.063	5	1	

Sample Proportions	% dry mass		
Very coarse	0		
Gravel	15		
Sand	81		
Fines <0.063mm	5		

Grading Analysis	}	
D100	mm	28
D60	mm	0.44
D30	mm	0.287
D10	mm	0.179
Uniformity Coefficient		2.5
Curvature Coefficient		1

Uniformity Coefficient and Coefficient of Curvature calculated in accordance with BS EN ISO 14688-2: 2004 + A1: 2013

Note: Tested in Accordance with BS1377:Part 2:1990, clause 9.2

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section

testing.

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd



Particle Size Distribution

Tested in Accordance with: BS 1377-2: 1990

12 Analytical Ltd Unit 8 Harrowden Road Bracionills Industrial Estate Northampton NN4 7EB



LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 27/04/2021 Date Received: 26/04/2021 Date Tested: 26/05/2021

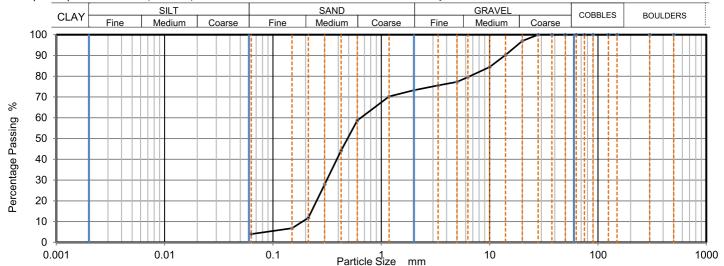
Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872309 Depth Top [m]: 4.00 Depth Base [m]: Not Given BH102 Hole No .: Sample Reference: Not Given Sample Type: B

Brown slightly clayey slightly silty gravelly SAND Sample Description:

Sample was quartered, oven dried at 106.0 °C and broken down by hand. Sample Preparation:



Siev	ring	Sedime	ntation
Particle Size mm	% Passing	Particle Size mm	% Passing
500	100		
300	100		
150	100		
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	97		
14	90		
10	84		
6.3	80		
5	77		
3.35	76		
2	73		
1.18	70		
0.6	59		
0.425	44		
0.3	28		
0.212	12		
0.15	7		
0.063	4	7	

Sample Proportions	% dry mass		
Very coarse	0		
Gravel	27		
Sand	69		
Fines <0.063mm	4		

Grading Analysis		
D100	mm	28
D60	mm	0.647
D30	mm	0.314
D10	mm	0.187
Uniformity Coefficient		3.5
Curvature Coefficient		0.81

Uniformity Coefficient and Coefficient of Curvature calculated in accordance with BS EN ISO 14688-2: 2004 + A1: 2013

Note: Tested in Accordance with BS1377:Part 2:1990, clause 9.2

Remarks:

Signed:

Monika Janoszek PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

Opinions and interpretations expressed herein are outside of the scope of the UKAS Accreditation. This Months

Intille Page 1 of 1 **Date Reported: 02/06/2021**



Particle Size Distribution

Tested in Accordance with: BS 1377-2: 1990

12 Analytical Ltd Unit 8 Harrowden Road Bracionills Industrial Estate Northampton NN4 7EB



LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Job Number: 21-75176 Date Sampled: 27/04/2021 Date Received: 26/04/2021 Date Tested: 26/05/2021

Client Reference: LMB-MANOR ROAD

Sampled By: Client - PIL, DN & HB

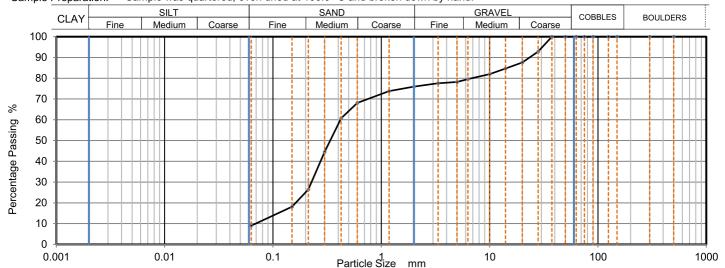
Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test Results:

Laboratory Reference: 1872312 Depth Top [m]: 2.30 Depth Base [m]: Not Given TP102 Hole No .: Sample Reference: Not Given Sample Type: B

Brown slightly clayey slightly silty gravelly SAND Sample Description:

Sample was quartered, oven dried at 106.0 °C and broken down by hand. Sample Preparation:



Siev	ing	Sedimer	ntation
Particle Size mm	% Passing	Particle Size mm	% Passing
500	100		
300	100		
150	100		
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	93		
20	88		
14	85		
10	82		
6.3	80		
5	78		
3.35	78		
2	76	1	
1.18	74		
0.6	68	1	
0.425	61	7	
0.3	45	7	
0.212	26		
0.15	18	7	
0.063	10	1	

Sample Proportions	% dry mass		
Very coarse	0		
Gravel	24		
Sand	66		
Fines <0.063mm	10		

Grading Analysis		
D100	mm	37.5
D60	mm	0.418
D30	mm	0.227
D10	mm	0.064
Uniformity Coefficient		6.5
Curvature Coefficient		1.9

Uniformity Coefficient and Coefficient of Curvature calculated in accordance with BS EN ISO 14688-2: 2004 + A1: 2013

Note: Tested in Accordance with BS1377:Part 2:1990, clause 9.2

Remarks:

Signed:

Monika Janoszek PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

Months Intille

Page 1 of 1

Date Reported: 02/06/2021



Particle Size Distribution

Tested in Accordance with: BS 1377-2: 1990

IZ Analytical Ltd Unit 5 Harrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Client Reference: LMB-MANOR ROAD Job Number: 21-75176

Date Sampled: 26/04/2021 Date Received: 26/04/2021 Date Tested: 26/05/2021

Sampled By: Client - PIL, DN & HB

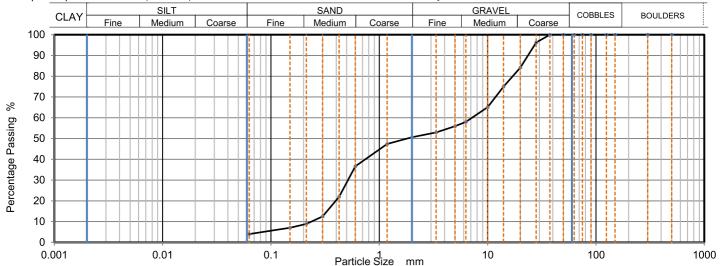
Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test Results:

Laboratory Reference:1872314Depth Top [m]: 2.00Hole No.:TP103Depth Base [m]: Not GivenSample Reference:Not GivenSample Type: B

Sample Description: Brown slightly clayey slightly silty very sandy GRAVEL

Sample Preparation: Sample was quartered, oven dried at 106.0 °C and broken down by hand.



Siev	ring	Sedime	ntation
Particle Size mm	% Passing	Particle Size mm	% Passing
500	100		
300	100		
150	100		
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	96		
20	84		
14	75		
10	65		
6.3	58		
5	56		
3.35	53		
2	51		
1.18	47		
0.6	37		
0.425	22		
0.3	13		
0.212	9		
0.15	7	7	
0.063	5	7	

Sample Proportions % dry mass	
Very coarse	0
Gravel	49
Sand	46
Fines <0.063mm	5

Grading Analysis		
D100	mm	37.5
D60	mm	7.19
D30	mm	0.513
D10	mm	0.236
Uniformity Coefficient		31
Curvature Coefficient		0.16

Uniformity Coefficient and Coefficient of Curvature calculated in accordance with BS EN ISO 14688-2: 2004 + A1: 2013

Note: Tested in Accordance with BS1377:Part 2:1990, clause 9.2

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

testing.

tor and on behalf of i2 Anal



Drv Density / Moisture Content

Relationship Light Compaction

Tested in Accordance with: BS 1377-4: 1990

12 Analytical Ltd Unit 5 Fierrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis
Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Date Sampled: 26/04/2021

Date Received: 26/04/2021

Date Tested: 31/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

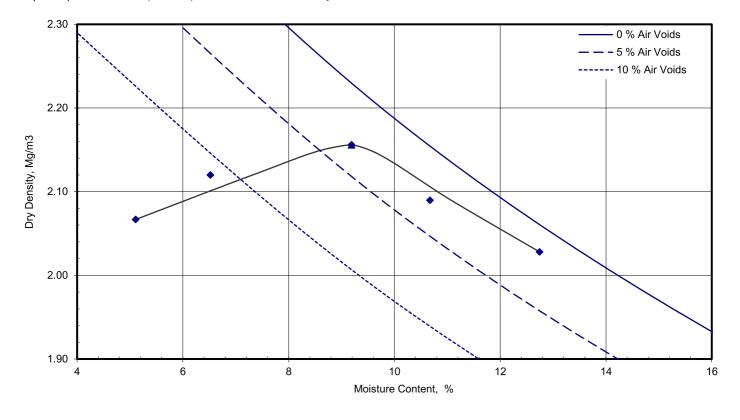
Laboratory Reference: 1872313 Hole No.: TP103 Sample Reference: Not Given

Sample Description: Yellowish brown gravelly clayey SAND

Sample Preparation: Sample was quartered and broken down by hand. Material used was natural.

Depth Top [m]: 1.00
Depth Base [m]: Not Given

Sample Type: B



Compaction Point No.		1	2	3	4	5
Moisture Content	%	5.1	6.5	9.2	11	13
Dry Density	Mg/m³	2.07	2.12	2.16	2.09	2.03

Mould Type		CBR		
Samples Used		Single sample tested		
Material Retained on 37.5 mm Sieve	%	3		
Material Retained on 20.0 mm Sieve	%	19		
Particle Density - Assumed	Mg/m³	2.80		
As received Moisture Content	%	5.1		
Maximum Dry Density	Mg/m³	2.16		

Optimum Moisture Content	%	9.2	

Note: Tested in Accordance with BS 1377-4: 1990: Clause 3.4 using 2.5kg [light] Rammer

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section

Page 1 of 1

for and on behalf of i2 Analytical Ltd

Date Reported: 02/06/2021

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GF 109.21





Determination of California Bearing Ratio

12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-4: 1990: Clause 7

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: Not Given Date Received: 26/04/2021 Date Tested: 01/06/2021

Sampled By: Client - PIL, DN & HB

Not soaked

Test Results:

Laboratory Reference: 1872306 BH101 Hole No.: Sample Reference: Not Given

Yellowish brown slightly gravelly SAND Sample Description:

Depth Top [m]: 3.00 Depth Base [m]: Not Given

Sample Type: B

Specimen Preparation:

Initial Specimen details

Condition Remoulded Soaking details Details Period of soaking Recompacted with specified standard effort using 2.5kg rammer

days Time to surface days Amount of swell recorded mm Dry density after soaking Mg/m3

Material retained on 20mm sieve removed

10 %

> Surcharge applied kg 4.8 kPa

Bulk density Dry density

2.16 Mq/m3 1.93 Mg/m3

Force v Penetration Plots

Moisture content 12

7.00 6.00 5.00 - Top data Force Applied kN · -· Top values 4.00 Top correction Base data 3.00 -- •- Base values 2.00 **Base Correction** 1.00

Results

0.00

TOP BASE

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report may not be reproduced other than in full without the prior written approval of the issuing

Curve	CBR Values, %			
correction applied	2.5mm	5mm	Highest	Average
No	7.8	16.0	16.0	17.0
No	8.6	18.0	18.0	17.0

Penetration mm

Moisture Content % 12 11

Remarks:

Test/ Specimen specific remarks:

Signed: Months

Monika Janoszek PL Deputy Head of Geotechnical Section

for and on behalf of i2 Analytical Ltd

laboratory. The results included within the report relate only to the sample(s) submitted for testing.

Page 1 of 1

Intille

Date Reported: 02/06/2021

GF 108.15



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8 12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 27/04/2021 Date Received: 26/04/2021 Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

½(σ1 - σ3)f

Test Results:

Laboratory Reference: 1872347 BH101 Hole No.: Sample Reference: Not Given

Greyish brown CLAY Sample Description:

Depth Top [m]: 8.00 Depth Base [m]: Not Given

Sample Type: U

%/min

kPa

kPa

%

Test Number
Length
Diameter
Bulk Density
Moisture Content
Dry Density

Membrane Correction

202.93 lmm 103.01 mm 1.97 Mg/m3 30 1.51 Mg/m3 0.72 kPa

Rate of Strain Cell Pressure Axial Strain at failure Deviator Stress, (σ 1 - σ 3)f Undrained Shear Strength, cu Mode of Failure

Membrane thickness

93 kPa Compound 0.30

1.97

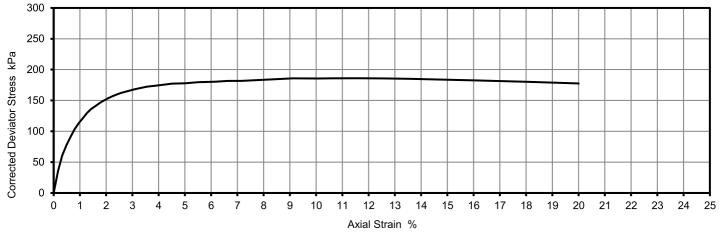
160

11.6

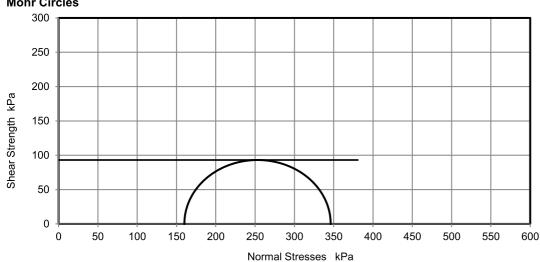
186

mm

Deviator Stress v Axial Strain



Mohr Circles





Position within sample



Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only

Remarks:

Signed: Marka

Monika Janoszek PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

Page 1 of 1

Intille

Date Reported: 02/06/2021



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Fierrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176
Date Sampled: 27/04/2021
Date Received: 26/04/2021
Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872348
Hole No.: BH102
Sample Reference: Not Given

Sample Description: Greyish brown CLAY

Depth Top [m]: 6.50
Depth Base [m]: Not Given

Sample Type: U

Test Number Length Diameter Bulk Density Moisture Content Dry Density

Membrane Correction

1 mm
202.62 mm
103.03 mm
1.97 Mg/m3
27 %
1.55 Mg/m3
0.30 kPa

Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (\sigma 1 - \sigma 3)f
Undrained Shear Strength, cu
Mode of Failure

Mode of Failure
Membrane thickness

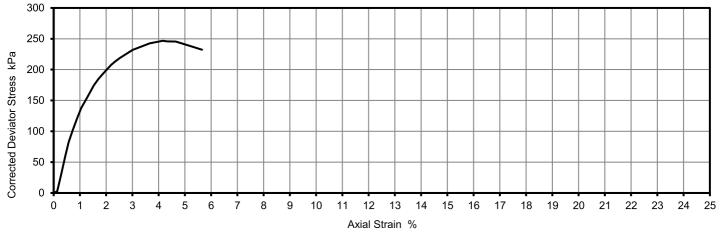
	-
1.97	%/min
130	kPa
4.2	%
247	kPa
100	

0.27

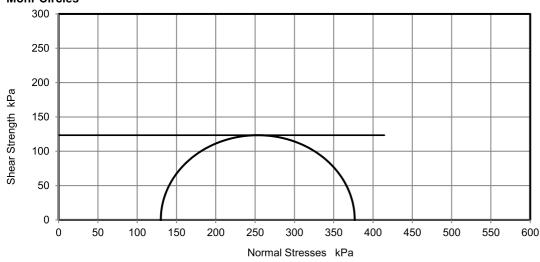
123 kPa ½(σ1 - σ3)f Brittle

mm

Deviator Stress v Axial Strain



Mohr Circles





Position within sample



Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

Page 1 of 1

Date Reported: 02/06/2021



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Fiarrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176
Date Sampled: 21/04/2021
Date Received: 26/04/2021
Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test Results:

Laboratory Reference: 1872350 Hole No.: BH103 Sample Reference: Not Given

Sample Description: Greyish brown CLAY

Depth Top [m]: 9.50
Depth Base [m]: Not Given

Sample Type: U

Test Number Length Diameter Bulk Density Moisture Content Dry Density

Membrane Correction

1 mm
103.49 mm
2.00 Mg/m3
26 %
1.59 Mg/m3
0.21 kPa

Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (σ 1 - σ 3)f
Undrained Shear Strength, cu

Mode of Failure
Membrane thickness

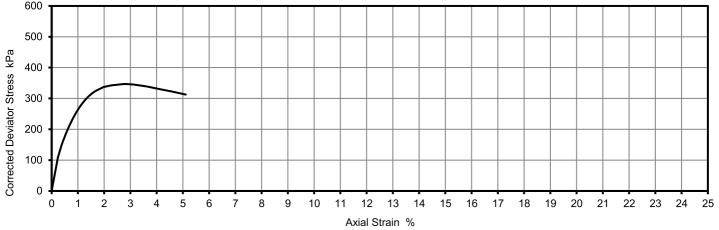
1.98 %/min 190 kPa 2.8 % 347 kPa

0.28

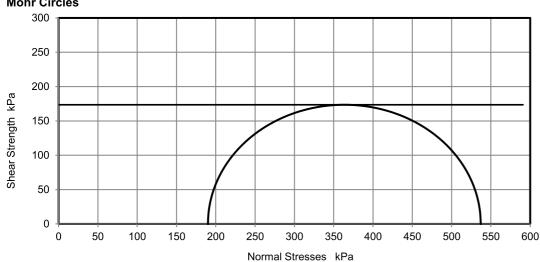
174 kPa ½(σ1 - σ3)f Brittle

mm

Deviator Stress v Axial Strain



Mohr Circles





Position within sample



Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

Page 1 of 1

Date Reported: 02/06/2021

GF 184.11

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Unconsolidated Undrained

<u>Triaxial Compression</u>
Tested in Accordance with:

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Flarrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis
Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176
Date Sampled: 22/04/2021
Date Received: 26/04/2021
Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872353 Hole No.: BH104 Sample Reference: Not Given

Sample Description: Greyish brown CLAY

Depth Top [m]: 11.00 Depth Base [m]: Not Given

Sample Type: U

Membrane Correction

1 mm
201.69 mm
102.29 mm
2.04 Mg/m3
24 %
1.64 Mg/m3
0.45 kPa

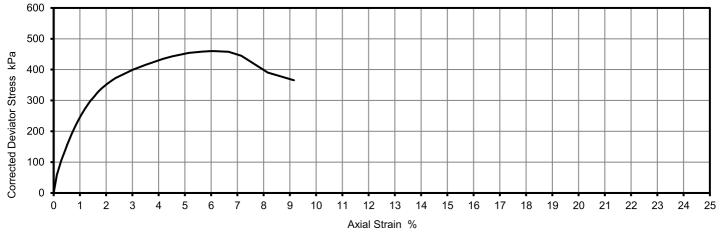
Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (σ1 - σ3)f
Undrained Shear Strength, cu

Mode of Failure Membrane thickness

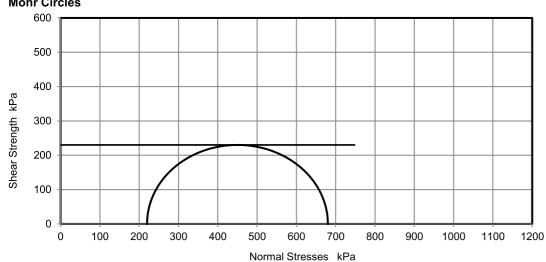
	_
1.98	%/min
220	kPa
6.0	%
460	kPa
230	kPa

230 kPa ½(σ1 - σ3)f Brittle 0.29 mm

Deviator Stress v Axial Strain



Mohr Circles



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Position within sample



Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

Page 1 of 1

Date Reported: 02/06/2021



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Flarrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis
Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176
Date Sampled: 22/04/2021
Date Received: 26/04/2021
Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872354 Hole No.: BH104 Sample Reference: Not Given

Sample Description: Greyish brown CLAY

Depth Top [m]: 17.00

Depth Base [m]: Not Given

Sample Type: U

Test Number Length Diameter Bulk Density Moisture Content Dry Density

Membrane Correction

1 mm
102.44 mm
2.06 Mg/m3
24 %
1.66 Mg/m3
0.24 kPa

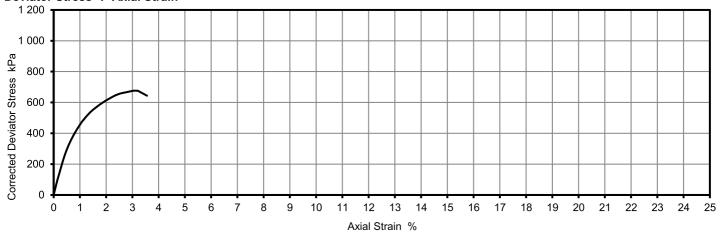
Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (σ1 - σ3)f
Undrained Shear Strength, cu
Mode of Failure

Membrane thickness

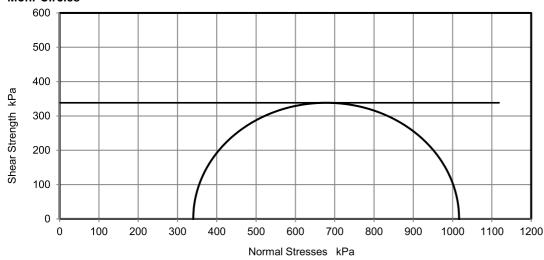
1.99 %/min 340 kPa 3.0 % 677 kPa 338 kPa ½(σ1-σ3)f

Brittle 0.30 mm

	_			
Deviator	Stress	ν	Axial	Strain



Mohr Circles



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report may not be reproduced other than in full without the prior written approval of the issuing



Position within sample



Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

Page 1 of 1

Intille

Date Reported: 02/06/2021



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Fiarrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Date Sampled: 22/04/2021
Date Received: 26/04/2021

Date Tested: 20/05/2021 Sampled By: Client - PIL, DN & HB

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test Results:

Laboratory Reference:1872355Depth Top [m]: 20.00Hole No.:BH104Depth Base [m]: Not GivenSample Reference:Not GivenSample Type: U

Sample Description: Greyish brown CLAY

Test Number	1	
Length	200.60	mm
Diameter	102.82	mm
Bulk Density	2.01	Mg/m3
Moisture Content	26	%
Dry Density	1.60	Mg/m3
Membrane Correction	0.10	kPa

Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (\sigma 1 - \sigma 3)f
Undrained Shear Strength, cu

Mode of Failure Membrane thickness

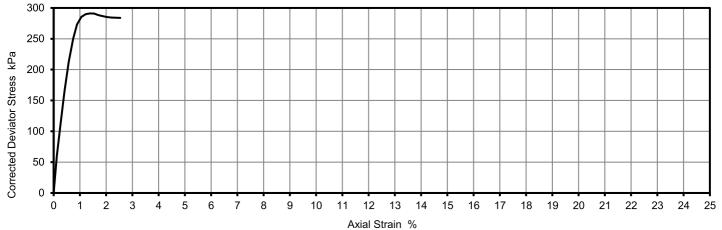
	_
1.99	%/min
400	kPa
1.4	%
291	kPa

0.27

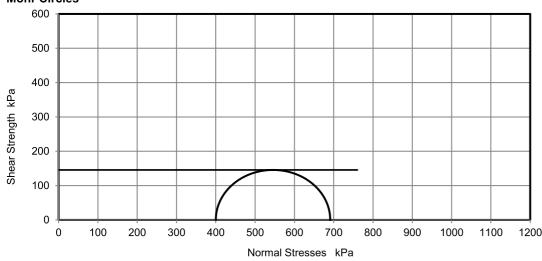
146 kPa ½(σ1 - σ3)f

mm

Deviator Stress v Axial Strain



Mohr Circles





Position within sample



Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

Page 1 of 1

Date Reported: 02/06/2021



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Fiarrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176
Date Sampled: 23/04/2021
Date Received: 26/04/2021
Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872356 Hole No.: BH104 Sample Reference: Not Given

Sample Description: Greyish brown CLAY

Depth Top [m]: 23.00 Depth Base [m]: Not Given

Sample Type: U

Test Number
Length
Diameter
Bulk Density
Moisture Content
Dry Density

Membrane Correction

Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (σ1 - σ3)f
Undrained Shear Strength, cu

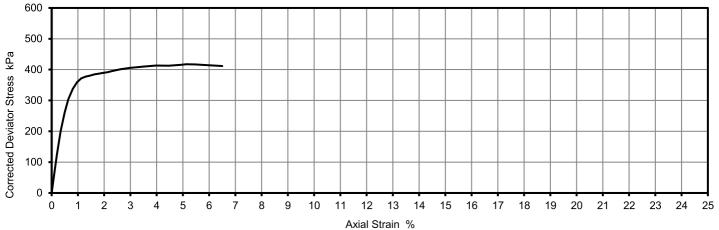
Mode of Failure Membrane thickness

2.00	%/min
460	kPa
5.1	%
417	kPa
200	I _L D _o

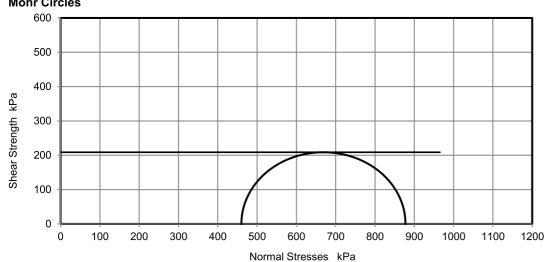
0.28

209 kPa ½(σ1 - σ3)f

Deviator Stress v Axial Strain



Mohr Circles



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laboratory. The results included within the report relate only to the sample(s) submitted for testing.



Position within sample



Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section

Page 1 of 1

for and on behalf of i2 Analytical Ltd

Date Reported: 02/06/2021



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Fierrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 26/04/2021 Date Received: 26/04/2021 Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872358
Hole No.: BH105
Sample Reference: Not Given

Sample Description: Greyish brown CLAY

Depth Top [m]: 12.50 Depth Base [m]: Not Given

Sample Type: U

Test Number
Length
Diameter
Bulk Density
Moisture Content
Dry Density

Membrane Correction

1	
201.29	mm
103.59	mm
2.00	Mg/m3
26	%
1.58	Mg/m3
0.24	kPa

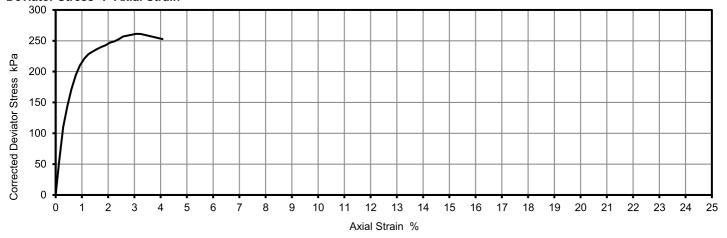
Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (σ 1 - σ 3)f
Undrained Shear Strength, cu
Mode of Failure

Membrane thickness

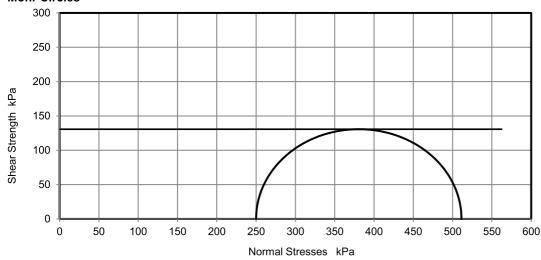
1.99	%/min
250	kPa
3.1	%
261	kPa
131	kPa ½(σ1 - σ3)f

Brittle 0.29 mm

Deviator Stress v Axial Strain



Mohr Circles





Position within sample

Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

Page 1 of 1

Intille

Date Reported: 02/06/2021



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8 12 Analytical Ltd Unit 8 Harrowden Road Bracionilla Industrial Estate Northampton NN4 7EB



LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 26/04/2021 Date Received: 26/04/2021 Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872359 BH105 Hole No.: Sample Reference: Not Given

Greyish brown CLAY Sample Description:

Depth Top [m]: 15.50 Depth Base [m]: Not Given

Sample Type: U

Test Number Lenath Diameter **Bulk Density** Moisture Content Dry Density

Membrane Correction

200.23 mm 102.67 mm 2.05 Mg/m3 25 1 64 Mg/m3 0.49 kPa

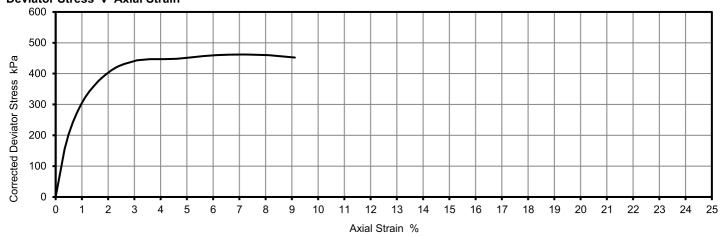
Rate of Strain Cell Pressure Axial Strain at failure Deviator Stress, (σ 1 - σ 3)f Undrained Shear Strength, cu Mode of Failure

Membrane thickness

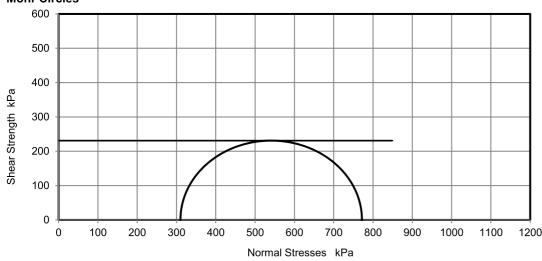
2.00 %/min 310 kPa 7.3 % 462 kPa 231 kPa ½(σ1 - σ3)f

Brittle 0.28 mm

Deviator Stress v Axial Strain



Mohr Circles



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laboratory. The results included within the report relate only to the sample(s) submitted for testing.



Position within sample

Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only

Remarks:

Signed: Marka

Monika Janoszek PL Deputy Head of Geotechnical Section

Intille

for and on behalf of i2 Analytical Ltd **Date Reported: 02/06/2021**

Page 1 of 1



Unconsolidated Undrained

Triaxial Compression

Tested in Accordance with: BS 1377-7: 1990: Clause 8

12 Analytical Ltd Unit 5 Fiarrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis

Site Address: Manor Road

Philip Lewis

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 26/04/2021 Date Received: 26/04/2021 Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Test Results:

Laboratory Reference: 1872361

Hole No.: BH105

Sample Reference: Not Given

Sample Description: Greyish brown CLAY

Sample Type: U

1.99 %/min

Depth Top [m]: 21.50

Depth Base [m]: Not Given

Test Number
Length
Diameter
Bulk Density
Moisture Content
Dry Density

Membrane Correction

1	
201.21	mm
103.33	mm
2.02	Mg/m3
27	%
1.59	Mg/m3
0.27	kPa

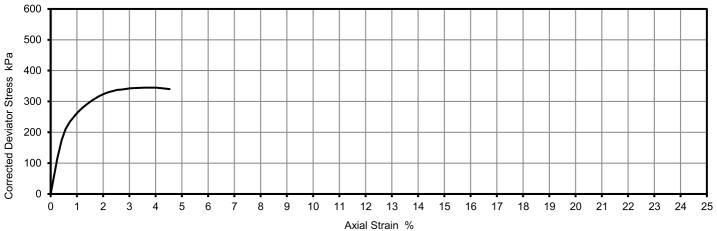
Rate of Strain
Cell Pressure
Axial Strain at failure
Deviator Stress, (σ 1 - σ 3)f
Undrained Shear Strength, cu
Mode of Failure

430	k₽a	
3.5	%	
345	kPa	
172	kPa	½(σ1 - σ3)f
Brittle		

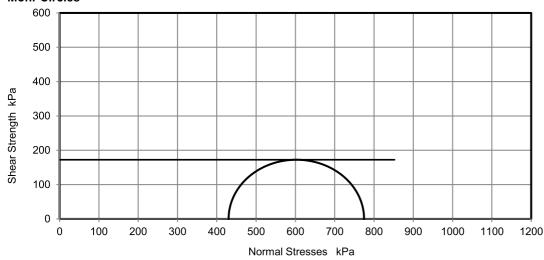
Mode of Failure Membrane thickness

0.29 mm

Deviator Stress v Axial Strain



Mohr Circles





Position within sample



Note: Deviator stress corrected for area change and membrane effects. Mohr circles and their interpretation is not covered by BS1377. This is provided for information only.

Remarks:

Signed:

Monika Janoszek
PL Deputy Head of Geotechnical Section
for and on behalf of i2 Analytical Ltd

Page 1 of 1

Date Reported: 02/06/2021





12 Analytical Ltd Unit 5 Harrowden Road Brackmills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-5: 1990: Clause 3

Client: LMB Geosolutions Ltd

Client Address: 28 Dresden Road, London,

20 Diesdell Road, Lolldoll

N19 3BD

Contact: Philip Lewis
Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Date Sampled: 21/04/2021

Date Received: 26/04/2021

Date Tested: 20/05/2021

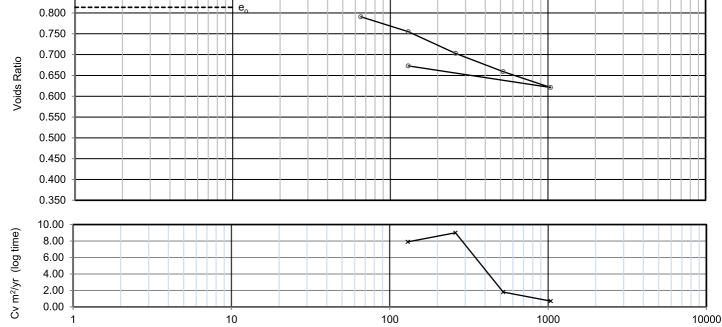
Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872349
Hole No.: BH103
Sample Reference: Not Given

Depth Top [m]: 6.50
Depth Base [m]: Not Given
Sample Type: U





Applied Pressure kPa

Applied Pressure	Voids ratio	Μv	Cv (t50, log)	Cv (t90, root	Csec
kPa		m2/MN	m2/yr	m2/yr	
0	0.814	-	-	-	-
65	0.791	0.19	N/A	N/A	N/A
130	0.755	0.31	7.9	13	0.00064
260	0.703	0.23	9	18	0.00064
520	0.659	0.098	1.8	15	0.0013
1 040	0.621	0.044	0.7	2.2	0.0013
130	0.673	0.035			

Preparation

Index tests

Orientation of the sample Particle density Liquid limit

Liquid limit Plastic limit

Vertical		_
assumed	2.65	Mg/m3
N/A		%
N/A		%

Specimen details
Diameter
Height
Moisture Content
Bulk density
Dry density
Voids Ratio
Saturation
Avg. temperature for test
Swelling Pressure
Settlement on saturation

Initial	Final	
50.00	-	mm
20.02	18.46	mm
29	30	%
1.88	2.06	Mg/m3
1.46	1.58	Mg/m3
0.814	0.673	
93	118	%
22	°C	
Not measured		kPa
		%

Note: Cv corrected to 20°C

Remarks:

Signed:

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd





12 Analytical Ltd Unit 5 Herrowden Road Brackmills Industrial Estate Northampton NN4 7EB



4041 Tested in Accordance with: BS 1377-5: 1990: Clause 3
Client: LMB Geosolutions Ltd

Client Address:

28 Dresden Road, London,

N19 3BD

Brown CLAY

Contact: Philip Lewis
Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176

Date Sampled: 21/04/2021

Date Received: 26/04/2021

Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

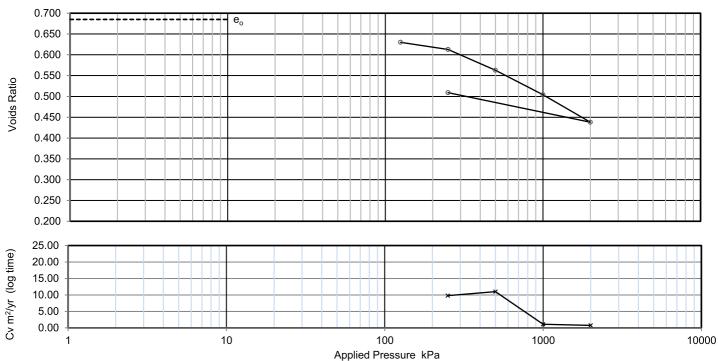
Test Results:

Sample Description:

Laboratory Reference: 1872351
Hole No.: BH103
Sample Reference: Not Given

Depth Base [m]: Not Given Sample Type: U

Depth Top [m]: 12.50



Preparation

Applied Pressure	Voids ratio	Mv	Cv (t50, log)	Cv (t90, root	Csec
kPa		m2/MN	m2/yr	m2/yr	
0	0.685	-	-	-	-
125	0.630	0.26	N/A	N/A	N/A
250	0.613	0.083	9.8	25	0.00062
500	0.563	0.12	11	14	0.0013
1 000	0.504	0.075	1.1	4.8	0.0019
2 000	0.438	0.044	0.75	1.1	0.0025
250	0.509	0.028			

Index tests
Orientation of the sample
Particle density

Liquid limit Plastic limit

Vertical		_
assumed	2.65	Mg/m3
N/A		%
N/A		%

Specimen details
Diameter
Height
Moisture Content
Bulk density
Dry density
Voids Ratio
Saturation
Avg. temperature for test
Swelling Pressure
Settlement on saturation

Initial	Final	
50.00	-	mm
20.13	18.03	mm
25	26	%
1.96	2.21	Mg/m3
1.57	1.76	Mg/m3
0.685	0.509	
96	134	%
22	°C	
Not measured		kPa
		%

Note: Cv corrected to 20°C

Remarks:

Signed:

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd





12 Analytical Ltd Unit 8 Harrowden Road Bracionills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-5: 1990: Clause 3

LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Brown CLAY

Contact: Philip Lewis

Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 22/04/2021 Date Received: 26/04/2021 Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

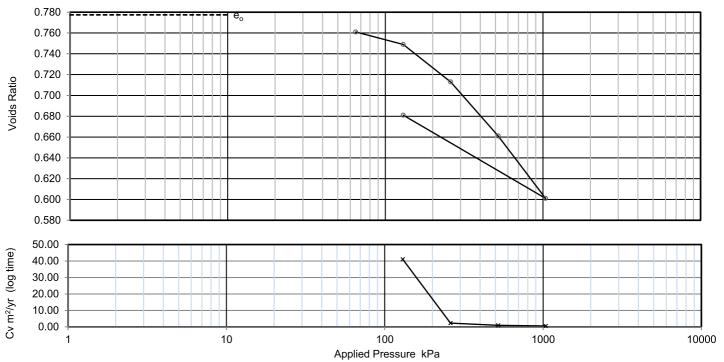
Test Results:

Sample Description:

Laboratory Reference: 1872352 Hole No.: BH104 Not Given Sample Reference:

Depth Top [m]: 6.50 Depth Base [m]: Not Given

Sample Type: U



Applied	Voids	Mv	Cv	Cv	Csec
Pressure kPa	ratio		(t50, log)		
		m2/MN	m2/yr	m2/yr	
0	0.777	-	-	-	-
65	0.761	0.14	N/A	N/A	N/A
130	0.749	0.1	41	45	0.00043
260	0.713	0.16	2.3	12	0.0013
520	0.661	0.12	0.88	3.5	0.0019
1 040	0.601	0.069	0.55	0.96	0.0013
130	0.681	0.054			

Preparation

Index tests

Orientation of the sample Particle density

Liquid limit Plastic limit

Vertical

assumed

N/A N/A

2.65	Mg/m3
	<u></u> %
	%

Specimen details Diameter Height Moisture Content Bulk density Dry density Voids Ratio Saturation Avg. temperature for test Swelling Pressure Settlement on saturation

Initial	Final	
50.10	-	mm
20.04	18.95	mm
29	30	%
1.93	2.04	Mg/m3
1.49	1.58	Mg/m3
0.777	0.681	
100	115	%
22	°C	
Not measured		kPa

Note: Cv corrected to 20°C

Remarks:

Signed: Monks

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd





12 Analytical Ltd Unit 8 Harrowden Road Bracionills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-5: 1990: Clause 3

LMB Geosolutions Ltd Client:

Client Address: 28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

Job Number: 21-75176 Date Sampled: 26/04/2021 Date Received: 26/04/2021 Date Tested: 20/05/2021

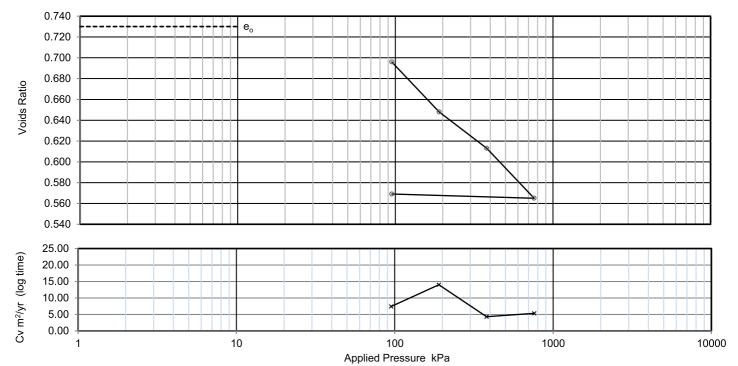
Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872357 Hole No.: BH105 Not Given Sample Reference: Sample Description: **Brown CLAY**

Depth Top [m]: 9.50 Depth Base [m]: Not Given

Sample Type: U



Applied Pressure	Voids ratio	Mv	Cv (t50, log)	Cv (t90, root	Csec
kPa		m2/MN	m2/yr	m2/yr	
0	0.730	-	-	-	-
95	0.696	0.21	7.4	11	0.00021
190	0.648	0.3	14	16	0.0013
380	0.613	0.11	4.3	6.6	0.0016
760	0.565	0.079	5.3	6.4	0.0012
95	0.569	0.0036			

Preparation

Index tests

Orientation of the sample Particle density

Liquid limit Plastic limit

Vertical		_
assumed	2.65	Mg/m3
N/A		%
N/A		%
	-	

Final

mm

Specimen details Diameter Height Moisture Content Bulk density Dry density Voids Ratio Saturation Avg. temperature for test

20.09	18.22	mm
26	27	%
1.93	2.14	Mg/m3
1.53	1.69	Mg/m3
0.730	0.569	
94	126	%
22	°C	
Not me	kPa	

Initial

50.05

Swelling Pressure Settlement on saturation

Note: Cv corrected to 20°C

Remarks:

Signed: Monks

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

Date Reported: 02/06/2021

Opinions and interpretations expressed herein are outside of the scope of the UKAS Accreditation. This





12 Analytical Ltd Unit 8 Harrowden Road Bracionills Industrial Estate Northampton NN4 7EB



Tested in Accordance with: BS 1377-5: 1990: Clause 3 LMB Geosolutions Ltd Client:

Client Address:

28 Dresden Road, London,

N19 3BD

Contact: Philip Lewis Site Address: Manor Road

Testing carried out at i2 Analytical Limited, ul. Pionierow 39, 41-711 Ruda Slaska, Poland

Client Reference: LMB-MANOR ROAD

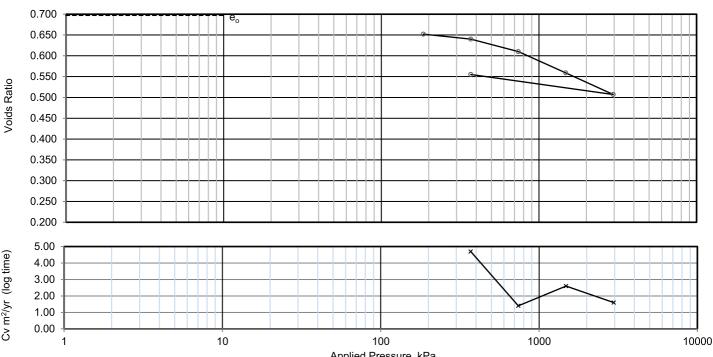
Job Number: 21-75176 Date Sampled: 26/04/2021 Date Received: 26/04/2021 Date Tested: 20/05/2021

Sampled By: Client - PIL, DN & HB

Test Results:

Laboratory Reference: 1872360 BH105 Hole No.: Not Given Sample Reference: Sample Description: **Brown CLAY**

Depth Top [m]: 18.50 Depth Base [m]: Not Given Sample Type: U



Applied Pressure kPa

Applied Pressure	Voids ratio	Mv	Cv (t50, log)	Cv (t90, root	Csec
kPa		m2/MN	m2/yr	m2/yr	
0	0.698	-	-	-	-
185	0.652	0.15	N/A	N/A	N/A
370	0.640	0.038	4.7	N/A	N/A
740	0.610	0.05	1.4	N/A	N/A
1 480	0.559	0.043	2.6	23	N/A
2 960	0.507	0.023	1.6	8.2	0.0025
370	0.555	0.012			

Preparation

Index tests

Orientation of the sample Particle density Liquid limit Plastic limit

Vertical		_
assumed	2.65	Mg/m3
N/A		%
N/A		%

Specimen details Diameter Height Moisture Content Bulk density Dry density Voids Ratio Saturation Avg. temperature for test Swelling Pressure Settlement on saturation

Initial	Final	
50.06	-	mm
20.03	18.35	mm
25	26	%
1.95	2.14	Mg/m3
1.56	1.70	Mg/m3
0.698	0.555	1
95	122	%
22.0		°C
Not measured		kPa
		%

Note: Cv corrected to 20°C

Remarks:

Signed: Months

Monika Janoszek

PL Deputy Head of Geotechnical Section for and on behalf of i2 Analytical Ltd

APPENDICES

APPENDIX D MONITORING RESULTS

APPENDICES

GROUND GAS & GROUNDWATER

PROJECT: Manor Road, Richmond

Date 05/05/2021 Operator PIL

Start 1009 Weather Dry, sunny & cold Atmospheric Pressure (mb) Finish 1009 Rins 08:15

PROJECT: Manor Road, Richmond

Date 13/05/2021 Operator PIL

Start 1006 Atmospheric Pressure (mb) Finish 1004

Weather cloudy with rain Time: 09:30

Ground	Elevation		6.703			6:399			6:539			6.246			6.254	
. Level	lgqm		4.24			4.08			2.63			2.41			2.71	
Groundwater Level	m AOD		2.46			2.32			3.91			3.84			3.54	
	rio (ppm)		06.0			0.80			0.00			1.40			1.20	
(14/1)	FIOW (I/III.)		0.00			0.00			0.00			0.00			0.00	
(maa/)	125 (ppiii)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
(2007)	CO(ppm)	0.00	0.00	0.00	00.00	00.00	00.00	0.00	0.00	0.00	00.00	00.00	00.00	0.00	00.00	0.00
(%)	02 (%)	15.10	15.10	15.10	09'9	09'9	09'9	17.60	17.60	17.60	17.40	17.40	17.40	13.50	13.50	13.50
(70)	CO ₂ (%)	3.80	3.80	3.80	1.90	1.90	1.90	2.70	2.70	2.70	0.20	0.20	0.20	00'0	00.0	00.0
(70)	Cn4 (%)	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0
e H	a E E	09	120	240	09	120	240	09	120	240	09	120	240	09	120	240
1	Location		BH101			BH102			BH103			BH104			BH105	

PROJECT: Manor Road, Richmond

Date 26/05/2021 Operator PIL

Start 1015 Atmospheric Pressure (mb) Finish 1015

Weather cloud with sunny spells Time: 14:10

Ground	Elevation		6.703			6.399			6.539			6.246			6.254	
Level	mbgl		4.14			3.99			2.28			1.57			2.22	
Groundwater Level	m AOD		2.56			2.41			4.26			4.68			4.03	
(سطط/ ۱۵۵	0.90 0.60		06:0			06:0		1.40			0.40					
(44/1) ///	(III /I) MOIL		00.00			00.00		0.00			0.00			0.00		
(muu) S H	11 ₂ 2 (pp111)	00:0	00.0	00.0	00'0	00.0	00:0	00:0	00:0	00:0	00:0	00.0	00.0	00'0	00.0	000
(222)	(Indd)	00'0	00'0	00'0	00'0	00'0	00'0	00'0	00'0	00.0	00'0	00'0	00'0	00'0	00'0	000
(%) U	02 (70)	13.30	13.30	13.30	4.80	4.80	4.80	16.90	16.90	16.90	17.90	17.90	17.90	14.10	14.10	0111
(%) UJ	(20)	4.80	4.80	4.80	08'0	08'0	08'0	3.30	3.30	3.30	0.10	0.10	0.10	00'0	00'0	000
(%) HJ	(14 (70)	0.00	0.00	0.00	0.00	0.00	00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	000
Time	ט ב ב	09	120	240	09	120	240	09	120	240	09	120	240	09	120	OVC
acitico l	FOCATION		BH101			BH102			BH103			BH104			BH105	

PROJECT: Manor Road, Richmond

	Weather
PIL	10
Operator	1015
26/05/2021	Start
Date	

	Ground	Elevation		6.703			6.399			6.539			6.246			6.254	
	Level	mbgl		4.14			3.99			2.28			1.57			2.22	
unny spells	Groundwater Level	m AOD		2.56			2.41			4.26			4.68			4.03	
cloudy with sunny spells 14:10	(222)	נוווקל) בוב		06.0			09.0			06.0			1.40			0.40	
Weather Time	(*4/1) /****	riow (i/iii)		0.00		00.00		00.0		0.00			0.00				
	(maa) S.H	(1125 (12511)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	00'0
	(222)	co(ppiii)	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	00.0	0.00
	(%)	02 (70)	13.30	13.30	13.30	4.80	4.80	4.80	16.90	16.90	16.90	17.90	17.90	17.90	14.10	14.10	14.10
1015 1015	(%)	CO2 (70)	4.80	4.80	4.80	0:30	0:30	0:30	3.30	3.30	3.30	0.10	0.10	0.10	00.0	00.0	0.00
Start Finish	(%) HJ	C114 (70)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	00.00
ssure (mb)	Timo	ווום	09	120	240	09	120	240	09	120	240	09	120	240	09	120	240
Atmospheric Pressure (mb)	acite o	LOCATION		BH101			BH102			BH103			BH104			BH105	

APPENDICES

LOW FLOW SAMPLING PARAMETERS

Project: Manor Road, Richmond

05/05/2021 11:19 0.7282129 6.827741

6.601953

6.538225

6.607172

05/05/2021 11:20 0.7026778

287.7785

290.0841

290.9618

289.1287

287.6305

12.19998

12.29278

12.4142

12.40891

12.29461

380.9014

383.0545

383.0404

380.6777

379.7969

 $0.1824522 \qquad 0.246868 \quad 8.735367 \quad \text{-}98.30719 \quad 26.37241$

26.39124

26.3683

26.37948

Title:	Low flow gro		pling parameters							
	Ü									
BH101										
Date Time	RDO (mg/L)		Actual Cond (μS/cm)	Temp (°C)		Salinity (PSU)	TDS (ppt)	pН	pH mV (mV)	ORP (mV)
05/05/2021 10:42		77.23751	668.1374	15.11404	823.6628	0.4066848	0.5353808		-79.69743	68.12289
05/05/2021 10:43	1.801131	17.65158	660.3032	14.1179	833.5562	0.4112798	0.5418115		-80.95766	69.90865
05/05/2021 10:44		8.457654	657.9756	13.17078	850.0297	0.4191783	0.5525193		-81.67908	69.56227
05/05/2021 10:45		6.133173 4.316171	656.4459	12.67223	858.616	0.4232542	0.5581004		-81.60744	68.86504
05/05/2021 10:46 05/05/2021 10:47	0.4581451 0.377678	3.547046	655.8508 655.0607	12.34008 12.20251	865.0154 866.9779	0.4263009 0.4272072	0.56226 0.5635357	8.424559	-81.35359 -80.89933	67.8484 66.8737
05/05/2021 10:48		3.352278	654.3108	12.02267	869.9403	0.4272072	0.5654612		-80.33058	65.83188
05/05/2021 10:48		3.570752	654.348	12.02207	870.1019	0.4285314	0.5655662	8.39649	-79.73145	65.0733
05/05/2021 10:49		3.148088	654.0021	12.01967	869.5961	0.4284134	0.5652375		-79.17606	64.52388
05/05/2021 10:51		2.90106	655.4849	12.0367	871.1909	0.4292408	0.566274	8.37446	-78.53481	63.99355
05/05/2021 10:52		2.980915	655.2318	12.0733	870.0461	0.4286825	0.56553	8.362201	-77.87997	63.63115
05/05/2021 10:53	0.314369	2.946393	655.0823	12.11124	869.0115	0.4281812	0.5648575		-77.01004	63.54078
05/05/2021 10:54	0.2920138	2.736843	654.6047	12.11308	868.3375	0.4278381	0.5644194	8.33438	-76.36929	63.04823
05/05/2021 10:55	0.2586502	2.421644	654.0107	12.06799	868.5419	0.4279098	0.5645522	8.32434	-75.80959	62.56103
05/05/2021 10:56	0.2176058	2.033168	652.9325	11.97938	869.0632	0.4281114	0.5648911	8.319927	-75.54541	62.10019
BH102										
Date Time	RDO (mg/L)	RDO (%Sat)	Actual Cond (μS/cm)	Temp (°C)	Specific Cond (μS/cm)	Salinity (PSU)	TDS (ppt)	рН	pH mV (mV)	ORP (mV)
05/05/2021 10:13	7.556651	74.23085	730.4267	14.21008	920.0342	0.4556993	0.5980222		-94.83688	121.2939
05/05/2021 10:14		70.66264	721.5174	14.16933	909.7041	0.450364		8.614758	-92.32573	104.9369
05/05/2021 10:15	6.911228	67.82539	720.3942	14.16412	908.4019	0.4496917	0.5904613		-90.2065	93.85905
05/05/2021 10:16		65.84639	720.8614	14.18246	908.5897	0.4497985	0.5905833		-88.78932	86.01592
05/05/2021 10:17	6.598314	64.8259	720.5057	14.21325	907.4686	0.4492395	0.5898547		-87.42088	78.50063
05/05/2021 10:18		62.96823	719.4929	14.26148	905.1428	0.4480707	0.5883428		-86.04246	71.43269
05/05/2021 10:19 05/05/2021 10:20	6.194984	60.97193 58.94492	717.878	14.30086	902.2576	0.4466094	0.5864674 0.5875601		-84.94746 -83.88134	66.62542
	6.001934		717.4919	14.20103	903.9386	0.4474182				61.10301
05/05/2021 10:21	5.669492 2.830679	55.66006	719.9942	14.17816	907.5905	0.4492825	0.5899339		-83.14591	56.95773
05/05/2021 10:22 05/05/2021 10:23	1.205499	27.85766	721.6919 718.9145	14.29475	907.1841 902.9816	0.4491384	0.5896697 0.5869381	8.42918	-82.1646 -81.24643	53.44404 49.79145
05/05/2021 10:23		11.86955 9.127595	715.3638	14.32755 14.1367	902.6547	0.4469961 0.4467225	0.5869381		-81.24643 -80.60056	49.79145
05/05/2021 10:24	1.103255	10.77306	712.7466	13.93971	903.6425	0.4467223		8.399355	-80.42566	44.15204
05/05/2021 10:25	0.920544	8.95225	711.3823	13.75588	905.9456	0.4471173	0.5888646	8.396805	-80.23419	41.84929
05/05/2021 10:27		8.985035	711.4564	13.66727	907.9969	0.4481925	0.590198	8.392066	-79.9495	39.77244
BH103	0.3237274	0.505055	711.4304	13.00727	307.3303	0.4431323	0.550150	0.552000	-73.5433	33.77244
Date Time	RDO (mg/L)	RDO (%Sat)	Actual Cond (μS/cm)	Temp (°C)	Specific Cond (µS/cm)	Salinity (PSU)	TDS (ppt)	рН	pH mV (mV)	ORP (mV)
05/05/2021 11:54		78.85471	614.6922	15.2622	755.1427	0.3717332	0.4908428	8.444586	-83.29088	66.68552
05/05/2021 11:55	1.908258	18.89764	609.6241	14.61986	760.3771	0.3741436	0.4942451		-87.52925	74.56171
05/05/2021 11:56		5.794157	608.7193	14.20306	766.8629	0.3772646		8.551888	-88.87122	76.82431
05/05/2021 11:57		3.510677	607.2022	13.89342	770.6937	0.3790705		8.548281	-88.58092	76.8114
05/05/2021 11:58		2.694971	604.6499	13.57339	773.455	0.3803175	0.5027457	8.5406	-88.05513	75.88409
05/05/2021 11:59	0.2340062	2.249701	603.3826	13.26607	777.6732	0.3823043	0.5054876		-87.20631	74.41362
05/05/2021 12:00		2.115569	606.7093	13.19312	783.3676	0.3851647	0.509189	8.507113	-86.11819	73.39217
05/05/2021 12:01		1.978378	607.6974	13.16553	785.1776	0.3860715	0.5103654	8.488784	-85.10545	72.29496
05/05/2021 12:02	0.1948431	1.868985	607.8431	13.16349	785.4055	0.3861865	0.5105136	8.472677	-84.22292	71.2588
05/05/2021 12:03	0.1852591	1.775463	607.0056	13.12517	785.0657	0.3859922	0.5102927	8.452774	-83.1222	70.25278
05/05/2021 12:04	0.1746506	1.673551	604.7597	13.11855	782.2891	0.3845743	0.5084879	8.438547	-82.34122	69.36745
05/05/2021 12:05	0.1579125	1.513112	602.097	13.1181	778.8534	0.3828245	0.5062547	8.425121	-81.60609	68.66571
05/05/2021 12:06	0.1571626	1.508888	599.1669	13.20772	773.3507	0.3800721	0.502678	8.408623	-80.72845	67.90296
05/05/2021 12:07		1.502141	595.3541	13.25695	767.4979	0.3771196	0.4988736	8.398083	-80.16466	67.75018
05/05/2021 12:08	0.1487668	1.431327	586.3081	13.30412	754.9594	0.3707662	0.4907236	8.384337	-79.42491	66.55438
BH104										
Date Time					Specific Cond (µS/cm)			pН	pH mV (mV)	
05/05/2021 09:43		94.31985	389.2034	15.68669	473.4167	0.2296761	0.3077209		-96.83071	197.6958
05/05/2021 09:44	3.01545	29.78647	382.5605	14.52226	478.2752	0.231836	0.3108789	8.73497	-99.0365	156.0789
05/05/2021 09:45		22.40708	380.3952	13.64614	485.7299 489.6621	0.2353119		8.747896	-99.45983 -99.56835	140.4572
05/05/2021 09:46 05/05/2021 09:47	2.168454 2.032953	20.76633 19.29364	378.4934 377.9351	13.11353	489.6621	0.2371057 0.2390853	0.3182804 0.321043	8.753529	-99.56835 -99.56538	132.3889 126.2875
05/05/2021 09:47		18.79078	377.9351	12.70611 12.57513	493.9122 497.9832	0.2390853	0.321043	8.75579 8.741215	-99.56538 -98.72332	120.9547
05/05/2021 09:49	1.940698	18.43806	381.0872	12.74226	497.5828	0.2410663	0.3234288		-98.72332 -98.07315	117.1655
05/05/2021 09:50		18.12527	381.6316	12.74226	495.9408	0.2409239	0.3234288		-98.07313 -97.65157	113.6895
05/05/2021 09:51		17.909	382.3911	13.0647	495.3023	0.2398995	0.3219465		-97.18412	112.5821
05/05/2021 09:52		17.72472	382.3281	13.14524	494.2359	0.2393946	0.3212534		-96.71082	108.749
05/05/2021 09:53		17.61024	381.5777	13.1279	493.4771	0.2390109	0.3207601		-96.34608	106.4446
05/05/2021 09:54		17.42799	381.7734	13.05921	494.5694	0.2395325	0.3214701		-96.04498	104.2928
05/05/2021 09:55		17.20867	382.6132	13.06584	495.576	0.2400362	0.3221244		-95.93182	102.2902
05/05/2021 09:56		17.04882	381.556	13.1076	493.6967	0.2391136	0.3209029		-95.62495	100.6576
05/05/2021 09:57		16.89458	381.6548	12.97907	495.398	0.2399186	0.3220087		-95.57556	99.0428
BH105					-		,			
Date Time	RDO (mg/L)	RDO (%Sat)	Actual Cond (μS/cm)	Temp (°C)	Specific Cond (µS/cm)	Salinity (PSU)	TDS (ppt)	рН	pH mV (mV)	ORP (mV)
05/05/2021 11:09		87.19529	297.5347	12.52288	390.6259	0.1878497		8.489305	-84.94285	53.2842
05/05/2021 11:10		26.65323	288.3651	12.32883	380.4386	0.1827776	0.2472851		-87.36933	36.66759
05/05/2021 11:11		12.31678	286.8521	12.11583	380.4847	0.1827408	0.2473151		-88.75644	29.19265
05/05/2021 11:12		10.43116	286.2044	11.9681	381.0517	0.1829775	0.2476836		-90.16037	26.54598
05/05/2021 11:13		9.381794	285.9891	11.86282	381.7871	0.1833085	0.2481616		-91.397	25.57231
05/05/2021 11:14		8.517929	285.5616	11.77821	382.0408	0.1834081	0.2483265		-92.42249	25.61559
05/05/2021 11:15		7.909876	285.3089	11.74223	382.0539	0.1834038		8.649614	-93.44305	25.69355
05/05/2021 11:16		7.463638	286.9645	11.78006	383.8995	0.1843232	0.2495347	8.666391	-94.37147	26.13738
05/05/2021 11:17		7.086498	288.3802	11.96972	383.9328	0.1843958	0.2495563		-95.30289	26.35827
05/05/2021 11:18	0.7724446	7.232401	289.0989	12.13975	383.2326	0.1841002	0.2491012	8.695709	-96.09018	26.35078
05/05/2021 11:10	0.7292120	6 9277/1	207 7705	12 10009	290 0014	0.1920606	0.2475950	9 705924	06 66245	26 20052

APPENDICES

APPENDIX E SPT CALIBRATION CERTIFICATES

SPT Hammer Energy Test Report

In incrordance with 85EN 15O 22476-3:2005

Southern Testing Unit 11 Charlwood Road East Grinstead West Sussex RH19 2HU SFT Hammer Ref: 110RP113 Test Date: 21/0W2020 Report Date: 21/0W2020 File Name: 110RP113.ppt

Test Operator NPB

Instrumented Rod Data

Diameter (J. (mm): 54

Wall Thickness t. (mm): 6.3

Assumed Modulus E. (GPa): 208

Accelerometer No.1: 6458

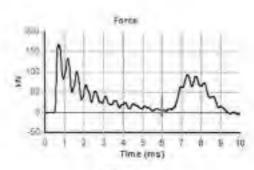
Accelerometer No.2: 9607

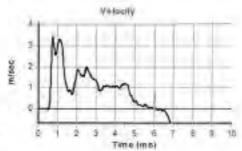
SPT Hammer Information

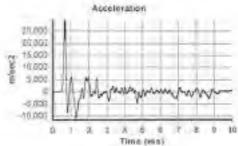
Hammer Mass m (kg): 63.5 Falling Height h (mm): 700 SPT String Length L (m): 14.5

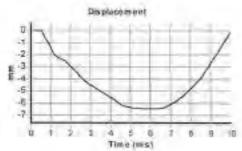
Comments / Location

CHARLWOODS









Calculations

Area of Rod A (mm2): 944
Thiranepical Energy E_{theor} (J): 473
Messured Energy E_{thross} (J): 384

Energy Ratio E, (%): 81

Signed: Neil Burrows

Title: Field Operations Manager

The recommended calibration interval is 12 months

SPT Hammer Energy Test Report

in accordance with 85EN ISO 22476-3:2005

Southern Testino	0
Unit 11	
Charlwood Road	
East Grinstead	
West Sussex	
DUISO DUIL	

SPT Harmmer Rel	MERCHALL
Test Date	23/09/2020
Report Date:	23/09/2020
Elia Garrier	11/10/09/9 see

Truck	Open	STATE .	NPB	ı
100	Pilmeria	#UD7	145.57	ř

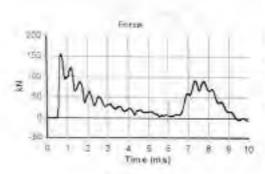
Instrumented Rod Data

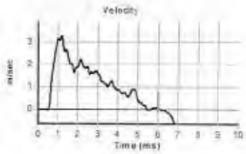
Drameter d. (mm):	54
Wall Thickness 1, (mm);	6.3
Assumed Modulus E., (GPa)	208
Accelerometer No.1:	6458
Accelerouseter No.2:	9607

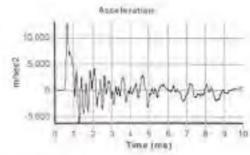
SPT Hammer Information

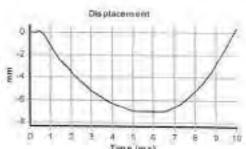
Hammer Mass in (kg):	63.5
Falling Height in (mm):	760
SIFT String Length L (m):	14.5

Comments / Location CHARLWOODS









Calculations

Area of Rad A (mm2):		944
Theoretical Energy E _{thers}	(3)	473
Measured Energy E _{melia}	(1):	384

Energy Ratio E, (%):

Styred: Stuart Simmonds

Title: Field Operations Technician

The recommended calibration interval is 12 months

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APPENDICES

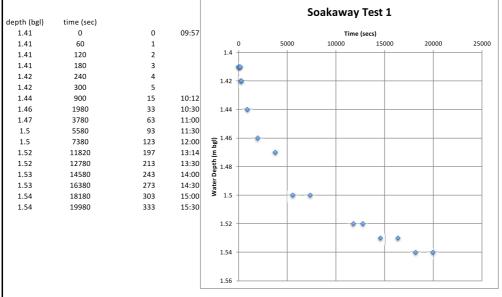
APPENDIX F SOAKAWAY INFILTRATION TEST WORKSHEETS

LMB GEOSOLUTIONS LTD

Soakaway Test (TP101)

(Based on BRE Digets 365: Soakaway Design)





Dimensions (m)

Depth 2.32 Top Length 1.6 Base Length 1.3 Width 0.6 Infiltration Test Calcualtion

Soil Infiltration Rate

f = Vp75 - 25 / ap50 x tp75 - 25 =

FAIL m/s

2.74 m²

i) Vp75 - 25

Applying and effective storage depth of =

1.6375 d25 =

d75 = Vp75 - 25 = 0.40 m³

2.0925

0.91

ii) ap50 ap50 =

iii) tp75 - 25

tp75 - 25 = N/A seconds

Notes

- 1. Invert Level assumed to be coincident with start depth of water.
- 2. Soil infiltration rate is calculated from the time taken for the water level to

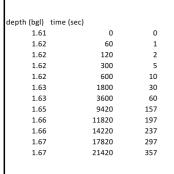
fall from 75% to 25% effective depth.

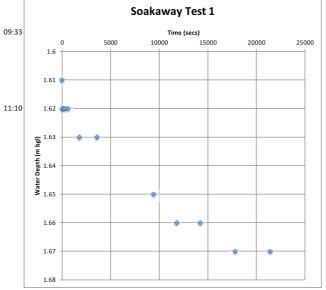
LMB GEOSOLUTIONS LTD

Soakaway Test (TP102)

(Based on BRE Digets 365: Soakaway Design)







Dimensions (m)

Depth 2.31 Top Length 1.7 Base Length 1.5 Width 0.6

Infiltration Test Calcualtion

Soil Infiltration Rate f = Vp75 - 25 / ap50 x tp75 - 25 =

FAIL m/s

i) Vp75 - 25

Vp75 - 25 =

Applying and effective storage depth of = d25 = d75 = 1.785 2.135

0.70

ii) ap50 ap50 =

iii) tp75 - 25

tp75 - 25 = N/A seconds

2.50 m²

Notes

0.34 m³

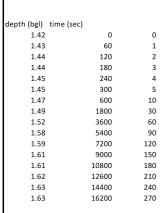
1. Invert Level assumed to be coincident with start depth of water. 2. Soil infiltration rate is calculated from the time taken for the water level to

fall from 75% to 25% effective depth.

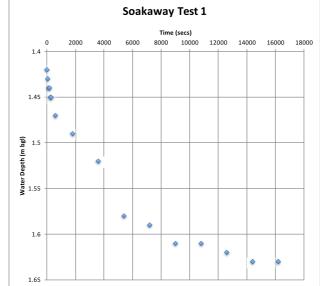
LMB GEOSOLUTIONS LTD

Soakaway Test (TP103)

(Based on BRE Digets 365: Soakaway Design)







Dimensions (m)

Depth 2.46 Top Length 1.7 Base Length 1.3 Width 0.6

i) Vp75 - 25

Applying and effective storage depth of =

1.68 d25 = d75 = 2.2 Vp75 - 25 = 0.47 m³ Infiltration Test Calcualtion

1.04

Soil Infiltration Rate f = Vp75 - 25 / ap50 x tp75 - 25 =

FAIL m/s

3.08 m²

ii) ap50

ap50 = iii) tp75 - 25

tp75 - 25 = N/A seconds

Notes

- Invert Level assumed to be coincident with start depth of water.
 Soil infiltration rate is calculated from the time taken for the water level to

fall from 75% to 25% effective depth.

APPENDICES

APPENDIX G GAS ANALYSER & PID CALIBRATION CERTIFICATES

TEST DATE:	NHEC	EXDITION	9			
Date	10/06/2020					
Atmospheric Press	ure -	998	mB			
Amblem Tempers		22.2	P.C			
Environtes Social	yn.	5089				

GFM436 Final Inspection & Calibration Check Certificate

Customer	LMB Geosolutions Ltd
Cartificate Number	121787
Order Number	325987

Sertal Number	12102
Software Version	G436-00.0027/0009

GAS DATA LTD Unit 4. Fairfield Court Seren State Estate Wheler Rd Coverity CV3 41. Tel 02476303311 Fan 02476307711

10/06/21

V	Instruit	pent Checks				
Keyboard	1	Display Contrast	*			
Pump Flow In	500 00 5200 (1700)	Pump Flow @ 200mB	300 Street and the sale			
Clock Set / Rounting	ock Set / Rauming		X			

			Gas Checks				
	CH .		CO 2		0,		
	Instrument Gas	True Gas	Instrument Gar	True Gas	Instrument Gas	True Gas	
	Readings %	Value %	alue % Readings % No		Readings %	Value Vi	
Sensor	60.2	60	39.5	40	20.9	20.9	
	Acception N.	.00	Accept part	70	Accept. 20, 1	20/7	
	5	5	5	-	6	6	
	Autog0 ±0,7		ACCOUNT #RIA		Accept 21/J	a a	
lero	0	0	0	- 0	0	0	
ending 00% N2			леди эпл		ACCEPT FALT		

			Optional Ga	s Checks					
Applied Gas & Range		Concentration Tested @		Instrument Readings (ppm)					
Gas Type	Range (ppm)	(ppm)	2	Instrument Gas Readi					
125	5000	1800	n	Acopress (1500	Armyrand			
CO	2000	1600	0-	Ameni ±0.0	1000	Acrept AEA			
Heyane	200	2.0%	0	Auregista.	1.99	-keeps 2.70.0			



Calibration Certificate

Calibration Date: 28/10/2020

Customer: LMB Geosolutions

Model: MiniRAE Lite (PGM-7350)

Serial Number: 595-002144 Date of Purchase: 11/07/2017

Date of Last In-Situ Calibration: 31/10/2019

Visual Inspection: New inlet plastic filter fitted.

Calibrated at 0 ppm Calibrated at 100 ppm

Method: Isobutelene Gas:- Batch No: WO229218-2



APPENDIX C – ASSESSMENT SCREENING TABLES

	Exceedances	
DWS	EQS	Both

			ation	BH101	BH102	BH103	BH104	BH105
Analyte	Units	DWS	ate EQS	05/05/2021	05/05/2021	05/05/2021	05/05/2021	05/05/2021
aH Rectrical Conductivity at 20 °C	pH µS/om µgl	(µg/l)	(µg/l)	6.9 780 21	7 810	6.8	7.5 450	8.1 350
Bestrical Conductivity at 20 °C Total Openide Complex Cyarride Free Cyarride Sulphate as SO4	fgų fgų fau	50	1	21 21 < 10	810 < 10 < 10 < 10 84700	6.8 680 < 10 < 10 < 10 158000 < 5.0 47 24	7.5 490 < 10 < 10 < 10 90600 < 5.0 23	8,1 350 < 10 < 10 < 10 14800
Chloride	fgų fgų fgm			21 < 10 121000 < 5.0 87 110	84700 < 5.0 90	156000 < 5.0 47	90600 < 5.0 23	14600 < 5.0 49
Ammoniacal Nitrogen as N Ammoniacal Nitrogen as NH3 Ammoniacal Nitrogen as NH4 Fotal Organic Carbon (TOC)	fgų fgų fgų fgm	500	15	110 130 140	47	24 29 30	680	800
Fotal Organic Carbon (TOC) Fotal Phenois Fotal Phenois (monohydric)				3.32	4.17	3.5	11.3	4.23
Speciated PAHs	Pgų			< 10	< 10	< 10	< 10	< 10
Naphthalene Noenaphthylene Noenaphthane	Fgy Fgy Fgy		2.4	< 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01
Nucrene Phenanthrene	fau fau fau fau		0.1	< 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01 < 0.01	< 0.01	< 0.01	< 0.01
Purcanhene Pyrene Benzo(a)anfiracine Chrysene Benzo(b)fluoranhene			0.1	< 0.01	< 0.01	< 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01 < 0.01 < 0.01 < 0.01 < 0.01 < 0.01
orrzouapamiracione Chrysene Sanzo(b)fluoranthene	fgų fgų fgų	0.1		< 0.01	< 0.01 < 0.01 < 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01 < 0.01	< 0.01
Benzo(a)pyrene	feu feu feu	0.1	0.01	< 0.01 < 0.01 < 0.01				< 0.01
Dbenz(a,h)anthracene Benzo(ghi)perylene	Fpu Fpu		0.05	< 0.01 < 0.01	< 0.01 < 0.01	< 0.01 < 0.01	< 0.01	< 0.01
Total PAH Total EPA-16 PAHs	Pgų			< 0.16	< 0.16	< 0.16	< 0.16	< 0.16
Heavy Metals / Metalloids Boron (dissolved) Calcium (dissolved) Chromium (hoxavalent)	fgų fgm	1000	2000	170 140	110 130	200 110	32 93 < 5.0 0.016	58 43
	fgu fgm fgm	200	3.4 1000	140 < 5.0 0.018 10 97.2	130 < 5.0 0.02 7.7	110 < 5.0 0.046 11	< 5.0 0.016 4.9	43 < 5.0 0.1 1.6 385
Aggresium (dissolved) Phosphorus (dissolved) Potisseium (dissolved) Sodium (dissolved)	fgu fgm fgm			97.2 12 63	7.7 40.3 13 83	11 208 15 55	4.9 51.9 4.8 22	385 5.4 57
Intimony (dissolved) Insenic (dissolved) Barium (dissolved)	I fqu	5 10 700	50	0.4			23 0.75	0.7 2.96
	feu feu feu	700	0.25	53 < 0.1 < 0.02	< 0.4 2.54 63 < 0.1 < 0.02	< 0.4 0.94 57 < 0.1 < 0.02	84 < 0.1 0.03	14 < 0.1 < 0.02
Cadmium (dissolved) Chromium (dissolved) Cobalt (dissolved)	μg٩	Ľ	3	0.7	1.5	1.1 2.8 1.3	0.3	0.7
Copper (dissolved) ,ead (dissolved) Aanganese (dissolved)	feu feu feu feu	2000 25 50	7.2	2.7 < 0.2 93	< 0.2 98	< 0.2	9.2 < 0.2 9.6 < 0.05	2.5 < 0.2 39 < 0.05
/fremsum (disselved) // Cebat (disselved)	feu feu feu	20	0.05	< 0.05 0.92 3.8	< 0.05 0.76 5.2	< 0.05 0.37 5.2	3.5	1.8
'in (dissolved) /anadium (dissolved)	Feu Feu Feu	10	25 20	0.26 0.9	< 0.20 4.6	1.9 < 0.20 0.6	< 0.6 0.72 1.9	< 0.6 < 0.20
linc (dissolved)	Pgų	3000	8	8.9	19	4.3	11	2.7
Monoaromatics & Oxygenates Jenzene Foluene Ethylbenoane	feu feu fou	700 300	10 50 20	< 1.0	<1.0	< 1.0	< 1.0 < 1.0	< 1.0
thybenzene i & m-xylene i-xylene ITBE (Methyl Tertiary Butyl Ether)	Pau Pau Pau	300	20	< 1.0 < 1.0 < 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0 <1.0 <1.0 <1.0	<1.0 <1.0 <1.0 <1.0 <1.0 <1.0	<1.0 <1.0 <1.0 <1.0 <1.0	<1.0 <1.0 <1.0 <1.0 <1.0 <1.0
Petroleum Hydrocarbons	Fgu			< 1.0			<1.0	
Vineral Oil (C10 - C40) Diesel Range Organics (C10 - C25)	Pgų Pgų			< 10.0	< 10.0 < 10	< 10.0 < 10	< 10.0 < 10	< 10.0 < 10
PH1 (C10 - C40) PH2 (C6 - C10)	Fau Fau			< 10	< 10	< 10	< 10	< 10
	fgu fgu	300	20	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
PH-OWG - Alighade 205 - 08 PH-OWG - Nighade 208 - 08 PH-OWG - Nighade 208 - 010 PH-OWG - Nighade 2010 - 012 PH-OWG - Nighade 2010 - 012 PH-OWG - Nighade 2010 - 012 PH-OWG - Nighade 2010 - 021 PH-OWG - Nighade (05 - 031) PH-OWG - Nighade (05 - 035)	feu feu	300 300 300	20	< 1.0 < 1.0 < 10	< 1.0 < 1.0 < 10	< 1.0 < 1.0 < 10	< 1.0 < 1.0 < 10	< 1.0 < 1.0 < 10
PH-CWG - Aliphatic >C12 - C16 PH-CWG - Aliphatic >C16 - C21 PH-CWG - Aliphatic >C21 - C35	fau fau fau fau	300	20	< 10 < 10 < 10 < 10	< 10 < 10 < 10	< 10 × 10 < 10 < 10	< 10 < 10 < 10 < 10	< 10 < 10 < 10 < 10
PH-CWG - Aliphatic (C5 - C35) PH-CWG - Aromatic >C5 - C7	PΩN				< 1.0			
PH-CWG - Acomatic >C5 - C7 PH-CWG - Acomatic >C5 - C7 PH-CWG - Acomatic >C7 - C8 PH-CWG - Acomatic >C8 - C10 PH-CWG - Acomatic >C10 - C12 PH-CWG - Acomatic >C10 - C12 PH-CWG - Acomatic >C10 - C21 PH-CWG - Acomatic >C10 - C21 PH-CWG - Acomatic >C10 - C31 PH-CWG - Acomatic >C31 - C35 PH-CWG - Acomatic >C31 - C35	Feu Feu Feu	700 300 100 100	50 20 20	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 10	< 1.0 < 1.0 < 1.0 < 10
PH-CWG - Aromatic >C12 - C16 PH-CWG - Aromatic >C16 - C21 PH-CWG - Aromatic >C21 - C35	fgų fgų fau	90 90	20 20 20 20	< 10 < 10 < 10 < 10	< 10 < 10 < 10 < 10	< 10 < 10 < 10 < 10	< 10 < 10 < 10 < 10	< 10 < 10 < 10 < 10
PH-CWG - Aromatic (C5 - C35)	Pgų			< 10	< 10	< 10	< 10	< 10
NOCs Ottoromethane Ottoroethane Romomethane	Fgu Fgu			< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0
richlorofluoromethane	Feu Feu Feu	0.5		< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
,1-Dichloroethene ,1,2-Trichloro-1,2,2-trifluoroethene Dis-1,2-dichloroethene	fgų fgų fgų			< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
ATBE (Methyl Tertiary Butyl Ether) ,1-Dichloroethene 1.2-Dichloropropane	feu feu feu feu			< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0 <1.0	< 1.0 < 1.0 < 1.0 < 1.0
1,32: Inchicro-1,22-influorceltrans 36:1,2-dishforoethere MTBE (Methyl Terliary Butyl Ether) 1,Dishforoethere 1,2-Dishforoethere 1,2-Dishforoethere 1,2-Dishforoethere 1,Dishforoethere	fgų fgų fgų	200	100	< 1.0 < 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	<1.0 <1.0	< 1.0 < 1.0 < 1.0
rans-1,2-dichloroethene	feu feu feu	ļ.,	10	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
	Lou		10	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
2-Dichloropropane Fichloropthane Doromomethane Romodichloromethane Sa-1,3-dichloropropane	leu leu leu leu			< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0 <1.0
Foluene	fqu fqu fqu	700	50	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
1,1,2-Trichloroethere 1,3-Dichloropropane Dipromochloromethere	FQU FQU FQU	5	50 400	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
etrachloroethene _2-Dibromoethane	feu feu feu feu	10 0.4 100		< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
Chloroberosine 1,11.2 Tetrachloroethane Ethylberozene & m-Xylane Syrene		300	20	< 1.0	< 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0	<1.0 <1.0
	Pgų Pgų Pgų	20	50	< 1.0	< 1.0	< 1.0	<1.0	<1.0
-Xylene .1,2,2-Tetrachloroethane sopropylbenzene	fgų fgų fgų			< 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
iconoberosne i-Propylbenzene - Chlorotoluene - Chlorotoluene - 3,5-17 firmethylbenzene	fau fau fau fau			< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
-Chlorotoluene ,3,5-Trimethylbenzene ert-Butylbenzene	feu feu feu			< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0	< 1.0 < 1.0 < 1.0
2.4-Trimethylbenzene ec-Butylbenzene 3-Dichlorobenzene	fgu fgu fgu			< 1.0 < 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0 <1.0	<1.0 <1.0 <1.0 <1.0	< 1.0 < 1.0 < 1.0	<1.0 <1.0 <1.0
-Isopropyltoluene ,2-Dichlorobenzene	feu feu		600 80	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0
Sutylbenzene 2-Ditromp-3-chioropropane	fau fau fau fau		00	< 1.0 < 1.0	< 1.0	×10	<1.0 <1.0 <1.0 <1.0 <1.0	<1.0 <1.0 <1.0 <1.0 <1.0
2-Ottomores 2-Ottomores 2-Ottomores-Artinopropine 2-4-Trichlorobenzene texachlorobutadiene 2-3-Trichlorobenzene	feu feu feu			< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0
SVOCs	Pau						< 0.06	
Vniline Phenol I-Chlorophenol Sis(2-chloroethyl)ether	fou.	7.7		< 0.06 < 0.06 < 0.06 < 0.06	< 0.06 < 0.06 < 0.06 < 0.06	< 0.05 < 0.05 < 0.05 < 0.06	< 0.05 < 0.05 < 0.05 < 0.06	< 0.06 < 0.06 < 0.06 < 0.06
is(2-chloroeth)letter 3-Dichlorobenzene 2-Dichlorobenzene 4-Dichlorobenzene	feu feu feu feu			< 0.05	< 0.05 < 0.05 < 0.05 < 0.05		< 0.05 < 0.05 < 0.05 < 0.05	
.4-Dichlorobenzene isi(2-chloroisopropyl)ether -Methylphenol lexachloroethane	Fgu Fgu			< 0.06 < 0.06 < 0.06 < 0.06	< 0.05 < 0.05 < 0.06	< 0.05 < 0.05 < 0.05 < 0.05 < 0.05	< 0.05	< 0.05 < 0.05 < 0.05 < 0.05 < 0.06
-Methylphenol	fgu fgu fgu			< 0.05 < 0.05 < 0.06 < 0.06	< 0.05 < 0.05 < 0.06 < 0.06	< 0.05 < 0.05 < 0.06 < 0.06	< 0.05 < 0.05 < 0.06 < 0.05	< 0.05 < 0.05 < 0.06 < 0.06
ophorone -Ntrophenol ,4-Dimethylphenol	fgu fau			< 0.05	< 0.05	< 0.05	< 0.05	< 0.06
A-Dimetryphenol is(2-chlorosthoxymethane 2,4-Trichlorobenzene laphthalene A-Dichlorophenol	feu feu feu feu	2.4		< 0.05 < 0.05 < 0.01 < 0.05	< 0.06	< 0.06	< 0.05 < 0.05 < 0.01 < 0.05	< 0.06
A-Dichlorophenol -Chlorophine lexachlorobutadiene	Igu Igu Igu			< 0.05 < 0.05 < 0.05	< 0.06 < 0.01 < 0.05 < 0.06 < 0.06 < 0.05 < 0.06 < 0.06 < 0.06	< 0.06 < 0.01 < 0.05 < 0.06 < 0.06 < 0.06 < 0.06 < 0.06 < 0.06 < 0.06	< 0.05 < 0.06 < 0.06	< 0.06 < 0.01 < 0.06 < 0.06 < 0.06 < 0.06 < 0.06 < 0.06 < 0.06
-Chloro-3-methylphenol .4,6-Trichlorophenol .4,5-Trichlorophenol	feu feu feu			< 0.05 < 0.05 < 0.05	< 0.05 < 0.05 < 0.05	< 0.05 < 0.05 < 0.05	< 0.06 < 0.06 < 0.06	< 0.05 < 0.06 < 0.06
Mathdrarithalana	Lou			< 0.05	< 0.05	< 0.05	< 0.06 < 0.05	< 0.06
-Chloronaphthalene limetrylphthalate .8-Diritrotoluene conaphthylene	Feu Feu Feu Feu			< 0.05 < 0.05 < 0.01	< 0.05 < 0.05 < 0.01 < 0.01 < 0.06 < 0.06 < 0.06	< 0.05	< 0.05 < 0.06 < 0.01	< 0.05 < 0.01
.4-Dinitrotoluene liberaofuran	feu feu feu			< 0.01 < 0.05 < 0.05	< 0.01 < 0.06 < 0.05	< 0.06 < 0.01 < 0.01 < 0.06 < 0.06	< 0.06	< 0.06 < 0.01 < 0.01 < 0.06 < 0.06
-Chlorophenyl phenyl ether liethyl phthalate	Feu Feu Feu			< 0.05 < 0.05 < 0.05	< 0.05	< 0.05	< 0.06 < 0.06 < 0.05	< 0.06
-Nitroaniline lucrene zobenzene romophenyl phenyl ether	μg٩			< 0.01 < 0.05 < 0.05	< 0.01	< 0.01	< 0.01	< 0.01
lucrene scobenzene scobenzene scomophenyl phenyl ether lesstrikorbenzene herardhirene nithrasiene arbazole schotzole schotzole schotzole schotzole schotzole schotzole	feu feu feu feu feu		0.1	< 0.05 < 0.01 < 0.01	< 0.05 < 0.05 < 0.01 < 0.01	< 0.05 < 0.05 < 0.01 < 0.01	< 0.05 < 0.05 < 0.01 < 0.01	< 0.06 < 0.06 < 0.01 < 0.01 < 0.06 < 0.06 < 0.06 < 0.06
arbazole ibuty phthalate	Pau Pau Pau		0.1	< 0.05 < 0.05	< 0.01 < 0.05 < 0.06 < 0.06 < 0.01	< 0.01 < 0.05 < 0.06 < 0.06 < 0.01	< 0.05	< 0.06 < 0.06
luoranthene yrene	Pgų Pgų Pgų		0.1	< 0.05 < 0.01 < 0.01			< 0.05 < 0.01 < 0.01	< 0.01
lutyl benzyl phthalate	fau fau fau fau		0.05	< 0.05 < 0.01 < 0.01	< 0.06 < 0.01 < 0.01	< 0.05 < 0.01 < 0.01	< 0.06 < 0.01 < 0.01	< 0.06 < 0.01 < 0.01
hysene	feu feu feu	0.1 0.1 0.01	0.03 0.03 0.05 0.002 0.05	< 0.01	< 0.01	< 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01 < 0.01 < 0.01
thrysene enzo(b)fluoranthene enzo(s)fluoranthene enzo(s)pyrene	1gu	0.01 0.1	0.002 0.05 0.002	< 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01 < 0.01 < 0.01	< 0.01 < 0.01 < 0.01
lenzo(b)flucrenthene senzo(s)(hucranthene senzo(s)pyrane ndeno(1,2,3-cd)pyrane Dibero(a,h)arrthracene	µg1				< 0.10	< 0.10	< 0.10	< 0.10
Serzo(a)anthracene Trysaina Enros(a)fucranthene Serzo(a)fucranthene Serzo(a)fucranthene Serzo(a)fucranthene Index(a)fucranthene Index(a)fucranthene Serzo(a)fucrante Serzo(a)fucrante Serzo(a)fucrante Serzo(a)fucrante Serzo(a)fucrante Serzo(a)fucrante Serzo(a)fucrante Serzo(a)fucrante	Feu Feu Feu	0.1		< 0.10	-0.10			
leraco/procenthene Beraco(a) function there Be	Feu Feu Feu	0.1		< 0.10				
leraco/procenthene Beraco(a) function there Be	Teu Teu Teu Teu Teu Teu Teu Teu	0.1		< 0.10	< 0.02	:	:	-
enscolphuscambene enscolphuscambene enscolphuscame obeno (1,2,3-cd)pyrene ebero(a, h)archina cine enscolphipsiytene S4-Methylphenol	Feu Feu Feu Feu Feu Feu Feu	0.1		< 0.10			-	

Human Health Screening Table Residential Without Plant Uptake GAC (SOM 1%)								Exceedance	l .	
		Locatio	n BH101 m bgl) 0.80	BH102 0.50	BH103 0.20	BH104 1.80	BH105	TP101 0.80	TP102 0.40	TP10
Analyte Asbestos in Soil Screen / Identification Name Asbestos in Soil	Unit Type Type	GAC (mg/kg) Depth (m bgl) 0.80		0.20 - Not-detected	-	0.50	-	Chrysotile Detected	0.5 Chryso
Asbestos Quantification (Stage 2) Asbestos Quantification Total	%				-	-	-	-	0.005	0.006 0.006
General Inorganics										<u> </u>
pH - Automated Total Cyanide	pH Units mg/kg	34 (free)	7.9 < 1.0	10.3	7.7 < 1.0	7.9 < 1.0	7.7	7.4 120	10.1	9.5 < 1.0 1500
Total Sulphate as SO4 Water Soluble SO4 16hr extraction (2:1 Leachate Equivalent)	mg/kg g1		3800 0.095	0.67	630 0.022	240 0.096	4000 0.46	1000 0.46	1000 0.15	0.17
Sulphide Organic Matter Total Organic Carbon (TOC)	mg/kg %		2.8	17 - 0.5	21 - 0.1	3.2 - 0.2	13 1.1 0.6	1.8	17 - 0.2	0.2
	70		-	0.5	0.1	0.2	0.6	1.1	0.2	0.2
Total Phenois Total Phenois (monohydric)	mg/kg		< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Speciated PAHs Naphthalene	mg/kg	2.3	< 0.06	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05	< 0.00
Acenaphthylene Acenaphthene	mg/kg mg/kg	2900 3000	< 0.05 < 0.06	< 0.05 < 0.05	< 0.05 < 0.05	< 0.05 < 0.05	< 0.05 < 0.05	< 0.05 < 0.05	< 0.05 < 0.05	< 0.00
Fluorene Phenanthrene	mg/kg mg/kg	2800	< 0.05				< 0.05 < 0.05		< 0.05 < 0.05	
Anthracene Fluoranthene	mg/kg mg/kg	1300 31000 1500	0.46 4.6	1.7 0.44 2.6	< 0.05 < 0.05 0.35	0.59 0.27 1.9	< 0.05 0.49	< 0.05 < 0.05 < 0.06	< 0.05	< 0.0 < 0.0 0.39
Pyrene Renzo(a)anthracene	mg/kg mg/kg	3700 11	4.3 2.6	2.8	0.39	1.9	0.74	< 0.05 < 0.05	< 0.05 < 0.05	0.42
Chrysene Benzo(b)fluoranthene	mg/kg mg/kg	30 3.9	3 3.3	1.5	< 0.05 < 0.05	1.2	< 0.05 < 0.05	< 0.05 < 0.05	< 0.05 < 0.05	0.33
Benzo(k)fluoranthene Benzo(a)pyrene	mg/kg mg/kg	110 3.2 45	2.1 3.2	0.55	< 0.05 < 0.05 < 0.05	0.72 1.1 0.6	< 0.05 < 0.05 < 0.05	< 0.05 < 0.06 < 0.05	< 0.05 < 0.05 < 0.05	0.27 0.46 0.35
Indeno(1,2,3-cd)pyrene Dibenz(a,h)anthracene	mg/kg mg/kg	0.31	1.6 0.54	0.67	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05	< 0.0
Benzo(ghi)perylene	mg/kg	360	2	0.71	< 0.05	0.72	< 0.05	< 0.05	< 0.05	0.4
Total PAH Speciated Total EPA-16 PAHs	mg/kg		29.7	15.1	< 0.80	10.8	1.23	< 0.80	< 0.80	3.47
Heavy Metals / Metalloids									—	
Arsenic (aqua regla extractable) Boron (water soluble)	mg/kg mg/kg mg/kg	40 11000	65 1.3	2.6	0.3	< 0.2	9.9	1.8	0.5	0.2
Cadmium (aqua regia extractable) Chromium (hexavalent)	mg/kg	85 6	< 4.0	0.4 < 4.0	< 0.2	< 0.2	< 0.2	< 0.2	< 0.2	< 0.2 < 4.0
Chromium (aqua regia extractable) Copper (aqua regia extractable)	mg/kg mg/kg	910 7100	20 110	25 37	11	27 15	22	28 62	19 35	9.4
Lead (aqua regia extractable) Mercury (aqua regia extractable)	mg/kg mg/kg	310 15 180	1.3 33	120 0.3 19	58 < 0.3	20 < 0.3	40 < 0.3	270 < 0.3	140	52 < 0.3
Nickel (aqua regia extractable) Selenium (aqua regia extractable)	mg/kg mg/kg	430	< 1.0	< 1.0	9.9 3.6	2.2	15 < 1.0	2.9	17 < 1.0	< 1.0
Zinc (aqua regia extractable)	mg/kg	40000	310	130	140	63	68	130	150	96
Monoaromatics & Oxygenates Benzene	μg/kg		< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Toluene Ethylbenzene	µg/kg µg/kg		< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0 < 1.0 < 1.0	< 1.0	< 1.0 < 1.0 < 1.0
p & m-xylene n-xylene	µg/kg µg/kg		< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
MTBE (Methyl Tertiary Butyl Ether)	µg/kg		< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Petroleum Hydrocarbons TPH C10 - C40	mg/kg		74	220	< 10	100	470	49	< 10	190
TPH-CWG - Aliphatic >EC5 - EC6	mg/kg	42 100	< 0.001	< 0.001	< 0.001	< 0.001		< 0.001	< 0.001	< 0.00
TPH-CWG - Aliphatic >EC6 - EC8 TPH-CWG - Aliphatic >EC8 - EC10	mg/kg mg/kg	271	< 0.001	< 0.001	< 0.001	< 0.001	< 0.001 < 0.001 < 0.001	< 0.001	< 0.001	< 0.00
TPH-CWG - Aliphatic >EC10 - EC12 TPH-CWG - Aliphatic >EC12 - EC18	mg/kg mg/kg	130 1100	< 1.0 < 2.0	1.1 5.9	< 1.0	< 1.0 2.6	< 1.0	< 1.0 13	< 1.0 < 2.0	< 1.0
TPH-CWG - Aliphatic >EC16 - EC21 TPH-CWG - Aliphatic >EC21 - EC35	mg/kg mg/kg		< 8.0 < 8.0	16 62	< 8.0 < 8.0	< 8.0 41	35 250	18 18	< 8.0 < 8.0	18 91
TPH-CWG - Aliphatic (EC5 - EC35)	mg/kg		< 10	85	< 10	49	300	49	< 10	110
TPH-CWG - Aromatic >EC5 - EC7 TPH-CWG - Aromatic >EC7 - EC8	mg/kg mg/kg mg/kg	370 860	< 0.001	< 0.001	< 0.001 < 0.001	< 0.001 < 0.001	< 0.001 < 0.001	< 0.001 < 0.001	< 0.001 < 0.001	< 0.00
TPH-CWG - Aromatic >EC8 - EC10 TPH-CWG - Aromatic >EC10 - EC12	mg/kg	47 250	< 0.001 7.1	< 0.001	< 0.001	< 0.001 < 1.0	< 0.001 < 1.0	< 0.001 < 1.0	< 0.001 < 1.0	< 0.00
TPH-CWG - Aromatic >EC12 - EC16 TPH-CWG - Aromatic >EC16 - EC21	mg/kg mg/kg	1800 1900	12 19	7 24	< 2.0 < 10	< 2.0	5.7 24	< 2.0	< 2.0	< 2.1
TPH-CWG - Aromatic >EC21 - EC35 TPH-CWG - Aromatic (EC5 - EC35)	mg/kg mg/kg	1900	36 74	62 93	< 10 < 10	32 46	85 110	< 10 < 10	< 10 < 10	34 37
VOCs				_						
Chloromethane Chloroethane	µg/kg µg/kg		-	< 1.0 < 1.0	- :	- :	- :	< 1.0 < 1.0	-:-	
Bromomethane vinyl Chloride	µg/kg µg/kg		-	< 1.0 < 1.0	- :	- :	- :	< 1.0 < 1.0	-:-	
Trichlorofluoromethane 1,1-Dichloroethene 1,1,2-Trichloro 1,2,2-Trifluoroethane	µg/kg µg/kg			< 1.0	-	-	- 1	< 1.0	-:-	
Cis-1,2-dichloroethene	µg/kg µg/kg		-	< 1.0	-	- :	- :	< 1.0	-:-	
MTBE (Methyl Tertiary Butyl Ether) 1,1-Dichloroethane	µg/kg µg/kg		-	< 1.0 < 1.0	-	- :	- :	< 1.0 < 1.0	-:-	
2,2-Dichloropropene Trichloromethane	µg/kg µg/kg		-	< 1.0 < 1.0				< 1.0 < 1.0		
1,1,1-Trichloroethane 1,2-Dichloroethane	µg/kg		-	< 1.0 < 1.0 < 1.0	-		-	< 1.0 < 1.0 < 1.0	<u> </u>	
Trans-1,2-dichloroethene	µg/kg µg/kg		-	< 1.0	- :		- :	< 1.0	-	
Benzene Tetrachloromethane	µg/kg µg/kg		-	< 1.0	- :		- :	< 1.0	-	
1,2-Dichloropropene Trichloroethene	µg/kg µg/kg		-	< 1.0 < 1.0	- :		- :	< 1.0 < 1.0	-	
Dibromomethane Bromodichloromethane	µg/kg µg/kg		-	< 1.0	- :		- :	< 1.0	-	
Cis-1,3-dichloropropene Trans-1,3-dichloropropene	µg/kg µg/kg			< 1.0 < 1.0 < 1.0	- :	-	- :	< 1.0 < 1.0 < 1.0	-:-	
Toluene 1.1.2-Trichloroethane	µg/kg µg/kg			< 1.0	- :	-	- :	< 1.0 < 1.0	-:-	
1,3-Dichloropropene Dibromochloromethane	µg/kg µg/kg			< 1.0 < 1.0	- :	-	- :	< 1.0 < 1.0	-:-	
Tetrachloroethene 1,2-Dibromoethane	µg/kg µg/kg			< 1.0 < 1.0	- :	-	- :	< 1.0 < 1.0	-:-	
Chlorobenzene 1.1.1.2-Tetrachloroethane	µg/kg µg/kg		-	< 1.0	- :	-	- :	< 1.0	-	
Ethylbenzene p & m-Xylene	µg/kg µg/kg		-	< 1.0	- :		- :	< 1.0	-	
Styrene Tribromomethane	µg/kg µg/kg		-	< 1.0 < 1.0	- :		- :	< 1.0 < 1.0	-	
5-Xylene 1,1,2,2-Tetrachloroethane	µg/kg µg/kg			< 1.0 < 1.0	-	- 1	- 1	< 1.0 < 1.0 < 1.0	- :	
Isopropytoenzene Bromobenzene	µg/kg µg/kg			< 1.0	-	- 1	- 1	< 1.0	- :	
n-Propylbenzene 2-Chlorotoluene	µg/kg µg/kg			< 1.0 < 1.0	-	- 1	- 1	< 1.0 < 1.0	-:-	
4-Chlorotoluene 1,3,5-Trimethylbenzene	µg/kg µg/kg			< 1.0	-	- 1	- 1	< 1.0 < 1.0	- :	
tert-Butylbenzene	µg/kg µg/kg			< 1.0	-	- 1	- 1	< 1.0	- :	
1,2,4-Trimethylbenzene sec-Butylbenzene 1,3-Dichlorobenzene	µg/kg µg/kg			< 1.0				< 1.0	==	
p-Isopropytoluene 1.2-Dichlorobenzene	µg/kg µg/kg			< 1.0 < 1.0				< 1.0 < 1.0	==	
1,4-Dichlorobenzene Butylbenzene	µg/kg µg/kg			< 1.0 < 1.0				< 1.0 < 1.0		Ħ
1,2,4-Trichlorobenzene	µg/kg µg/kg			< 1.0	- :		-:-	< 1.0	==	
Hexachlorobutadiene 1,2,3-Trichlorobenzene	µg/kg µg/kg			< 1.0 < 1.0	- :		-:-	< 1.0 < 1.0	==	
SVOCs										
Aniline Phenol	mg/kg mg/kg		-	< 0.1	- :	- :	-:-	< 0.1	<u> </u>	
2-Chlorophenol Bis(2-chloroethyl)ether	mg/kg mg/kg			< 0.1	-	-	- :	< 0.1	-	
1,3-Dichlorobenzene	mg/kg mg/kg	0.44		< 0.2	- :	-	-	0.5 < 0.1	1	
1.4-Dichlorobenzene Bis(2-chloroisopropyl)ether 2-Methylohenol	mg/kg mg/kg			< 0.2 < 0.1 < 0.3	-	-	-	< 0.2 < 0.1 < 0.3	Ė	
Z-Methylphenol Hexachloroethane Nitrobenzene	mg/kg mg/kg mg/kg			< 0.3 < 0.05 < 0.3	-	-	-	< 0.3 < 0.05 < 0.3	Ė	
4-Methylphenol	mg/kg		-	< 0.2	- :	-	-	< 0.2	Ė	
sophorone 2-Nitrophenol 2,4-Dimethylphenol	mg/kg mg/kg mg/kg			< 0.2 < 0.3 < 0.3			-	< 0.2 < 0.3 < 0.3	=	
Bis(2-chloroethoxy)methane	mg/kg mg/kg			< 0.3	-		Ė	< 0.3	Ė	
Naphthalene 2.4-Dichlorophenol	mg/kg mg/kg			< 0.3 < 0.05 < 0.3	-		Ė	< 0.05 < 0.3	Ė	
4-Chlorophenoi 4-Chlorophiline Hexachlorobutadiene	mg/kg mg/kg			< 0.1	-		Ė	< 0.1 < 0.1	Ė	
4-Chloro-3-methylphenol 2,4,6-Trichlorophenol	mg/kg mg/kg			< 0.1	-	-	-	< 0.1	=	
2.4.5-Trichlorophenol 2.4.6-Trichlorophenol 2-Methylnaphthalene	mg/kg mg/kg				-	-	-	< 0.2	=	
Metrymaphmateria 2-Chiloronaphthalene Dimethylohthalate	mg/kg mg/kg			< 0.1 < 0.1 < 0.1	-	-	-	< 0.1	=	
,6-Dinitrotoluene scenephthylene	mg/kg mg/kg	2900	-	< 0.1		- :		< 0.1	-	
Acenaphthene 2.4-Dinitrotoluene	mg/kg mg/kg	3000		< 0.05				< 0.05	E	E
Obenzofuran -Chlorophenyl ohenyl ether	mg/kg mg/kg			< 0.2				< 0.2	E	E
histhyl phthalate -Nitroaniline	mg/kg mg/kg	1800	-	< 0.3 < 0.2 < 0.2		- :		< 0.3 < 0.2 < 0.2	-	
Lucrene Lucrene	mg/kg mg/kg	2800	-	< 0.05		- :		< 0.05	-	
romaphenyl phenyl ether lexachlorobenzene	mg/kg mg/kg			< 0.2	- :	- :	-:	< 0.2	-:-	
Phenanthrene unthracene	mg/kg mg/kg	1300 31000		0.44	-	-	-	< 0.05	-	
Carbazole Dibutyl phthalate	mg/kg mg/kg	450		< 0.3	-	-	-	< 0.3	-	
unthraquinone Nuoranthene	mg/kg mg/kg	1500		< 0.2	-			< 0.2	Ė	
Pyrene	mg/kg	3700		2.6 2.8 < 0.3	-	-	-	< 0.05	=	
Butyl benzyl phthalate Benzo(a)anthracene	mg/kg mg/kg	42000 11 30		< 0.3 1.5 1.5	-	-	-	< 0.3 < 0.05	Ė	
Chrysene Benzo(b)fluoranthene	mg/kg mg/kg	3.9	-	1.4	-	-	-	< 0.05 < 0.05 < 0.05	Ė	
	mg/kg mg/kg	110 3.2 45		0.55	- :	-	-	< 0.05	<u> </u>	
3enzo(k)fluoranthene 3enzo(a)pyrene	mg/kg mg/kg	0.31		0.67	- :	-	-	< 0.05	<u> </u>	
Jenzo(k)fluoranthene Benzo(a)pyrene ndeno(1,2,3-od)pyrene Diberuz(a,1)anthracene		360	<u> </u>	0.71	-		Ė.	< 0.05	Ė	
Benzo(s)/fluoranthene Benzo(s)/prene Indeno(1,2,3-od)pyrene Dibenz(s,b)/enthracene Benzo(ghi/penytene	mg/kg			1						_
Berapik/Jucrambine Berapik/Jucrambine Berapik/Jucrambine Indanch (2,3-odi)prane Diberatia hijanthracene Berapik/Jucrambine Berapik/Jucrambine PCBs by GC-MS PCB Congener 28	mg/kg mg/kg				-	-	<u> </u>	< 0.001	<u> </u>	
Berack/fuverwihme Berack/fuverwihme Berack/fuverme Indianct (1,2-dd)preme Doerruic hjamitrocene Berack/fuparylene Doerruic hjamitrocene Berack/fuparylene PCB bry GG-4MS PCB Congener 28	mg/kg mg/kg mg/kg							< 0.001		
Serool Murambras Berool Murambras	mg/kg mg/kg mg/kg mg/kg mg/kg				-			< 0.001 < 0.001 < 0.001 < 0.001		
Sercol, Muranthewa Bread (2, 2), - (2	mg/kg mg/kg mg/kg mg/kg		-			-		< 0.001 < 0.001 < 0.001		
Bezool (Nuorathwas Bezool) (Nuorathwas) (Nuorathwas	mgkg mgkg mgkg mgkg mgkg mgkg		-	-	-	-		< 0.001 < 0.001 < 0.001 < 0.001 < 0.001		



Aberdeen
Aberdeen Westhill
Birmingham
Bristol
Dundee
Edinburgh
Elgin
Glasgow
Huddersfield

Inverness
Leeds
London
Newcastle upon Tyne
Plymouth
Sevenoaks
Taunton
Thurso
Watford





Manor Road, Richmond Geo-Environmental Site Investigation Report

Taylor Wimpey West London Limited



Status: Information Project No.: 21031

Revision: 0 Date: November 2022

Tel: 020 8390 9097



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- 1.0 Introduction
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- 9.0 Remedial Action Plan

Appendices

- APPENDIX A Boreholes and Trial Pit Records
- APPENDIX B Standpipe Record
- APPENDIX C Laboratory Rest Results
- APPENDIX D Original Testing Certificates
- APPENDIX E Figures

Tel: 020 8390 9097



1.0 Introduction

- a. This report has been prepared by Manhire Associates Geo-Environmental Ltd to assess the environmental risks and factors associated with the soils and groundwater encountered on the site following a programme of sampling and contaminant testing undertaken (Phase 2) at the Homebase site, located to the west of Manor Road, Richmond Upon Thames. This report supplements a previous investigation by Fairhurst [1] and should be read in conjunction with it.
- b. Planning condition 25 requires that "Development shall not commence until a strategy to deal with the potential risks associated with any contamination of the site has been submitted to, and approved in writing by, the Local Planning Authority"
- c. A Phase 1 Geo-Environmental and Geotechnical Preliminary Risk Assessment and a Phase 2 Ground Investigation Report has previously been prepared by Fairhurst as part of the Amended Application. Their Phase I Report identified potential sources of contamination on-site and a moderate risk was identified typically. It recommended that a ground investigations be undertaken to further quantify potential risks. The Phase 2 investigation undertaken by Fairhurst in July 2021 found low levels of contamination in 2 trial pits and detected asbestos in 2 samples. The report recommended that in areas of proposed soft landscaping, the placement of an engineered capping layer formed from imported, chemically 'clean' soils be installed and noted that on completion of these remedial works it would be considered that the risks to future site users would be considered Low. Ground water samples analysed were largely compliant with UKDWS with limited exceedances in 4 boreholes but were considered Low Risk to the shallow groundwater within the Kempton Park Gravel Member. Assessment of the ground gas indicated Very Low Risk for the site. The report recommended a second round of ground investigations to include boreholes, window sampling groundwater monitoring and geo-environmental and geo-technical sampling and testing to supplement the existing data.
- d. The purpose of this additional Phase 2 investigation and sampling was to determine the following:
 - Provision of sufficient geotechnical data to provide design parameters for the economic design of the foundations, earthworks and temporary works.
 - Assessment of problems and constraints associated with the construction of the works arising from the soil or ground water conditions, including temporary works, excavation, extraction and drainage.
 - Consideration of changes in the stability, drainage or other geotechnical aspects of the site and the surrounding ground and buildings which may arise from the works.
 - Assessment of the ground water regime.
 - Sampling and testing for an environmental risk assessment in accordance with CLEA guidelines, and the Environmental Protection Act 1990.

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- Pile design assessment of pile design parameters and assessment of various capacities for various pile diameters and recommendations for factors of safety.
- e. The intrusive site investigation consisted of three boreholes to a depth of 30m and a series of six continuous dynamic windowless sampler holes were carried out, supported by a programme of laboratory testing.
 - The ground conditions revealed by the investigation comprised Made Ground overlying Kempton Park Gravel with London Clay at depth.
 - Contamination testing found low levels of contamination, which were below the relevant trigger concentrations, however chemical analysis found some contamination above trigger levels in 3 isolated locations which will require limited remediation.
 - Standpipe readings found groundwater varied across the site with readings indicating groundwater at some 2.5m depth below ground level in BH1 and some 4.2m below ground level in BH2 & BH3.
 - The groundwater in BH3 was recorded at 2.36m AOD which is below the proposed basement excavation level and thus groundwater control will not be required.
- f. With regards to potential remediation of soil contaminated with PAH. metals or asbestos, it is considered that where in areas of hardstanding or below building footprints, contaminants can remain in situ, as the pathway in a source-pathway-receptor model will be blocked. Where they coincide with areas of soft landscaping, remediation will be necessary. A 450mm thick cap should be utilised, comprising imported clean soils with a capillary break layer at the base underlain by a geotextile barrier. This is to block the pathway in a source-pathway-receptor model and to discourage excavation below that level.
- g. **Planning Condition 26** requires that "Prior to occupation, a verification report demonstrating the completion of works set out in the approved remediation strategy and the effectiveness of the remediation shall be submitted to, and approved in writing, by the Local Planning Authority".
- h. **Planning Condition 27** requires that "If, during development, contamination not previously identified is found to be present at the site then no further development (unless otherwise agreed in writing with the Local Planning Authority) shall be carried out until a remediation strategy detailing how this contamination will be dealt with has been submitted to and approved in writing by the Local Planning Authority".
- i. This Report also describes measures being adopted on site so that any unexpected contamination identified on site is reported and details of what the post remediation verification report needs to include.
- j. This Report is prepared for the use of Taylor Wimpy West London in connection with the development. The report is not intended for, and should not be relied upon by any third party, and no responsibility is undertaken to any third party, without the express written agreement of Manhire Associates Geo-Environmental Ltd.

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2.0 Site description

The site is a triangular plot of land currently occupied by Homebase and Pets at Home retail outlets with associated car parking and yard area in Richmond upon Thames. Manor Road forms the southern boundary in the northern and western are marked by railway lines. To the east of the building is car parking whilst the yard is to the north. An Autoglass repair centre and "We Buy Any Car" franchises are located within the main car park area.

Figure 1 of Appendix E illustrates the general layout of the site and its immediate environs as they presently stand.

3.0 Development proposals

It is intended to construct a series of multi-storey blocks on the site with a small basement located under Block A measuring 31m x 6m extending approximately 3m below the existing ground level.

The proposed ground floor layouts are provided at Figure 2 of Appendix E.

4.0 Geology

Published records of the British Geological Survey indicate the site to lie Kempton Park Gravel overlying London Clay at depth.

5.0 Field work

The extent of the field work was specified by the client and comprised three boreholes drilled by light percussive techniques to 30m. These were supplemented by six windowless sampler holes to a maximum of $2\frac{1}{2}$ m depth. The location of the exploratory points is shown on Figure 1 at Appendix E.

Representative soil and water samples were recovered from the boreholes for subsequent laboratory examination and testing; whilst Standard Penetration Tests (SPT) were carried out as appropriate. Details of the strata encountered are provided on the Borehole and Trial Pit Records at Appendix A; together with particulars of the samples recovered, groundwater observations and SPT results. The profile of SPT verses depth is provided at Figure 3 of Appendix E.

Standpipes were installed in the three percussive boreholes to allow monitoring of groundwater levels. Three sets of monitoring of the standpipes have been undertaken and the results of this are presented at Appendix B.

6.0 Laboratory testing

The following laboratory tests were conducted on samples recovered during the field work: -

- Natural moisture content: to assess the in situ condition of the soil.
- Liquid and Plastic Limits: to classify cohesive soil into behavioural groups.
- Particle size distribution: by sieve analysis to classify granular material.

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- Unconsolidated undrained triaxial compression: to determine the shear strength of cohesive material and thus to assess its load bearing capacity.
- One-dimensional consolidation: to determine the deformation characteristics of clay under applied loading and unloading.
- Soluble sulphate concentration and pH value: for the specification of buried concrete.
- Soluble sulphate concentration and pH value: for the specification of buried concrete.
- Contamination: chemical analyses to detect the presence of contaminants as detailed in the Fairhurst report, viz: -

Metals & metalloids: Total arsenic, cadmium, chromium, copper, lead, mercury,

nickel

Inorganics: Water soluble boron.

Organic: Petroleum hydrocarbons (TPH), polyaromatic hydrocarbons

(PAH), volatile organic compounds (VOC), semi volatile

organic compounds (SVOC) and phenols.

Others: Asbestos screen and Waste Acceptance Criteria (WAC) for

off site disposal of construction arisings.

Results of these tests are presented at Appendix C, whilst original testing house sheets are at Appendix D.

7.0 Ground conditions

7.1 Stratigraphy

The stratigraphy of the site as revealed by the boreholes is consistent with that previously found by Fairhurst and are described in detail at Appendix A.

7.2 Groundwater

Groundwater was encountered at some $3 - 3\frac{1}{2}m$ depth. However, the speed of drilling, the requirement to add water in granular material and the use of casing to support the bore will have masked some inflows. Groundwater monitoring standpipes were installed in the three boreholes.

Details of all groundwater observations from the standpipes are provided on the standpipe monitoring records and Appendix B; but it should be noted that this work was undertaken at the end of a very long dry summer and water levels may be depressed below normal levels.

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8.0 Discussion

8.1 General

The site has evidently already carried development and the investigation has revealed a some Made Ground to be present. It is possible that other pockets of Made Ground may also be present; perhaps deeper, of different character or associated with the remains of underground construction; even though not detected by this investigation.

All remnants of previous construction should be removed prior to redevelopment to enable the proposals to be constructed without hindrance and to perform satisfactorily.

The magnitude of the expected loading is such that piled foundations are recommended.

8.2 Piled foundations

Either driven or bored piles would be suitable in the ground conditions found at this site. However, compared with bored piling, construction of driven piles generates greater noise and vibration which is unlikely to be acceptable in this environment. In particular, high levels of ground - borne vibrations could damage nearby structures. Consideration of the various advantages and disadvantages of the different pile types suggests CFA piles to be preferred.

They avoid many of the installation difficulties that would otherwise be experienced, particularly the need for casing and the control of groundwater inflows. Parameters for their preliminary design are provided in Tables 1 and 2.

Table 1: Design parameters for CFA piles - Shaft friction

Stratum	Typical depth, m	Ultimate unit shaft friction
All material	0.0 - 3.0	Ignore
Kempton Park Gravel	3.0 - 6.0	Increases linearly from 30 to 50 kPa
London Clay	6.0 - 30.0	Increases linearly from 40 to 145* kPa

^{*} the average unit shaft friction on the pile must not exceed 100kPa

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Table 1 has been derived in conjunction with an adhesion factor of 0.6 in the cohesive London Clay. For the Kempton Park Gravel a value of 1.0 is assumed for the coefficient of earth pressure at rest (K_0) as the gravel is assumed to be over-consolidated. The ratio between the lateral earth pressure (K_0) and K_0 is taken as 0.90 for CFA piles. For these piles the angle of friction between pile shaft and soil is assumed to be equal to the angle of internal friction of the soil itself (\emptyset).

Table 2: Design parameters for CFA piles - End bearing capacity

Stratum	Typical Ultimate unit end bearing capacit depth, m				
London Clay	6.0 - 30.0	Increases linearly from 630 to 2205 kPa			





A factor of safety must be applied to derive the allowable working load from the ultimate values obtained from Tables 1 and 2. An overall value of 2.5 is commonly employed in compression.

Working pile load tests could be carried out to verify the chosen FoS on a development of this scale. This could result in a lower factor of safety than 2.5 which could result in significant cost savings. The actual factor of safety will be dependent upon the knowledge and experience of the chosen pilling contractor and agreement with relevant parties but is generally in the order of 2.25.

Tables 1 and 2 predict that a CFA pile of 600 mm diameter, bored 24 m depth, will have an allowable load capacity 1400kPa under an overall factor of safety of 2.5.

Settlement of a single pile is not expected to exceed some 1 to 1½% of the pile diameter as the working load assuming it will be carried entirely on skin friction.

Table 3 provides preliminary load capacity for bored piles in compression under a factor of safety (FoS) of 2.5 for various pile diameters.

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Table 3: Pile diameters and capacities for bored piles

Depart	Cohetino	Of Capacity Million Load Capacity Allows				Allowater	THE CAL DAD							
Descon	23001	Stati		End.	1 1 7 7 7	Viction .	East	Total	Lago	Shirt		End	Total	Lood
0.1			Par	Searing	- 60		Dearing		Capacity		M	Bouries		Caputi
um -	300	Grava	C Clay	129	Direktor	L Clay	KN	100	10	Grayer	L. Cay	110	401	8.90
3.0	200	36		7.6	37	9 020		0	-	0		1	0	
3.5		3.7			22			22		30	6 1		30	
4.0		37			47			47		63			63	
4.5		90			74			74		99			99	
5.0		63			7514			104		rati			TAU	
0.3		47			735			125		181			181	
6.0		50	32	630	170	170	160	270		226	226	175	404	
6.8		-	34	653	1	209	166	356		(0.00)	257	387	454	
2.0			ds	dan		232	171	343			309	197	505	
7.5	100		49	728		Dist	118	382	itsá		354	206	560	- 23
8.0			55	751	1	-301	121	422	168		401	215	610	
8.5			53	79.4		237	120	1853	185		450	225	674	
0.0			35	827		276	1.12	507	203		300	234	335	
0.5			67	850		415	137	552	291		550	243	797	
10.0	-		60	853		407	142	590	239		600	262	661	
10.5			62	925		400	147	647	250		056	262	970	
11.0			64	958		544	162	asis	276		120	270	pas	
11.5			65	901	//	500	156	747	296		756	280	7087	
12.0			0.9	1024		537	143	300	120		850	289	#139	
12.5			170	1057		585	156	354	342		910	596	1214	
13.0			102	1059		727	173	#20	164		982	308	2290	
13.5			75	1122		759	178	967	397		1052	317	7369	
18.0			77	1155		(443)	156	1026	311		1123	221	1450	
14.5			79	1150		808	Yes	1087	438		1797	336	7833	
150			61	1221	11	255	194	1149	459		1273	345	1818	
15.5	10000		84	1253		1013	199	1212	485		1850	364	1765	
15.0			85	1285		1979	305	1277	217		1420	364	7794	
10.5			85	1310		1134	2/0	1394	538		1512	227	1805	
37.0			90	1352		7107	215	1412	566		1506	380	7978	
17.5			02	1355		7267	220	1482	860		1652	392	2073	
70.0			95	1410		1927	225	1000	127		1770	407	2171	
76.5			97	1450		7395	231	7626	950		1850	410	2270	
10.0			02	1493		7464	236	1700	680		1952	419	2372	
19.5	9/3/9/		109	15:0		1635	241	1770	710		2016	429	2475	
20.0	100000		107	1549		1607	246	1853	741		2143	408	2007	
20.5			105	1652		7681	252	1932	773		2241	447	2555	
21.0			108	1674	110	1756	267	2013	808		2341	466	2798	
21.5			110	1647		1839	262	2096	858		2444	460	2910	
22.0			1772	1670		1911	267	2679	871		2048	475	3073	
22.5			919	1703		2001	272	2284	205		2655	484	3158	
23.0			118	1746		2073	278	2350	940		2764	494	325	
23.5			119	1779		2585	787	24,76	978		2874	503	-3377	
74.0			121	1821		2240	256	2820	1011		7957	eta	3409	
24.5			122	1844	1	2327	293	2620	1048		3102	521	3623	
25.0			125	1877		2414	299	2713	1008		3219	631	3750	
25.15			127	1910		1503	304	2807	1120		2370	540	3078	
28.0			130	1943		2594	309	2903	yier		5459	549	4008	
26.5			135	1975		2886	314	3001	7200		3582	589	4140	
27.0			134	2009		3780	376	3790	1240		3707	568	4275	
27.5			135	2041		2075	325	3200	1200		3634	577	1007	
29.0			138	2074		2973	430	3,903	1321		3964	586	4650	
28.5			140	2107		3071	335	3406	1962		V009	506	4607	
29.0			147	2139		3171	240	3511			1228	365	1835	
20.6			145	2172		3273	345	3018			4354	814	4975	
30.0			147	2200		1378	Д51	3127			9801	523	5/25	

Single piles streds soil in excess of depth of investigation: prove ground conditions throughout cone of stress before using.
Pile groups should be considered separately.

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The actual load capacity achieved in practice depends upon the precise installation procedures. Advice should therefore be sought from specialist contractors to verify the load capacity and settlement characteristics of their particular piles in the ground conditions revealed by this investigation. In any event, it is recommended that the chosen pile configuration be confirmed by preliminary load tests conducted before installation of the contract piles in order to take advantage of minimum FoS and thus minimum cost.

8.3 Excavations

All material should be regarded as unstable. Some apparent stability may be present immediately on excavation, especially where there is a high clay content, but this must not be relied upon. All excavations should therefore be supported at all times unless battered to a safe angle of repose.

Provision of adequate support is especially important for the safety of personnel when required to work in or close to excavations. Particular care should be exercised where excavations are close to existing structures to ensure they do not experience any loss of support. Temporary and permanent works should be designed to resist the additional lateral earth pressures arising from any superimposed loads in addition to those generated by the soil itself, without significant deformation.

Groundwater was encountered at some 2½m depth in BH1 and over 4m in the other boreholes and thus is expected to be below the depth of general construction activities.

8.4 Contaminant analysis

8.4.1 Solid phase

Contaminant testing was undertaken on selected soil samples and the results compared with the limited number of CLEA[2] Soil Guideline Values (SGVs) that have been published to date. Where not available from that source, reference has also been made to the LQM CIEH Generic Assessment Criteria[3]. Appropriate trigger levels are given with the results at Appendix C.

Analysis for metals/metalloids revealed all determinands to be below the triggers for Residential land use without plant uptake.

There is no trigger presently available for lead, but the results are below the previous trigger of 450mg/kg. However, two results from BH1 & 3 are above the value of 276mg/kg provided in the AtRiskSoil database produced by Atkins and the more modern Category 4 Screening Levels of 310/kg produced by DEFRA for residential use without plant uptake.

The results for TPH analysis found very low levels that are below triggers or of little environmental interest. The PAH analysis also found the majority of determinands to be below the residential trigger levels apart from BH2,3 & WSA in which some individual determinands are above trigger levels.

Analysis for asbestos was carried out on ten selected samples. Two samples recorded some fibres being from WSB & E.

The results of this analysis are in broad agreement with that previously found by Fairhurst.

With regards to potential remediation of soil contaminated with PAH. metals or asbestos, it is considered that where in areas of hardstanding or below building footprints,

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contaminants can remain in situ, as the pathway in a source-pathway-receptor model will be blocked. Where they coincide with areas of soft landscaping, remediation will be necessary.

A 450mm thick cap should be utilised, comprising imported clean soils with a capillary break layer at the base underlain by a geotextile barrier. This is to block the pathway in a source-pathway-receptor model and to discourage excavation below that level.

Nevertheless, in view of the foregoing, it is recommended that appropriate health and safety precautions, such as detailed in HS(G)66[4] and elsewhere, should be followed by the construction workforce and others who may come into contact with potentially contaminated soil. They should be agreed with the Health and Safety Executive and are likely to include, but not be restricted to, the following: -.

- maintenance of good standards of personal hygiene.
- wearing personal protective clothing that is changed and cleaned frequently to eliminate skin contact.
- prevention of ingestion by using washing and changing facilities at all break times.
- prohibition of eating, drinking or smoking between break times.
- controlling the spread of dust and airborne mists to prevent inhalation.

8.4.2 Waste Acceptance Criteria (WAC) analysis

Four samples were subject to the WAC full solid waste and leachate suite. The results are presented at Appendix D. The results have been compared to the criteria contained in the Landfill Regulations 2002 as amended and are presented at Appendix D.

Within the compliance leachate waste suite, all determinands were within the Inert Waste Landfill criteria limits. Similarly, all parameters determined on the solid test were also within the Inert Waste Landfill criteria.

The contamination test results, and the WAC results should be forwarded to the contractor appointed to undertake any spoil removal. Transfer notes and chain of custody sheets should be retained for all arising removed from site.

8.5 Buried concrete

Laboratory tests for this work found soil samples yielding a characteristic soluble sulphate concentration of 0.46g/l which results in a Design Sulphate Class[5] of DS-1. Although the previous works should also be assessed to see if this level needs to be ungraded.

Ground conditions are such that groundwater must be considered to be mobile, and all pH determinations were in excess of 6.5. Therefore, the Aggressive Chemical Environment for Concrete, ACEC, is classed as AC-1.

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9.0 Remedial Action Plan

9.1 General

Contaminant testing was undertaken on selected soil samples and the results compared with the limited number of CLEA[1] Soil Guideline Values (SGVs) for residential land use that have been published to date. Where not available from that source, reference has also been made to the LQM CIEH Generic Assessment Criteria[2]. Appropriate trigger levels are given with the results at Appendix C and individual values exceeding the triggers have been highlighted.

Further WAC tests will be undertaken by the Ground Works Contractor as necessary during the works to categorize the soil arisings prior to disposal off site.

A Discovery Strategy and Watching Brief is also being adopted on site so that any unexpected contamination identified on site is reported.

9.2 Prevention of Pollution

Care should be taken in order to minimize the production of dust generated on the site, especially in dry climatic conditions.

All vehicles leaving the site should be clear of any potential contaminated debris other than that to be specifically removed. Vehicles transporting wastes for disposal should be appropriate for the required means in order to prevent release during transit. Specific materials removed from site for subsequent disposal should be transported to a

suitably approved and licensed facility. The contractor shall maintain a full documentary record in accordance with Duty of Care including copies of all Waste Transfer Notes for verification.

The programme of works and any subsequent modification should be maintained in such a manner as to avoid potential cross-contamination. A watching brief should be maintained and Manhire Geo-Environmental Ltd contacted in the event that evidence of any additional 'contamination' is encountered. The contractor should allow for any delays resulting from the presence of possible contamination and the necessary measures thereof.

It is the responsibility of all personnel on site to report any evidence of possible contamination to the suitably qualified and experienced Site Agent who will contact Manhire Geo-Environmental Ltd where there is any doubt about this contamination.

9.3 Remediation

Based on the previous investigations, it is considered that where contamination coincides with areas of soft landscaping, remediation will be necessary. A 450mm thick cap should be utilised, comprising imported clean soils with a capillary break layer at the base underlain by a geotextile barrier. This is to block the pathway in a source-pathway-receptor model and to discourage excavation below that level.

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9.4 Validation of Imported/ Site Sourced Materials - Demolition Materials & Waste

Imported / site sourced materials can generally be split into two categories imported or site sourced demolition materials and imported soils validation of both types of materials will be required.

It should be noted that should material come from more than one source then the minimum following specified samples per source will be required.

9.4.1 Site Sourced / Imported Demolition Materials

All material should be visually assessed to check for any contamination, including asbestos presence. Any suspected materials should be removed from site. For all material the following amount of validation testing is required as a minimum number of samples per source should be 4 and samples should be analysed at a rate of 1 sample per 100m3 of material, but allowing for the above minimum. Assuming no other contamination is suspected all samples should be analysed for asbestos, CLEA metals, phenols, speciated PAH and TPH.

9.4.2 Imported Soils

All imported soils should be certified clean by the supplying facility, with tests certificates supplied.

In addition, on site sampling of the soils will be required, this should ideally be done when it has been placed, but as a minimum be undertaken on site. The following amount of validation testing is required. A minimum number of samples per source should be 4 and samples should be analysed at a rate of 1 samples per 50m3 of material but allowing for the above minimum.

All samples should be analysed for asbestos, CLEA metals, phenols, speciated PAH and TPH

9.5 Discovery Strategy and Watching Brief

During the course of the development, it will be the responsibility of the on-site manager to ensure watching briefs are undertaken and documented. The watching brief will consist of a record of:

- a. Observations of contamination made during the course of the development by any member of site staff, contractor or visitor; and
- b. The key stages of the development and occurrences including contamination found during the course of the development, the formation levels of excavations, any reduced level dig/mass excavation, formation of landscaped or garden areas, etc. with supporting photographs.

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9.6 Watching Brief Written Statement

Upon completion of the development, a written and signed statement will be prepared by the following parties:

- a. Ground Works Contractor(s) upon completion of foundations and ground works;
- b. On-site manager upon completion of groundworks and landscaping work; and

The written statement will clearly state whether or not evidence of contamination was identified during the course of the development along with any remedial action that was taken.

9.7 Staff Training

All site staff, contractors and visitors, will be briefed on the potential presence of land, groundwater or airborne contamination before commencing work on the site.

This will include the following information:

- a. Relevant health & safety considerations;
- b. The type of land, water or airborne contamination present and potentially present at the site;
- c. Any particular areas of the site which are likely to be affected; and
- d. Staff responsibilities under the discovery strategy.

The on-site manager will provide written confirmation that site staff were briefed about contaminated land in line with these recommendations.

9.8 Construction Workers Risk Assessments

A detailed health and safety risk assessment will be carried out, in accordance with current guidance, before works commence on-site and appropriate personal protective equipment will be worn by all construction workers.

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10.0 Discovery Strategy & Watching Brief

If unexpected contamination is found at any time, then this will be reported to Environmental Health, The Royal Borough of Richmond Upon Thames (RBRUT) within seven days and 2 weeks notice given prior to undertaking any remediation if considered necessary.

The discovery strategy sets out the actions that will be taken if contamination is encountered during the course of a development. Examples of the types of observations that will be considered are detailed in Table 1 below, following which examples of the general actions that will be taken are set out in Table 2, and examples of any emergency actions are detailed in Table 3.

Evidence	Description
Visual	 Fuel or oil-like substances mixed in with or smeared on the soil or floating on perched, groundwater or surface waters. Waste materials (refuse, barrels, industrial wastes, ash, tar, etc.) Buried at specific location across the site. Marked variation in colour, e.g., red, orange, yellow, green, light or
	 dark blue, etc. may indicate contamination from a variety of contaminants. Soils including large amounts of ash and clinker where such contamination of soils wasn't expected. Evidence of suspected asbestos.
Odours	Fuel, oil and chemical-type odours.Unusual odours such as sweet or fishy odours.
Well-being	 Lightheadedness and/or nausea when in excavations, at the working face of an excavation, when visual or olfactory evidence of contamination exists, etc. Burning of nasal passages, throat, lungs or skin. Blistering or reddening of skin due to contact with soil.

Table 1 - Potentially significant observations (not exhaustive)

Person observing contamination	□ To be reported to	 Action to be taken
Site visitor	 Must report observations to the site manager. 	 Stop work and where possible, make area safe and secure area before reporting to site manager.
 Contractor 	 Must report observations to the site manager. 	 Stop work and where possible, make area safe and secure area before reporting to site manager.
 On site manager 	 Must report observations to their direct manager, the appointed Environmental Consultant, the Planning Authority and Contaminated Land Officer at LBS. 	 Stop work and, where possible, make area safe and secure area before reporting to others.
 Environment al Consultant 	 Must report observations to the site manager, the planning Authority and Contaminated Land Officer LBS. 	 Advise that work stops and where possible, make sure the area is made safe before reporting to others.

Table 2 - General actions to be performed

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 Occurrence 	 Description 	□ Contact
Risk to the public	 If at any point residents, the public or other may be at risk as a result of contamination found during the course of investigation, remediation or development works. 	 Contact the emergency services if there is a risk to life. Contaminated Land Officer at LBS. Health & Safety Executive.
 Nuisance to residents/the public 	 If a nuisance has been or is likely to be caused to nearby residents, the public and others - for example, odours, dust, noise, vibration, etc. 	Environmental Health at LBS.
Pollution of controlled waters	If any surface, culverted or groundwater has been polluted, e.g., slurry, contaminated soil/water or a chemical spillage entering a river or canal.	 Environmental Agency. Planning Authority and Contaminated Land Officer at LBS.
 Pollution of adjoining land 	 If land outside the boundary of the development site is polluted from site activities, e.g., slurry, contaminated soil/water or a chemical spillage. 	 The owner of the land. Planning Authority and contaminated Land Officer at LBS.

Table 3 - Emergency actions to be performed

In addition to the above, the following should also be noted:

- a. Should gross contamination be encountered during groundworks, then the advice of an environmental consultant will be sought;
- b. Any soil arising from areas where visual or olfactory evidence of contamination has been observed will be handled as potentially hazardous waste and temporarily stockpiled appropriately;
- c. Stockpiled soils may undergo validation testing to determine whether they can be used elsewhere on the site at or close to the ground; and
- d. The Contaminated Land Officer at RBKUT will be contacted to agree any further remedial works that become necessary during the course of the development.

Manhire Associates

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11.0 Verification Report

Following the completion of any remedial action and the development of the site, a post -remediation verification report will be produced and submitted to the planning authority.

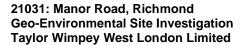
The report will detail fully any actions undertaken, including any report from the discovery strategy and watching brief.

All work undertaken will be supported by photographic evidence, along with any other documented items such as waste transfer noted, asbestos removal reports, soil invoices etc.

All imported soil will undergo confirmatory analysis in order to satisfy their suitability for use. Laboratory certificated results will be included within the report.

Finally the report will include the aforementioned declaration from the site manager, groundworks contractor and appointed environmental consultant stating that wither no suspected additional contamination was detected, or, that all additional detected contamination has been addressed appropriately.

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References

- [1] The Contaminated Land Exposure Assessment Model Department for Environment, Food and Rural Affairs The Environment Agency
 R & D Publications SGV 1 *et al.*, March 2002
- [2] Protection of workers and the general public during the development of contaminated land HS(G)66 Health and Safety Executive, 1991
- [3] Waste Management Paper 27 : Landfill gas Department of the Environment HMSO, Fourth impression 1996
- [4] Concrete in aggressive ground BRE Special Digest 1, 3rd Edition Building Research Establishment, 2005

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PROCEDURAL NOTES for GROUND INVESTIGATIONS

General

This report is based upon data obtained from field descriptions of the strata and examination of the samples by an engineer, together with the results of in situ and laboratory tests as appropriate. Responsibility cannot be accepted for variations in ground conditions between and around any of the exploratory points that is not revealed by the data. Whilst the report may offer an opinion on the ground conditions between exploratory points and below the depth of investigation, this is for guidance only and no liability is accepted for its accuracy.

Drilling procedure

Boring by light cable percussion drilling allows the ground conditions to be reasonably well established. However, a certain amount of disturbance is inevitable, and some mixing of soils can occur.

Sampling procedure

"Undisturbed" samples of predominantly cohesive soils are taken with a 100mm diameter open tube sampler, generally in accordance with BS 5930: 1999.

Where appropriate, or where an undisturbed sample is unsuccessful, disturbed samples are recovered and sealed into polythene bags.

Groundwater samples are taken when water is encountered in sufficient quantity.

Standard penetration tests

The test is conducted generally in accordance with BS 1377: Part 9: 1990. The sampler tube is subject to a seating drive of 150mm into the soil at the base of the borehole. Results are given on the Borehole Records as the number of blows required to drive the sampler tube a further 300mm and this is known as the "N" value. Where the driving resistance is such that full penetration is not achieved, the test is generally terminated after 50 blows and the actual distance penetrated is recorded.

Groundwater

Groundwater observations necessarily reflect the conditions encountered at the time of the exploratory work. Long term monitoring of standpipes is usually required to establish an equilibrium water level since the normal rate of boring is too fast to permit steady state conditions to be achieved.

Groundwater levels are subject to variations caused by changes in drainage conditions and seasonal climatic changes.

Water may necessarily be added to advance the bore whilst casing may be required to maintain an open hole. These can both mask subsequent groundwater observations and are therefore noted on the individual Borehole Record.

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APPENDIX A BOREHOLE AND TRIAL PIT RECORDS

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SYMBOLS and ABBREVIATIONS

Samples

Undisturbed

U Standard open drive "undisturbed"

102mm dia. in boreholes

38mm dia. in trial pits, window sampler

and hand auger

T Thin wall open drive

P Piston

CBR CBR mould

_ Windowless sampler liner

Disturbed

D Small

B Bulk

W Water

C Contaminants: plastic tub

Contaminants: brown glass jar

In situ tests

SPT Standard Penetration Test, open shoe

CPT solid cone

N value is number of blows for $300 \, \text{mm}$

penetration.

Blow count also given as seating drive followed by four increments of 75mm.

 $V\ (\quad) \quad Vane\ test\ (c_{\scriptscriptstyle u}\ kPa)$

P() Hand penetrometer (c_u kg/cm²)

M () Mexe probe (CBR %)

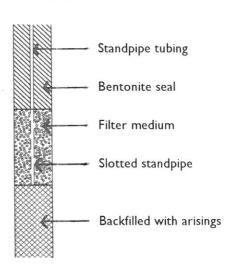
Water records

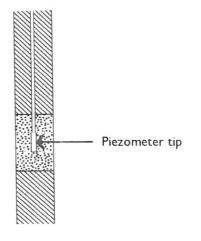
▼
₂ Standing level

→ Depth encountered

suffix identifies separate strikes

Standpipes





				E ASSOCIATES VIRONMENTAL		0		Site Manor Road, Richmond		N	orehole umber BH1
Boring Met			Diamete 0mm cas	r ed to 7.00m	Ground	Leve	I (mOD)	Client Taylor Wimpey West London		N	ob umber
		Locatio Se	n e site pla	ın	Dates 08	3/08/2 9/08/2	022- 022	Engineer		SI	heet 1/4
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	(Thi	epth (m) ckness)	Description	Legend	Water	Instr
							(0.10) 0.10 (0.10) 0.20 (0.80)	BLOCK PAVING MADE GROUND: Sand MADE GROUND: Reddy brown angular gravel			
1.00-1.50 1.20-1.65	B1 SPT(C) N=29	1.20	WET	2,4/6,6,8,9			1.00 (0.50) 1.50	MADE GROUND: Brick rubble with some orange brown sand and gravel Medium dense to dense brown and orange brown sandy GRAVEL			
2.00-2.45 2.00-2.50	SPT(C) N=44 B2	2.00	1.50	3,7/9,11,11,13				Salluy GRAVEL			
3.00-3.45 3.00-3.50	SPT(C) N=41 B3	3.00	2.00	3,7/9,11,13,8							
4.00-4.45 4.00-4.50	SPT(C) N=25 B4	4.00	3.00	3,5/5,6,7,7			(5.20)				100 - 100 -
5.00-5.45 5.00-5.50	SPT(C) N=16 B5	5.00	4.00	2,3/3,4,4,5							6. 1 10 10 10 10 10 10 10 10 10 10 10 10 1
6.50-6.95 6.50-7.00	SPT(C) N=15 B6	6.50	4.00	2,2/3,3,4,5			6.70	Stiff becoming very stiff fissured dark grey silty CLAY with some sandy laminations	X x		1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
8.00-8.45	U1	7.00	DRY	100 blows					×		
9.50-9.95	D1 SPT N=24	7.00	DRY	2,4/5,6,6,7					X		
Remarks UXO survey Chiselling fro	taken at 1.2m, 2.0m om 0.00m to 1.20m f	, 4.0m, 6.0 or 1.0 hou	0, 8.0, 10 r. Water	.m added from 0.70m to	6.50m.				Scale (approx)	Lo By	ogged y ljs
									Figure N	lo. 67 B⊦	——— H1

		M. Gi	ANHIRI EO EN	E ASSOCIATES VIRONMENTAL	LIMITED)	Site HOMEBASE, 84 MANOR ROAD, RICHMOND UI THAMES, TW9 1YB	PON	Borehol Number BH1 Job Number 2/4 end by Instr				
Boring Meth Cable Percus			Diamete 0mm cas	r ed to 7.00m	Ground	Level (mOD)	Client Taylor Wimpey West London		N	lumber			
		Locatio Se	n e site pla	ın	Dates 08	3/08/2022- 9/08/2022	Engineer		SI				
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr			
							Stiff becoming very stiff fissured dark grey silty CLAY with some sandy laminations	× · · · · · · · · · · · · · · · · · · ·					
11.00-11.45	U2	7.00	DRY	100 blows				× · · · · · · · · · · · · · · · · · · ·					
11.50	D2							× × × × × × × × × × × × × × × × × × ×					
12.50-12.95	SPT N=29	7.00	DRY	3,4/6,6,8,9				× × × × × × × × × × × × × × × × × × ×					
14.00-14.45	U3	7.00	DRY	100 blows				× × × × × × × × × × × × × × × × × × ×					
14.50 15.50-15.95	D3 SPT N=32	7.00	DRY	3,4/6,7,9,10				X					
17.00-17.45 17.50	U4 D4	7.00	DRY	100 blows				× × × × × × × × × × × × × × × × × × ×					
18.50-18.95	SPT N=36	7.00	DRY	3,4/6,9,10,11				X					
Remarks								Scale (approx)	LiB	ogged by			
								1:50 Figure N	 No. 87.BH	ljs H1			

	-	M	ANHIR EO EN	E ASSOCIATES VIRONMENTAL	LIMITE)	Site Manor Road, Richmond el (mOD) Client				
Boring Meth Cable Percus			Diamete 0mm cas	r ed to 7.00m	Ground	Level (mOD)	Client Taylor Wimpey West London		N	ob umber 1631	
		Locatio Se	n e site pla	ın	Dates 08	3/08/2022- 9/08/2022	Engineer		S	heet 3/4	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr	
20.00-20.30	U5	7.00	DRY	100 blows			Very stiff fissured dark grey silty CLAY with some sandy laminations	× · · · · · ·			
20.35	D5					<u></u>	,	× · · · · · · · · · · · · · · · · · · ·	: -		
21.50-21.95	SPT N=41	7.00	DRY	3,6/9,10,11,11				X			
23.00-23.30 23.35	U6 D6	7.00	DRY	100 blows				X			
24.50-24.95	SPT N=42	7.00	DRY	3,6/9,10,10,13		E (23.30)					
26.00-26.35 26.40	U7 D7	7.00	DRY	100 blows				X			
27.50-27.95	SPT N=47	7.00	DRY	3,6/10,13,11,13				× · · · · · · · · · · · · · · · · · · ·			
29.00-29.45	U8	7.00	DRY	100 blows				× × × × × × × × × × × × × × × × × × ×			
29.50	D8			09/08/2022:DRY	_	=- =- =- =-		× × × × × × × × × × × × × × × × × × ×	1		
Remarks						30.00	<u> </u>	Scale (approx)	L _i	ogged y	
								1:50 Figure N	No. 67.Bl	ljs H1	

		M	ANHIRI EO EN	E ASSOCIATES VIRONMENTAL	0	Site Manor Road, Richmond				
Boring Meth Cable Percus		Diamete 0mm cas	r ed to 7.00m	Ground	Level (mOD)	Client Taylor Wimpey West London		N	ob lumber	
		Locatio Se	n e site pla	ın	Dates 08	3/08/2022- 9/08/2022	Engineer		s	heet 4/4
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legen	Water	Instr
30.00-30.45	SPT N=49	7.00	DRY	4,6/10,12,13,14						
Remarks		1	I	ı		1		Scale (approx)	L	ogged y
						1:50 Figure	No.	ljs		
									Νο. 67 Β	H1

	-	M	ANHIR EO EN	E ASSOCIATES VIRONMENTAL	LIMITE	D		Site Manor Road, Richmond		N	Borehole lumber BH2
Boring Met			Diamete 0mm cas	r ed to 7.00m	Ground	Leve	el (mOD)	Client Taylor Wimpey West London		N	lob lumber 21931
		Locatio Se	n e site pla	ın	Dates 05	5/08/2	2022	Engineer		S	Sheet 1/4
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	(Th	Depth (m) ickness)	Description	Legend	Water	Instr
0.30-0.70 0.80-1.20	B1 B2						(0.01) 0.01 (0.29) (0.40) 0.70 (0.80)	BLOCK PAVING MADE GROUND: Sand MADE GROUND: Reddy brown angular gravel MADE GROUND: Brick rubble with some orange brown sand and gravel			
1.50-1.95	SPT N=17	1.50	DRY	2,2/3,4,5,5			1.50	Medium dense to dense orange brown and brown sandy GRAVEL			
2.00-2.45 2.00	SPT N=22 D1	2.00	DRY	2,2/3,4,6,9							
3.00-3.45 3.00-3.50	SPT(C) N=39 B3	3.00	DRY	5,7/9,10,11,9			(3.90)				
4.00-4.45	SPT(C) N=40	4.00	DRY	7,9/10,10,11,9							
5.00-5.45 5.00-5.50	SPT(C) N=32 B4	5.00	WET	4,7/9,9,7,7			5.40	Stiff becoming very stiff fissured dark grey silty CLAY with some sandy laminations	×		
6.50-6.95	SPT N=16	6.00	DRY	2,2/3,4,4,5					X		158 158 158 158 158 158 158 158 158 158
8.00-8.45	U1	6.00	DRY	50 blows					× × × × × × × × × × × × × × × × × × ×		
9.50-9.95	D2 SPT N=20	6.00	DRY	2,2/4,5,5,6					x x x x x x x x x x x x x x x x x x x		
Remarks UXO survey Chiselling fr	/ taken at 1.50m 3.0n om 0.00m to 1.20m f	n, 5.0m, 7. for 1 hour.	0m, 9.0m Water ad	n, 11.0m Ided from 4.00m to 6	.00m.				Scale (approx)	L _e	ogged By
									1:50 Figure N 556		ljs H2

		M	ANHIRI EO-EN	E ASSOCIATES VIRONMENTAL	LIMITED		Taylor Wimpey West London Engineer						
Boring Meth Cable Percus			Diamete 0mm cas	r ed to 7.00m	Ground	Level (mOD)			N	ob lumber			
		Locatio Se	n e site pla	ın	Dates 05	5/08/2022	Engineer		SI	heet 2/4			
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr			
							Stiff becoming very stiff fissured dark grey silty CLAY with some sandy laminations	× · · · · · · · · · · · · · · · · · · ·					
11.00-11.45	U2	6.00	DRY	90 blows		<u>-</u> - - - - - - -		× × × × × × × × × × × × × × × × × × ×					
11.50	D3							× × × × × × × × × × × × × × × × × × ×	<u>.</u>				
12.50-12.95	SPT N=25	6.00	DRY	2,3/4,5,7,9				× × × × × × × × × × × × × × × × × × ×					
14.00-14.45	U3	6.00	DRY	100 blows				× × × × × × × × × × × × × × × × × × ×					
14.50 15.50-15.95	D4 SPT N=26	6.00	DRY	2,3/5,5,8,8		<u> </u>							
17.00-17.30 17.35	U4 D5	6.00	DRY	100 blows				× × × × × × × × × × × × × × × × × × ×					
18.50-18.95	SPT N=28	6.00	DRY	3,3/4,6,8,10				× × × × × × × × × × × × × × × × × × ×					
Remarks	<u> </u>	1	l	1	-		I	Scale (approx)	Lo B	ogged y			
								1:50 Figure N	No. 67.Bl	ljs H2			

		M	ANHIRI EO EN	E ASSOCIATES VIRONMENTAL	LIMITED)	Site Manor Road, Richmond	3						
Boring Meth Cable Percus			Diamete 0mm cas	r ed to 7.00m	Ground	Level (mOD)	Client Taylor Wimpey West London		N	lumber				
		Locatio Se	n e site pla	n	Dates 05	5/08/2022	Engineer		SI					
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr				
20.00-20.35	U5	6.00	DRY	100 blows		 E E	Very stiff fissured dark grey silty CLAY with some sandy laminations	× · · · · · · · · · · · ·						
20.40	D6					<u></u>	Some sunay luminations	* - *	.					
21.50-21.95	SPT N=35	6.00	DRY	3,4/6,9,10,10				X						
23.00-23.40	U6	6.00	DRY	100 blows				× × × × × × × × × × × × × × × × × × ×						
23.45	D7							x						
24.50-24.95	SPT N=41	6.00	DRY	3,6/9,10,10,12				× · · · · · · · · · · · · · · · · · · ·						
26.00-26.45	SPT N=45	6.00	DRY	3,7/10,11,11,13	-			X						
27.50-27.95	SPT N=44	6.00	DRY	4,7/9,10,11,14				X						
30.00-30.45 Remarks	SPT N=47	6.00	DRY	4,8/11,11,11,14				× × × × × × × × × × × × × × × × × × ×		Outed				
								Scale (approx)	B	ogged y				
								1:50	<u></u>	ljs				
								Figure N 556	νο. 37 ΒΗ	H2				

		M G	ANHIRE EO-EN	ASSOCIATES /IRONMENTAL)	Site Manor Road, Richmond	don Nt. ≥1					
Boring Met			Diameter 0mm case	ed to 7.00m	Ground	Level (mOD)	Client Taylor Wimpey West London		N	lob Number 21631		
		Locatio Se	n e site plar	1	Dates 05	5/08/2022	Engineer		S	Sheet 4/4		
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legen	Water	Instr		
Remarks						(25.10)	Very stiff fissured dark grey silty CLAY with some sandy laminations Complete at 30.50m					
Remarks								Scale (approx) E	ogged By		
								1:50 Figure	No.	ljs		
								567.B	H2			

	-			E ASSOCIATES		D		Site Manor Road, Richmond		N	orehole lumber BH3			
Boring Met			Diamete 0mm cas	r ed to 5.50m	Ground	Leve	el (mOD)	Client Taylor Wimpey West London		N	ob lumber Hgan			
		Locatio Se	n e site pla	ın		0/08/2 1/08/2		Engineer		egend She				
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	(Thi	Depth (m) ickness)	Description	Legend	Water	Instr			
0.60-1.00	B1						(0.10) 0.10 (0.10) 0.20 (0.40) 0.60 (0.70)	BLOCK PAVING MADE GROUND: Sand MADE GROUND: Reddy brown angular gravel MADE GROUND: Brick rubble with some orange brown sand and gravel	8.5.4.5.					
1.50-1.95 1.50-2.00	SPT(C) N=34 B2		DRY	3,4/7,7,7,13			1.30	Medium dense to dense orange brown and brown sandy GRAVEL						
2.00-2.31 2.00-2.50	SPT(C) 50/160 B3	2.00	DRY	10,15/0,24,20,6					2					
3.00-3.45 3.00-3.50	SPT(C) N=22 B4	3.00	2.00	3,6/6,6,5,5			(3.80)							
4.00-4.45 4.00-4.50	SPT(C) N=27 B5	4.00	3.00	3,6/7,7,6,7										
5.00-5.45 5.00-5.50	SPT(C) N=10 B6	5.00	4.00	2,2/3,2,2,3			5.10	Very stiff fissured dark grey silty CLAY with some sandy laminations	× × × × × × × × × × × × × × × × × × ×					
6.50-6.95	SPT N=14	5.50	DRY	1,2/2,3,4,5					X		92 (Sept. 1997)			
8.00-8.45	U1	5.50	DRY	100 blows					× · · · · · · · · · · · · · · · · · · ·					
9.50-9.95	D1 SPT N=21	5.50	DRY	2,3/5,5,5,6					x x x x x x x x x x x x x x x x x x x					
Remarks Borehole wa UXO survey Chiselling fro	as dry taklen at 1.5m, 3.0n om 0.00m to 1.20m f	n, 5.0m, 9. or 1 hour.	0m, 10.0 Water ad	m Ided from 2.00m to 5	.00m.				Scale (approx)	Lo By	ogged By			
									Figure N					

	-	N G	MANHIF	RE ASSOCIATES	LIMITE	D	Site Manor Road, Richmond I (mOD) Client				
Boring Meth Cable Percus		1	Diamete 0mm cas	r ed to 5.50m	Ground	Level (mOD)	Client Taylor Wimpey West London		N	lob Number 21931	
		Locatio Se	n e site pla	n	Dates	0/08/2022- 1/08/2022	Engineer		S	Sheet 2/4	
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr	
							Stiff becoming very stiff fissured dark grey silty CLAY with some sandy laminations	× × × × × × × × × × × × × × × × × × ×	- - -		
								× × ×	-		
11.00-11.40	U2	5.50	DRY	100 blows		= = ==		× × × × × × × × × × × × × × × × × × ×	<u>.</u>		
11.45	D2							× × × × × × × × × × × × × × × × × × ×			
12.50-12.95	SPT N=29	5.50	DRY	3,4/6,6,8,9				× × × × × × × × × × × × × × × × × × ×			
								× × × × × × × × × × × × × × × × × × ×			
14.00-14.40	U3	5.50	DRY	100 blows		= =_ =_ =_ =_ =_		×			
14.45	D3							× · · · · · · · · · · · · · · · · · · ·] - - -		
15.50-15.95	SPT N=34	5.50	DRY	3,6/7,8,9,10				× × × × × × × × × × × × × × × × × × ×	- -		
17.00-17.30	U4	5.50	DRY	100 blows				× · · · · · · · · · · · · · · · · · · ·			
17.35	D4							× × × × × × × × × × × × × × × × × × ×	- -		
18.50-18.95	SPT N=34	5.50	DRY	3,6/6,9,9,10				× · · · · · · · · · · · · · · · · · · ·	-		
						<u>E</u> <u>E</u> <u>E</u> <u>E</u>		× × × × × × × × × × × × × × × × × × ×	- - -		
Remarks	<u> </u>					<u>F</u>	<u> </u>	Scale (approx)	F	ogged By	
								1:50		ljs	
								Figure N	No. 67.B	:H3	

	-	M	ANHIR EO EN	E ASSOCIATES VIRONMENTAL	LIMITE	D	Site Manor Road, Richmond	N	orehole lumber BH3	
Boring Meth Cable Percus			Diamete 0mm cas	r ed to 5.50m	Ground	Level (mOD)	Client Taylor Wimpey West London		N	ob lumber 21931
		Locatio Se	n e site pla	n	Dates	0/08/2022- 1/08/2022	Engineer		SI	heet 3/4
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr
20.00-20.40	U5	5.50	DRY	100 blows			Very stiff fissured dark grey silty CLAY with some sandy laminations	× · · · · · · · · · · · · · · · · · · ·	<u></u>	
20.45	D5					<u> </u>		× · · · · · · · · · · · · · · · · · · ·	-	
21.50-21.95	SPT N=39	5.50	DRY	3,6/7,9,10,13				X X X X X X X X X X X X X X X X X X X		
23.00-23.30	U6 D6	5.50	DRY	100 blows				x		
24.50-24.95	SPT N=46	5.50	DRY	4,6/7,13,13,13				x		
26.00-26.30 26.30	U7 D7	5.50	DRY	100 blows				X		
27.50-27.95	SPT N=46	5.50	DRY	4,7/9,10,13,14				X		
29.00-29.30	U8	5.50	DRY	100 blows				× × × × × × × × × × × × × × × × × × ×		
29.35	D8							× × ×		
30.00-30.45 Remarks	SPT N=47	5.50	DRY	4,7/9,10,14,14		30.05		Scale (approx)	l l	ogged by
								1:50		ljs

MANHIRE ASSOCIATES GEO-ENVIRONMENTAL LIMITED Boring Method Casing Diameter Cable Percussion 150mm cased to 5.50m												
					Ground	Level (mOD)				ob umber 1931		
		Location	n e site plan	ı	Dates 10 11	1/08/2022- 1/08/2022	Engineer		SI	heet 4/4		
Depth (m)	Sample / Tests	Casing Depth (m)	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend	Water	Instr		
						(24.95) 30.05	Very stiff fissured dark grey silty CLAY with some sandy laminations		1			
							Complete at 30.50m					
Remarks	<u>I</u>				1		1	Scale (approx)	Lc B	ogged y		
								1:50		ljs		
								Figure N 556	No. 37.BH	- 13		

Debut		-	MAN	HIRE ASSOCIATE ENVIRONMENTA	j.	Site Manor Road, Richmond	Number WSA		
Septembrie			Dimension	ns	Ground	Level (mOD)	Tarabas and a superior and a superior of the superior and a superi	Number	
BLOCK PAVING BLOC				ite plan	Dates 10	0/08/2022	Engineer		
Complete at 2.12m Comp	Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend Nater	
2.00 C4 C4 C4 C4 C4 C4 C4 C5 C5 C6						0.06 (0.06) 0.12 (0.14) (0.30) (0.30) 0.56 (0.17)	MADE GROUND: Sand MADE GROUND: Reddy brown angular gravel MADE GROUND: Dark brown and grey gravelly sand with some clayey pockets and brick fragments MADE GROUND: Brown sandy gravel		
	1.50	C3				(1.39)			
1:25 ljs Figure No.	2.00	C4				2.12	Complete at 2.12m		
Figure No.	Remarks						Scale (approx	Logged By	
							Figure		

	-	MAN	HIRE ASSOCIATE ENVIRONMENTA	S L LIMITED	i.	Site Manor Road, Richmond	Number WSB	
Excavation Drive-in Win	Method dow Sampler	Dimension	s	Ground	Level (mOD)	Client Taylor Wimpey West London	Job Number	
		Location See si	te plan	Dates 10	//08/2022	Engineer	Sheet 1/1	_
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend Nate	
0.50	C1				(0.06) - (0.06) - (0.06) - (0.12) - (0.16) - 0.28	BLOCK PAVING MADE GROUND: Sand MADE GROUND: Reddy brown angular gravel MADE GROUND: Brown sandy clay with some gravel, brick and concrete fragments		
1.50	С3							
2.10	C4				2.10	Complete at 2.10m		
Remarks						Scale (approx)	Logged By	-
						1:25	ljs	-
						Figure I	No. 67.WSB	

	-	MAN	HIRE ASSOCIATE ENVIRONMENTA	S L LIMITED		Site Menor Road, Richmond	Number WSC
Excavation Drive-in Win	Method dow Sampler	Dimension	s	Ground	Level (mOD)	Client Taylor Wimpey West London	Job Number 21031
		Location See s	ite plan	Dates 11	/08/2022	Engineer	Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend Nater
0.80 1.05 1.50	C1 C2 C3				(0.06)	MADE GROUND: Sand MADE GROUND: Reddy brown angular gravel MADE GROUND: Granite blocks MADE GROUND: Brown sandy gravel with some bricks and concrete MADE GROUND: Dark grey rounded gravel MADE GROUND: Grey and brown sandy clay with occasional brick fragments Moderately compact orange brown and brown sandy GRAVEL Complete at 2.13m	
Remarks						Scal (appro	i ljs
							re No. 5567.WSC

	-	MAN	HIRE ASSOCIATE	S L LIMITED		Site Manor Road, Richmond	Number WSD
Excavation Drive-in Win	Method dow Sampler	Dimension	s	Ground	Level (mOD)	Client Taylor Wimpey West London	Job Number 21931
		Location See s	ite plan	Dates 12	2/08/2022	Engineer	Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend X
0.80 1.00 1.45 2.00	C1 C2 C3				(0.40) -	MADE GROUND: Concrete rubble with some brown silty clay and occasional gravel MADE GROUND: Orange brown and brown sandy clay with some gravel and brick fragments MADE GROUND: Dark grey and brown sandy gravel Moderately compact orange brown and brown sandy GRAVEL Complete at 2.70m	
Remarks						Scale (approx)	Logged By
						1:25	ljs
						Figure 55	No. 67 WSD

	-	MAN	HIRE ASSOCIATE ENVIRONMENTA	S L LIMITED)	Site Manor Road, Richmond	Number WSE
Excavation Drive-in Wir	Method ndow Sampler	Dimension	s	Ground	Level (mOD)	Client Taylor Wimpey West London	Job Number 21631
		Location See s	ite plan	Dates 11	/08/2022	Engineer	Sheet 1/1
Depth (m)	Sample / Tests	Water Depth (m)	Field Records	Level (mOD)	Depth (m) (Thickness)	Description	Legend se
0.60 1.10 1.50 2.06	C1 C2 C3				. (0.25) - (0.28) - (0.28) - (0.53) - (0.63) - (0.90) - (MADE GROUND: Reddy brown angular gravel MADE GROUND: Brick and concrete rubble with some gravel Moderately compact orange brown and brown sandy GRAVEL Complete at 2.06m	
Remarks						Scal (appro	e Logged x) By
						1:25	
							re No. 5567 WSE

Location See site plan Dates 11/08/2022 Engineer St	mber /SF	Site Manor Road, Richmond)	MANHIRE ASSOCIATES GEO-ENVIRONMENTAL LIMITED									
See site plan 11/08/2022	mber	TO SECURE A PROPERTY OF A SECURE ASSESSMENT	evel (mOD)	Ground	ns	Dimension							
0.50 C1	eet 1/1	Engineer	08/2022	Dates 11.	site plan								
0.50 C1 1.00 C2 1.50 C3 C4 C1 C2 C3 C4 C4 C4 C4 C5 C6 C7 C8 C8 C9 C9 C9 C9 C9 C9 C9 C9	Water	Description	Depth (m) (Thickness)	Level (mOD)	Field Records	Water Depth (m)	Sample / Tests	Depth (m)					
1.00 C2		MADE GROUND: Brown and light brown mottled silty clay	-				C1	0.50					
1.50 C3 C3 C4 Moderately compact orange brown and brown sandy GRAVEL (1.11) (2.20) C4 Moderately compact orange brown and brown sandy GRAVEL (1.11) (2.21) (3.21) (4.21) (5.21) (6.21) (7.21) (8.22) (9.22) (1.21) (9.22) (1.21) (9.22) (1.21) (9.22) (1.22)		2	 - -				C2	1.00					
2.20 C4 2.21		Moderately compact orange brown and brown sandy GRAVEL	- 1.10 				СЗ	1.50					
2.20 C4 = 2.21			 										
		Complete at 2.21m					C4	2.20					
	gged js	1:25						Remarks					



APPENDIX B STANDPIPE RECORDS

Tel: 020 8390 9097 Fax: 020 8390 7888

STANDPIPE RECORDS

WATER LEVELS

HOMEBASE, 84 MANOR ROAD, RICHMOND UPON THAMES Taylor Wimpey West London

Project No. 21031 Sheet No. I/I

Project: Client:

		K					
		Water level					
		Wat					
~							
BH3						·	
				4.15	4.17	4.2	
			Œ				
BH2			m OD				
Bł			m bgl	=	4.15	===	
			m	4.	4.	4.2	
			m OD				
BHI			m 				
В			m bgl	2.51	2.54	2.61	
			u				
ion	evel		e	19/08/2022	23/09/2022	11/10/2022	
Location	Red. level		Date	./80/6	3/09/	/01/	
					7	_	

Remarks







APPENDIX C LABORATORY TEST RESULTS

Tel: 020 8390 9097 Fax: 020 8390 7888

SUMMARY OF GEOTECHNICAL TESTS

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB

Project No: 21031 Sheet No: 1/3

Project: HOMEBASE, MANOR ROAD, Client: Taylor Wimpey West London

Hd	7.92	8.15			7.84			
Soil (Sol)	0.46	0.36			0.25			
Sulpha Water g/l								
cu, kPa Øu, deg								
Cohe cu, kPa assuming ϕ u = 0		139	<u>00</u>	171	117	80	139	981
Deviator Stress kPa	133	273	201	355	237	091	277	371
Radial Stress kPa	091	220	280	340	400	460	520	580
Bulk Density Mg/m³	2.02	2.07	2.01	2.07	2.01	2.09	2.04	2.06
Moisture Content	26	27	24	25	27	24	29	26
Type		UU 102	UU 102	UU 102	UU 102	UU 102	UU 102	UU 102
Class								
Mod. Plast. Index								
Passing 425µm								
Plast. Index %								
Plastic Limit								
Liquid Limit %								
Natural Moisture Content %								
Description	Firm to stiff fissured dark grey silty CLAY with some sandy laminations	Stiff fissured dark grey silty CLAY with some sandy laminations	Stiff fissured dark grey silty CLAY with some sandy laminations	Very stiff fissured dark grey silty CLAY with some sandy laminations	Stiff fissured dark grey silty CLAY with some sandy laminations	Stiff fissured dark grey silty CLAY with some sandy laminations	Stiff fissured dark grey silty CLAY with some sandy laminations	29.00 Very stiff fissured dark grey silty CLAY with some sandy laminations
e Depth	8.00	00.11	14.00	17.00	20.00	23.00	26.00	29.00
Sample	<u> </u>	⊃	⊃	<u></u>	⊃	⊃	⊃)
ocation	BHI							
	Description Descri	Description Natural Liquid Moisture Plastic Limit Plastic Content Limit	Natural Liquid Plastic Plast	Natural Liquid Plastic Plastic Plastic Plastic Content Conte	Description Description	Natural Liquid Plastic Plast Plast Plast	Property to the first of the composition Property to the compo	Particle Plant P

Note: Soil Classification based upon unmodified Plasticity Index



SUMMARY OF GEOTECHNICAL TESTS

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Taylor Wimpey West London

Project No: 21031 Sheet No: 2/3

Project: Client:

	_									
	Hd				7.99			7.96		7.87
CHEMICAL	Sulphate (SO4)	Soil	(Sol)	1/g	0.23			0.27		0.17
	Sulpha	Water		g/1						
	Cohesion	cu, kPa	Øu, deg							
STRESS		cu, l	assuming	Øu = 0	28	134	125	161	130	132
TRIAXIAL COMPRESSION - TOTAL STRESS	Deviator			кЪа		267	250	382	260	264
IPRESSIO	Radial	Stress		кЪа	091	220	280	340	400	460
XIAL CON	Bulk			Mg/m ³	2.00	2.04	2.04	2.10	2.03	2.02
TRIA	Moisture	Content		%	30	25	27	25	26	26
	Type	;			UU 102	UC 102	UU 102	UU 102	UU 102	00 103
	Class									
	Mod.		Index	%						
NOI	Plast. Passing	425µm		%						
CLASSIFICATION				%						
CLAS	Plastic	Limit		%						
	Liquid	Limit		%						
	Natural		Content	%						
	Description				Stiff fissured dark grey silty CLAY with some sandy laminations	II.00 Stiff fissured dark grey silty CLAY with some sandy laminations	14.00 Stiff fissured dark grey silty CLAY with some sandy laminations	Very stiff fissured dark grey silty CLAY with some sandy laminations	Stiff fissured dark grey silty CLAY with some sandy laminations	23.00 Stiff fissured dark grey silty CLAY with some sandy laminations
	Location Sample Depth	•		В	8.00	00:11	14.00	17.00	20.00	23.00
	Sample	Š			⊃	⊃	⊃	⊃	⊃	D
	Location				BH2					

Note: Soil Classification based upon unmodified Plasticity Index



SUMMARY OF GEOTECHNICAL TESTS

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB

Project No: 21031 Sheet No: 3/3

Project: HOMEBASE, MANOR ROAD, Client: Taylor Wimpey West London

	Hd				7.94	7.83			7.77			
CHEMICAL	(SO4)	Soil	(Sol)	1/2	0.48	0.29			0.34			
	Sulphate (SO4)	Water		g/l								
	sion	cu, kPa	Øu, deg									
STRESS	Cohesion	cu, kPa	assuming	0= nØ	111	<u> </u>	84	154	136	183	961	
TRIAXIAL COMPRESSION - TOTAL STRESS	Deviator	Stress	:	кРа	235	225	167	307	271	366	392	
PRESSION	Radial	Stress		кРа	091	220	280	340	400	460	580	
XIAL COM	Bulk		•	Mg/m³	66:1	2.05	2.02	2.08	2.03	2.06	2.03	
TRIA	Moisture	Content	1	%	28	25	23	25	27	26	27	
	Type				UU 102	UU 102	UU 102	UU 102	UU 102	UU 102	UU 102	
	Class											
	Mod.	Plast.	Index	%								
NOI	Plast. Passing	425µm	ì	%								
CLASSIFICATION	Plast.	Index	1	%								
CLAS	Plastic	Limit	ł	%								
	Liquid		1	%								
	Natural	Moisture	Content	%								
	Description				Stiff fissured dark grey silty CLAY with some sandy laminations	II.00 Stiff fissured dark grey silty CLAY with some sandy laminations	14.00 Stiff fissured dark grey silty CLAY with some sandy laminations	17.00 Very stiff fissured dark grey silty CLAY with some sandy laminations	Stiff fissured dark grey silty CLAY with some sandy laminations	23.00 Very stiff fissured dark grey silty CLAY with some sandy laminations	29.00 Very stiff fissured dark grey silty CLAY with some sandy laminations	
	Depth			E	8.00	00. 	14.00	17.00	20.00	23.00	29.00	
	Sample	Š)	⊃	⊃	<u></u>	⊃	⊃	⊃	
	Location Sample Depth				ВНЗ							

Note: Soil Classification based upon unmodified Plasticity Index



ONE - DIMENSIONAL CONSOLIDATION TEST

Project No: 21031 Sheet No. 1/3

Project: HOMEBASE, 84 MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Client: Taylor Wimpey West London

Borehole	Sample	Depth, m
H	5	8.00
Description		
1 T T 37.7	△ ∨ 	> 7

Depth, m	8.00		CLAY	Dry Density	Mg/m³	1.544		Coefficient of	Compressibility	m²/MN		0.062		0.192		0.174		0.117		0.117			
Sample	Б	ı	Stiff dark brown intact clean CLAY	Moisture Cont.	%	start 26	finish 25	Coefficient of	Consolidation	m²/year		0.839		0.599		0.440		0.595		0.289			
Borehole	BHI	Description	Stiff dark b	Specific	Gravity	2.750	measured	Pressure		kPa	0		09		120		240		480		09		

Voids Ratio vs log Effective Pressure						0001 001	Effective Vertical Pressure, kPa
	0.80	0.78	oitsA sbic	0.70	0.08	–	



ONE - DIMENSIONAL CONSOLIDATION TEST

Project No: 21031 Sheet No. 2/3

Project: HOMEBASE, 84 MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Client: Taylor Wimpey West London

Depth, m	20.00		d clean CLAY	Dry Density	Mg/m³	1.543		Coefficient of	Compressibility	m²/MN		0.142		0.184		0.132		0.085		690'0			
Sample	O.S	ı	Very stiff dark brown fissured clean CLAY	Moisture Cont.	%	start 23	finish 22	Coefficient of	Consolidation	m²/year		1.216		0.583		0.472		0.702		0.288			
Borehole	BH2	Description	Very stiff d	Specific	Gravity	2.720	measured	Pressure		kPa	0		<u>8</u>		200		400		800		<u>8</u>		

						00001 0001	
Voids Ratio vs log Effective Pressure						001	Effective Vertical Pressure, kPa
	00					01	
6	0.80	6/:0	oitsЯ sbioV	60 64	0.55	-	



ONE - DIMENSIONAL CONSOLIDATION TEST

Project No: 21031 Sheet No. 3/3

Project: HOMEBASE, 84 MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Client: Taylor Wimpey West London

Borehole	Sample	Depth, m
BH3	U2	26.00
Description	ı	
Very stiff d	Very stiff dark brown intact clean CLAY	clean CLAY
Specific	Moisture Cont.	Dry Density
Gravity	%	Mg/m³
2.750	start 20	1.750
measured	finish 19	
Pressure	Coefficient of	Coefficient of
	Consolidation	Compressibility
kPa	m²/year	m²/MN
0		
	0.646	0.188
<u>8</u>		
	0.592	0.179
200		
	0.502	0.122
400		
	0.682	0.073
800		
	0.433	0.073
<u>80</u>		

					0000	
					000	
				•	≌	
sure			•			ъ
e Press						ıre, kP
 Effectiv					0	Pressu
ws log I		•			<u>8</u>	/ertica
Voids Ratio vs log Effective Pressure						Effective Vertical Pressure, kPa
Voids						Εŧ
					<u>o</u>	
	00					
		2 6	o 1	<u> </u>	_ ,	
	9 7	Ratio			. ,	



HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Project: Client:

Project No: 21031 Sheet No: 1/4

Taylor Wimpey West London

Cobbles <u>8</u> coarse Brown and orange brown SAND & GRAVEL medium Gravel Sieve fine Description:coarse medium Sand Particle Size (mm) fine <u>-</u> coarse medium Silt 0.0 fine Hydrometer 0.00 Clay BH 2.00 Depth, m: Location: Sample: 0.000 2 6 30 20 <u>_</u> <u>8</u> 8 8 9 22 Percentage Passing

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Project: Client:

Project No: 21031 Sheet No: 2/4

Taylor Wimpey West London

Cobbles <u>8</u> coarse Brown and orange brown gravelly medium SAND medium Gravel Sieve fine Description:coarse medium Sand Particle Size (mm) fine <u>-</u>. coarse medium Silt 0.0 fine Hydrometer 0.00 Clay BH 5.00 Location: Depth, m: Sample: 0.000 2 30 20 <u>_</u> <u>8</u> 8 8 9 22 8 Percentage Passing

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Project: Client:

Project No: 21031 Sheet No: 3/4

Taylor Wimpey West London

Cobbles <u>8</u> Brown and orange brown sandymedium to coarse GRAVEL coarse medium Gravel Sieve fine Description:coarse medium Sand Particle Size (mm) fine <u>-</u> coarse medium Silt 0.0 fine Hydrometer 0.00 Clay BH3 2.00 Depth, m: Location: Sample: 0.000 2 30 20 <u>_</u> <u>8</u> 8 8 9 22 8 Percentage Passing

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Project: Client:

Project No: 21031 Sheet No: 4/4

Taylor Wimpey West London

Cobbles <u>8</u> coarse Brown and orange brown sandy coarse GRAVEL medium Gravel Sieve fine Description:coarse medium Sand Particle Size (mm) fine <u>-</u>. coarse medium Silt 0.0 fine Hydrometer 0.00 Clay BH3 4.00 Location: Depth, m: Sample: 0.000 2 6 30 20 <u>_</u> <u>8</u> 8 8 9 22 Percentage Passing



CONTAMINANTS IN SOIL

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB

Project No: 21031 Sheet No: 1/2

Taylor Wimpey West London

Project: Client:

SIOUSILA	J. D. Volonom Jos	0.1^	0. V	0: V	0: V	<u>o.</u> <u>o.</u>	0. V	380	1200	1300			
	lejo _t												
SNOO sq rhdl	0kg - 0ES												
**HdI	0 _{ES} _0 _{ES}												
	025.015												
3/1/2		021	49	0	00	88 30	140	3700	40000	730000	81000		
unibene ¹		4	37	4	45	27	32	410	1200	0006	2000		
nuingles		0. V	<u>^</u>	0: V	0: V	0. v v	0. ^	250	430	12000	00	320	13000
ISASIA		21	9	21	21	<u>4</u> $\overline{\omega}$	<u>6</u>	081	8	980	230		
AMO _L OW.	sinegroni	9.0	<0.3	0.8	0.7	<0.3	<0.3	40	26	00 -	120		
Deary		450	0	420	120	76 100	200						
¹³ ddo ₂		21	27	29	39	89	83	2400	7100	00089	12000	130	1800
MUNIMOND	Wester to	 	22	<u>&</u>	27	22	23	016	016	8600	1500		
Cadium		<0.2	<0.2	<0.2	<0.02	<0.2	<0.2	=	82	061	120		
HOTOR	70 ₅ 13/12 ₁₄	0.7	3.	0.8	<u>0:</u>	3.4 2.1	2.1	290	000	240000	21000		
THININ BE		0.79	0.76	0:	0.84	0.61	8.0	1.7	1.7	12	2.2		
Mulins		0 =	0	150	150	110	330						
NINS IN			4	22	<u>8</u>	- 2	1	37	4	640	79	32	640
A) do d	W	00:1	0.80	09:0	0.50	0.50	0.80	residential³	residential³ª	commercial	POS resi*	residential	commercial
Pldiuns.							()		resid	Com			com
Location		표	BH2	BH3	WSA	WSB	WSC	S4UL				CLEA ²	

I. S4UL given at 6% soil organic matter

2. CLEA SGVs given at 6% soil organic matter

3. Residential with plant uptake

3a. Residential without plant uptake

*. Public open space near residential housing

MANHEE ASSECTION OF THE STATES

Exceptions denoted thus: Residential

All units are mg/kg dry weight of soil unless otherwise stated, except for pH which is dimensionless



CONTAMINANTS IN SOIL

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB

Project No: 21031 Sheet No: 2/2

Taylor Wimpey West London Project: Client:

	<i>3</i> ₂ ,					_	_			
Slonolly	3/b?donom 301	0. <u>v</u> <u>v</u>	<u>0.</u>		380	1200	1300			
	l _{is} jo,ţ									
SWOO AG FIREL	015 05									
, stidl	0 _{ES} _0 _{ES}									
	025.015									
Silis		81	54		3700	40000	730000	81000		
unibenev		34	56	 	410	1200	0006	2000		
nuinsles		0. N 0. N	0. V		250	430	12000	00	350	13000
18 A SIA		21	<u>8</u>		081	<u>8</u>	086	230		
LINOJON .	3integroni	0.6 	<0.3		40	26	8	120		
PEST		74 720	170							
19ddo _O		27 56	26		2400	7100	00089	12000	130	1800
Chrominn	1113/12/111	27	<u>6</u>		016	016	8600	1500		
adiun		<0.2	<0.2		=	82	<u>8</u>	120		
40108	ios Islem	0.5	2.0		730	000	240000	21000		
Mulling		0.66	0.47		7.1		12	2.2		
Mulins			63							
Sinski		19	<u>m</u>		37	9	640	79	32	640
Aldold .	W	0.80	09:0		intial ³	residential ^{3a}	ercia	resi*	entia	ercial
³ Idu _{tes}					residential	reside	comm	POS resi*	reside	commercia
Location		MSD	WSE		S4UL'				CLEA ²	

I. S4UL given at 6% soil organic matter

All units are mg/kg dry weight of soil unless otherwise stated, except for pH which is dimensionless

Exceptions denoted thus: Residential XX

Commercial XX

2. CLEA SGVs given at 6% soil organic matter

3. Residential with plant uptake

3a. Residential without plant uptake

*. Public open space near residential housing



Project No: 21031 Sheet No: 1/2

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB

Taylor Wimpey West London Project: Client:

		Specie	ated Total Petro	leum Hydrocai	Speciated Total Petroleum Hydrocarbons (Aromatic / Aliphatic Split with BTEX)	/ Aliphatic Spl	it with BTEX)			
Location	BHI	BH2	BH3	WSA	WSB	WSB	WSC		LQM/CIEH	
Sample									S4UL	
Depth, m	00.1	08.0	09:0	0.50	0.50	00:1	080	residential	allotments	commercial
Determinand				Concentral	Concentration, mg/kg					
Aromatic Hydrocarbons										
C5 - C7	<0.001	<0.001	<0.001	<0.001	<0.001	<0.00	 	300	57	86000
>C7 - C8	<0.00	<0.00	<0.001	<0.001	<0.00	<0.001	<0.001	099	120	000081
>C8 - C10	<0.00	 	<0.001	00:0>	<0.001	<0.00	<0.001	061	5.	17000
>CI0-CI2	0. >	0. ^	0.1^	0.1>	=	8.8	0.1>	380	74	34000
>CI2 - CI6	<2.0	6.5	7.6	7.3	23	6	8.3	099	130	38000
>C16 - C21	<u>0 </u> ×	23	37	30	49	37	23	930	260	28000
>C2I - C35	0 >	09	82	74	92	70	54	1700	0091	28000
Total Aromatic Hydrocarbons	<u>0</u>	88	130	<u>0</u>	180	40	82			
Aliphatic Hydrocarbons										
C5 - C6	<0.001	 	<0.00	<0.001	<0.001	<0.00	00:0>	091	3900	12000
>C6 - C8	<0.001	 	<0.001	<0.001	<0.001	<0.00	00:0>	530	13000	40000
>C8 - CI0	<0.00	 	<0.00	<0.00	<0.001	<0.00	<0.001	150	1700	00011
>CI0-CI2	0. <u> </u> ^	0.	0. ^	0 <u>.</u> ^	0. >	0. >	0. >	760	7300	47000
>C12 - C16	<2.0	13	<2.0	=	20	0	7	4300	13000	00006
>C16 - C21	<8.0	22	<8.0	24	43	3	22			
>C21 - C35	<8.0	48	<8.0	51	0	16	53			
Total Aliphatic Hydrocarbons	0 >	83	<u>0</u> v	82	170	30	83			
Total Petroleum Hydrocarbons	<u>0</u>	172	130	195	350	270	891			
ВТЕХ				Concentra	Concentration, µg/kg					
Benzene	0.1>	0.1>	0.1>	0.1>	0.1>	0.1>	0.1>	370	75	00006
Toluene	0. <u> </u> ^	0:	0.1^	0. <u> </u> >	0.1	0:	0.1>	000099	120000	000000081
Ethyl Benzene	0. ^	0: <u> </u> >	0. >	0 <u>.</u> >	0.1>	0. >	0.1>	260000	00016	27000000
p & m-xylene	0 . ^	0. >	0.1 >	o: >	0.1>	0. >	0.1>	310000	000091	30000000
o-xylene	0. >	0. ^	0. ^	0. <u>l</u> >	0.1 >	0. ^	0: >			
MTBE	0. >	o: >	0.1>	0. >	<1.0	0. >	0.1>			

Notes

Total = Sum of compounds above detection limit.

S4UL given at 6% soil organic matter

*Results given as total of (ortho), (meta) and (para) xylene. SGV given is the lowest permissible value for any xylene compound

*Results given as total of (ortho), (meta) and (para) xylene.

Exceptions denoted thus:

Residentia Commercial Project No: 21031 Sheet No: 2/2

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Project: Client:

Taylor Wimpey West London

Standard S			Specia	ated Total Petro	Speciated Total Petroleum Hydrocarbons (Aromatic / Aliphatic Split with BTEX)	iphatic Split with BTEX)			
Pagin Pagi	Location	WSD	WSD	WSE				LQM/CIEH	
Options of the part of the property of the part of the	Sample							S4UL	
lydrocarbons Concentration, my/leg Concentration, my/leg 300 57 chool <0.001		0.80	1.00	09:0			residential	allotments	commercial
ydrocarbons control	Determinand				Concentration, mg/kg				
Advications \$\cup{continuous}{\cup{continuous}} \text{ conoting \$\cup{continuous}{\cup{continuous}} \text{ conotinuous} con									
Colorad Colo	Aromatic Hydrocarbons								
\$\limin{array}{c c c c c c c c c c c c c c c c c c c	C5 - C7	 	00.0>	<0.001			300	57	86000
Concentration Concentratio	>C7 - C8	 	<0.00	00.0>			099	120	000081
\$\leq 1.0 \$\le	>C8 - C10	<0.00 ×	<0.00	<0.001			061	51	17000
\$\leqsigned \text{c20} \leqsigned \text{c20} \leqsigned \text{c20} \leqsigned \text{c20} \text{c20} \text{c20} \	>CI0 - CI2	0. ^	0.1>	0.1^			380	74	34000
Concentrations Concentration Concentrati	>C12 - C16	<2.0	<2.0	<2.0			099	130	38000
Concentrations Concentration Light Concentration Light Concentration Light Concentration Light Concentration Light Light Light Light Concentration Light Lig	>C16 - C21	0 >	0 >	0 >			930	260	28000
National Page 10 23 10 23 10 23 210 23 210 23 210 23 23 23 23 23 23 23 2	>C2I - C35	0 >	15	0 >			1700	0091	28000
160 39000 3900 3900 3900 3900 3900 3900 3900 3900 390	Total Aromatic Hydrocarbons	0 I v	23	010					
Continue	Alinhatic Hydrocarbons								
Concentration, µg/kg		0	0				3	0000	0000
Concentration, hg/kg Concentration, hg/kg Concentration		00:0>	00.0>	<0.00			09	3900	7000
Concentration, lig/kg Conc	8D - 9D<	 00.0>	<0.00	 			230	3000	40000
Concentration, 19/kg	>C8 - C10	 	<0.00	 			120	1700	000
C2.0 C2.0 26 26 26 26 26 26 26 2	>CI0-CI2	0.	0.	0. <u> </u> ×			260	7300	47000
21	>CI2 - CI6	<2.0	<2.0	26			4300	13000	00006
Secondary Seco	>CI6 - C21	<8.0	<8.0	83					
troleum Hydrocarbons <10 23 300 Concentration, pg/kg <1.0	>C21 - C35	<8.0	<8.0	061					
troleum Hydrocarbons <10 23 300 Concentration, µg/kg < <1.0	Total Aliphatic Hydrocarbons	0 <u>I</u> v	<u>0</u>	300					
Concentration, µg/kg STO TS	Total Petroleum Hydrocarbons	<u>0</u> v	23	300					
concentration, µg/kg Concentration, µg/kg 75 < <1.0									
< -I.0 < -I.0 < -I.0 TS 75 scane < -I.0	ВТЕХ				Concentration, µg/kg				
< 1.0 < 1.0 < 1.0 < 1.0 120000 120000 zene < 1.0	Benzene	0.1	0.1^	0:			370	75	00006
zene <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0	Toluene	0.1^	0. ^	0. V	_	_	000099	120000	180000000
ene <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0 <1.0	Ethyl Benzene	0.1>	0.1^	0. >			260000	00016	27000000
0.1> 0.1>	p & m-xylene	0.1	0.	0. V			310000	000091	30000000
0.1>	o-xylene	0.	0.	0. V					
	MTBE	<i.0< td=""><td>o:I></td><td>o:l></td><td></td><td></td><td></td><td></td><td></td></i.0<>	o:I>	o:l>					

Notes

Total = Sum of compounds above detection limit.

S4UL given at 6% soil organic matter

*Results given as total of (ortho), (meta) and (para) xylene. SGV given is the lowest permissible value for any xylene compound

*Results given as total of (ortho), (meta) and (para) xylene.



CONTAMINANTS IN SOIL

HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB Project: Client:

Project No: 21031 Sheet No: I/I

Taylor Wimpey West London

					Speciate	d Polyaro	matic Hyd	Speciated Polyaromatic Hydrocarbons by GCMS	y GCMS				
	Location	BH2	BH3	WSB	WSB	WSC	WSD	WSE			P P	LQM/CIEH	
∞ 3	Sample											S4UL³	
De	Depth, m 0	0.80	09.0	0.50	00.1	0.80	00.	09.0		residentia	esidential4 residential5	I5 allotments	commercial
Determinand								Conce	Concentration, mg/kg				
РАН													
Naphthalene		<0.05	0.35	4.0	0.43	0.37	9.0	<0.05		<u>13</u>	<u>e</u>	24	8
Acenaphthylene		0.05	0.38	<0.05	<0.05	<0.05	<0.05	<0.05		920	0009	091	00000I
Acenaphthene	<u> </u>	<0.05	0.32	29.0	0.98	0.41	<0.05	<0.05		0011	9009	200	000001
Fluorene	<u> </u>	0.05	0.3	69.0	0.95	0.44	<0.05	<0.05		098	4500	091	71000
Phenanthrene		9:1	2	4.5	5.6	٣	<u>5.</u>	0.22		440	1500	8	23000
Anthracene	_	44.	=	99.0	0.87	0.51	0.21	<0.05		11000	37000	2200	540000
Fluoranthene		3.0	=	3.9	4.9	3.1	2.2	0.7		890	1600	290	23000
Pyrene		3.3	8.6	3.	3.9	5.6	6:	6.0		2000	3800	620	54000
Benzo(a)anthracene		2.2	6.5	1.7	2.1	9:1	4.	0.5		<u>13</u>	2	<u>~</u>	<u>8</u>
Chrysene		6:1	8.9	=:	4.	4.	=:	9.0		27	32	61	350
Benzo(b)fluoranthene		2.3	7.2	1.2	6:	1.7	<u>5.</u>	0.58		3.7	4.0	3.9	45
Benzo(k)fluoranthene		9:	4.4	0.87	29.0	0.58	0.55	0.39		<u>00</u>	<u></u>	130	1200
Benzo(a)pyrene		2.4	7.2	0:	9:	1.5	1.2	0.48		m	3.2	3.5	36
Indeno(123-cd)pyrene		1.2	3.8	19:0	0.73	0.62	0.59	0.28		4	46	39	510
Dibenzo(ah)anthracene		36	66.0	<0.05	<0.05	<0.05	<0.05	<0.05		0.3	0.32	0.43	3.6
Benzo(ghi)perylene		<u>9.</u>	4.6	0.77	0.92	0.88	0.74	0.37		350	360	640	4000
Total PAH (16)	7	21.9	69.7	21.4	26.9	18.7	13.3	4.98					

1. Total PAH = Sum of EPA16 identified components

×

Commercial

Exceptions denoted thus: Residential

2. The results are expressed as mg/kg dry weight soil after correction for moisture content

3. S4UL given at 6% soil organic matter 4.Residential with plant uptake 5.Residential without plant uptake





CONTAMINANTS IN SOIL

Project: HOMEBASE, MANOR ROAD, RICHMOND UPON THAMES, TW9 IYB

Client: Taylor Wimpey West London Sheet No: I/2

Project No: 21031

	Semi-	Volatile Or	ganic Con	mpounds by GC-MS
Location	BHI	WSA	WSD	
Sample				
Depth, m	1.00	0.50	0.80	
Determinand				Concentration, µg/kg
Aniline	<0.I	<0.1	<0.I	
Phenol	<0.2	<0.2	<0.2	
2-Chlorophenol	<0.I	<0.1	<0.I	
Bis(2-chloroethyl)ether	<0.2	<0.2	<0.2	
I,3-Dichlorobenzene	<0.2	<0.2	<0.2	
I,2-Dichlorobenzene	<0.1	<0.1	<0.1	
I,4-Dichlorobenzene	<0.2	<0.2	<0.2	
Bis(2-chloroisopropyl)ether	<0.I	<0.1	<0.1	
2-Methylphenol	<0.3	<0.3	<0.3	
Hexachloroethane	<0.05	<0.05	<0.05	
Nitrobenzene	<0.3	<0.3	<0.3	
4-Methykphenol	<0.2 <0.2	<0.2	<0.2 <0.2	
Isophorone	<0.2	<0.2 <0.3	<0.2	
2-Nitrophenol 2,4-Dimethylphenol	<0.3	<0.3	<0.3	
Bis(2-chloroethoxy)methane	<0.3	<0.3	<0.3	
1,2,4-Trichlorobenzene	<0.3	<0.3	<0.3	
Naphthalene	<0.05	0.25	<0.05	
2,4-Dichlorophenol	<0.03	<0.3	<0.03	
4-Chloroaniline	<0.3 <0.1	<0.3 <0.1	<0.3 <0.1	
Hexachlorobutadiene	<0.1 <0.1	<0.1	<0.1	
4-Chloro-3-methylphenol	<0.1	<0.1	<0.1	
2,4,6-Trichlorophenol	<0.1	<0.1	<0.1	
2,4,5-Trichlorophenol	<0.2	<0.2	<0.2	
2-Methylnaphthalene	<0.I	0.6	<0.1	
2-Chloronaphthalene	<0.I	0.1	<0.I	
Dimethylphthalate	<0.I	<0.1	<0.1	
2,6-Dinitrotoluene	<0.1	<0.1	<0.1	
Acenaphthylene	<0.05	<0.05	<0.05	
Acenaphthene	<0.05	0.3	<0.05	
2,4-Dinitrotoluene	<0.2	<0.2	<0.2	
Dibenzofuran	<0.2	<0.2	<0.2	
4-Chlorophenyl phenyl ether	<0.3	<0.3	<0.3	
Diethyl phthalate	<0.2	<0.2	<0.2	
4-Nitroaniline	<0.2	<0.2	<0.2	
Fluorene	<0.05	0.23	<0.05	
Azobenzene	<0.3	<0.3	<0.3	
Bromophenyl phenyl ether	<0.2	<0.2	<0.2	
Hexachlorobenzene	<0.3	<0.3	<0.3	
Phenanthrene	0.8	2.7	0.23	
	<u> </u>			